#### GENERAL NOTES

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THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

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REINFORCEMENT NOT SHOWN FOR CLARITY IN GENERAL SHEETS, SEE STRUCTURAL DRAWINGS

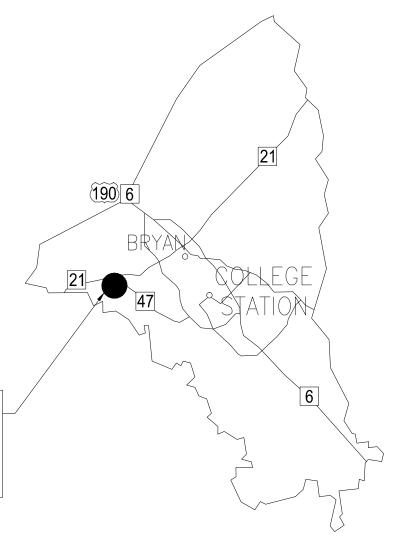
TEXAS A&M TRANSPORTATION
INSTITUTE'S
RIVERSIDE CAMPUS
(LOCATION OF SPECIMENS)

# TEXAS A&M TRANSPORTATION INSTITUTE

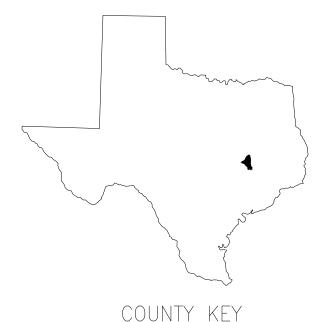
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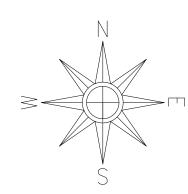
NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM (NCHRP)
PROJECT 14-28

FOR THE CONSTRUCTION OF THE MOCK-UP SPECIMENS SPECIFIED WITHIN THE UPDATED INTERIM REPORT II









Revision notes:		
Rev:	Date:	Notes:

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NCHRP 14-28		

CONDITION ASSESSMENT OF BRIDGE PT AND STAY CABLE SYSTEMS USING NDE METHODS

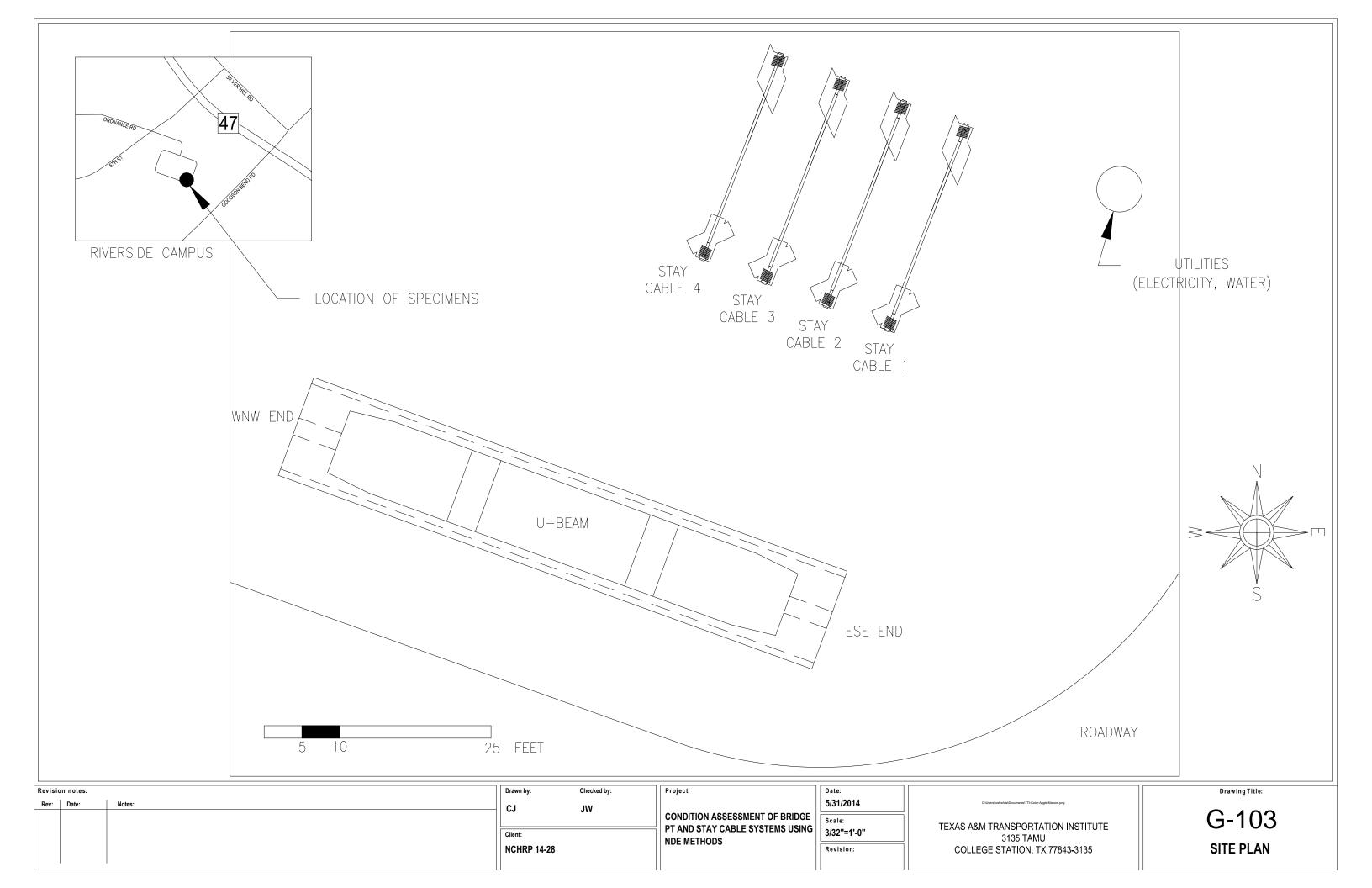
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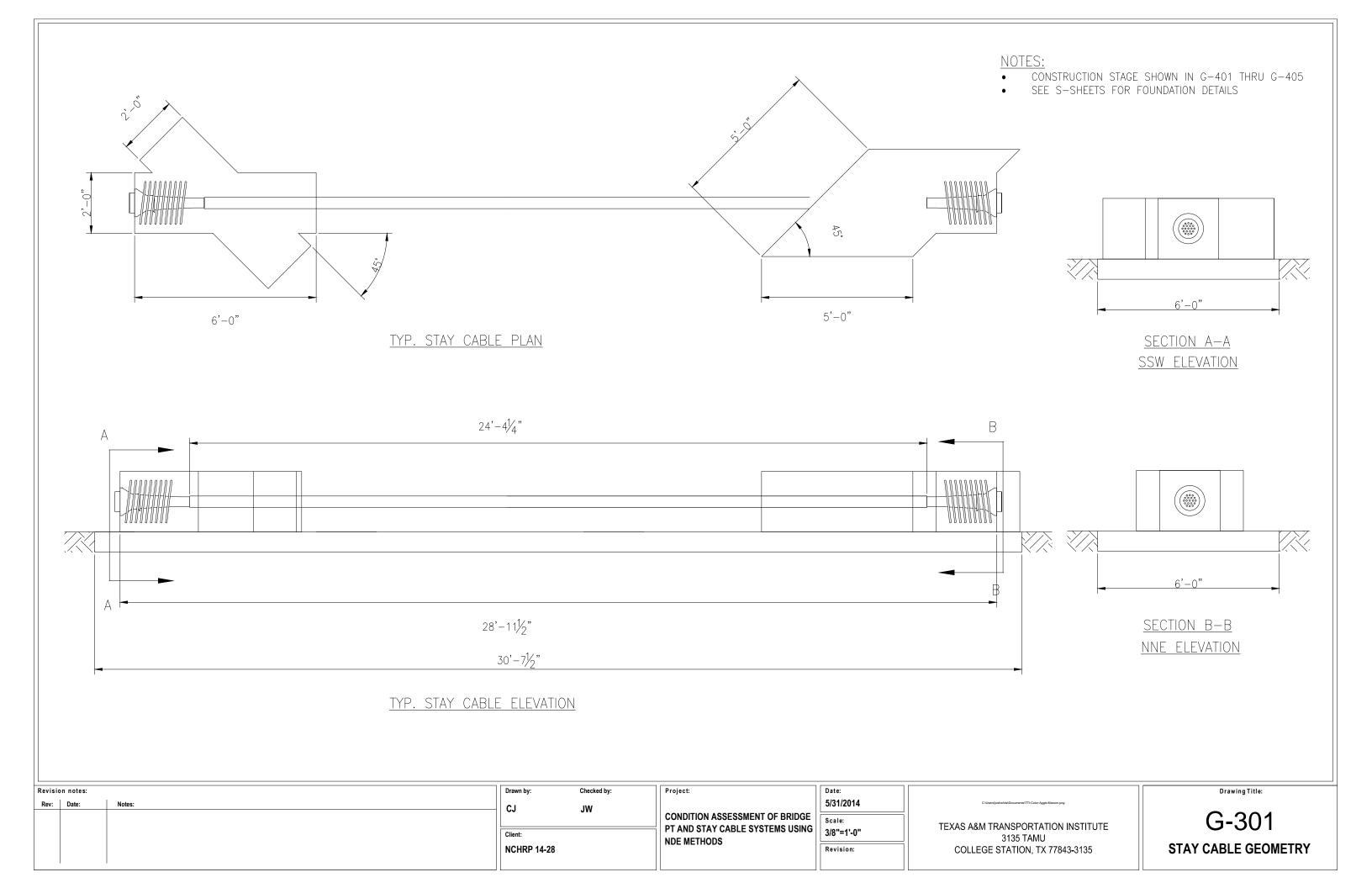
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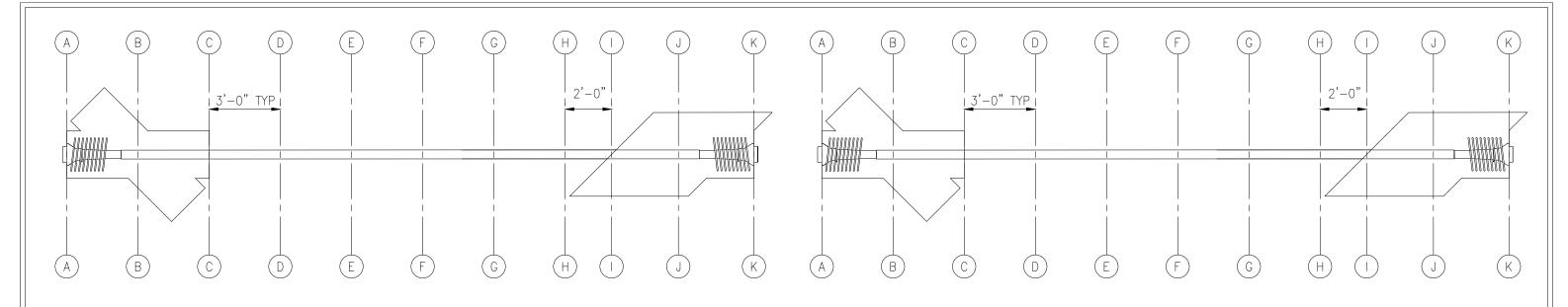
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G-101



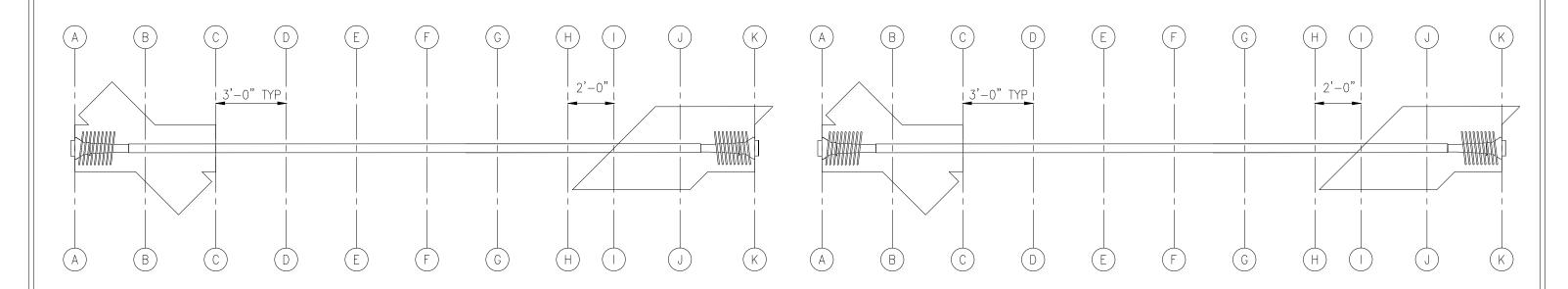




STAY CABLE SPECIMEN 1 (SC1)

STAY CABLE SPECIMEN 3 (SC3)

STAY CABLE SPECIMEN 2 (SC2)



STAY CABLE ID	STAY CABLE CONFIGURATIONS
SC1	GROUTED STRANDS IN STEEL PIPE
C C O	COCUTED STRAIDS IN LIDDE DIDE

### NOTES:

- CONSTRUCTION STAGES SHOWN IN G-401 THROUGH G-405
- GRID H-I IS 2'-0", ALL OTHER DEFECT GRIDS ARE 3'-0"

STAY CABLE ID	STAY CABLE CONFIGURATIONS
SC1	GROUTED STRANDS IN STEEL PIPE
SC2	GROUTED STRANDS IN HDPE PIPE
SC3	UNGROUTED MONOSTRANDS GREASED & SHEATHED IN HDPE PIPE
SC4	UNGROUTED EPOXY COATED STRANDS IN HDPE PIPE

STAY CABLE SPECIMEN 4 (SC4)

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	CONDITION ASSESSMENT OF BRIDGE
]	PT AND STAY CABLE SYSTEMS USING
	NDE METHODS
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TEXAS A&M TRANSPORTATION INSTITUTE 3135 TAMU COLLEGE STATION, TX 77843-3135

G-302 **STAY CABLE GEOMETRY** 

CONDITION DESCRIPTIONS						
CONDITION TYPE CONDITION CODE CONDITION DESCRIPTION		CONDITION TYPE	CONDITION CODE	CONDITION DESCRIPTION		
Z	CW1	LIGHT-MODERATE WIRE PITTING (<15% WCS, <2% SCS, <1% TCS)	STRAND BREAKAGE	BS1	1 OF 7 WIRES FRACTURED (14% SCS, <2% TCS)	
WIRE	CW2	SEVERE WIRE PITTING (35-65% WCS, 5-9% SCS, <1% TCS)	STRA	BS2	3 OF 7 WIRES FRACTURED (43% SCS, 2-4% TCS)	
00	CW3	EXTREME WIRE PITTING (85-100% WCS, 12-14% SCS, <2% TCS)		BT1	1 OF 19 STRANDS OR 1 OF 12 STRANDS FRACTURED (5-8% TCS)	
Z	CS1	1-2 OF 7 WIRES FULLY CORRODED (14-29% SCS, <3% TCS)	JON	BT2	3 OF 19 STRANDS OR 2 OF 12 STRANDS FRACTURED (16-17% TCS)	
STRAND	CS2	3-4 OF 7 WIRES FULLY CORRODED (43-57% SCS, 2-5% TCS)	TENDON BREAKAGE	BT3	10 OF 19 STRANDS OR 6 OF 12 STRANDS FRACTURED (50-53% TCS)	
, 33	CS3	7 OF 7 WIRES FULLY CORRODED (100% SCS, 5-9% TCS)		BT4	19 OF 19 STRANDS OR 12 OF 12 STRANDS FRACTURED (100% TCS)	
	CT1	1-2 OF 19 STRANDS OR 1-2 OF 12 STRANDS FULLY CORRODED (5-16% TCS)		GS1	APPROXIMATELY 50% FULL OF SEGREGATED GROUT	
TENDON	CT2	3-4 OF 19 STRANDS OR 2-3 OF 12 STRANDS FULLY CORRODED (16-25% TCS)	) SED	GS2	100% FULL OF SEGREGATED GROUT	
TENI	CT3	9-10 OF 19 STRANDS OR 6-7 OF 12 STRANDS FULLY CORRODED (47-59% TCS)	COMPROMISED GROUT	GU1	APPROXIMATELY 50% FULL OF UNHYDRATED GROUT	
	CT4	19 OF 19 STRANDS OR 12 OF 12 STRANDS FULLY CORRODED (100% TCS)	CON	GU2	100% FULL OF UNHYDRATED GROUT	
Z	W1	APPROXIMATELY 25% FULL OF WATER		GG	100% FULL OF GASSED GROUT	
WATER	W2	APPROXIMATELY 75% FULL OF WATER		V1	APPROXIMATELY 25% VOIDED	
¥	W3	100% FULL OF WATER	GIOA	V2	APPROXIMATELY 50% VOIDED	
SHEATHING DAMAGE	S1	EPOXY-COATING OF STRAND REMOVED	9	V3	APPROXIMATELY 75% VOIDED	
	S2	SHEATHING OF STRAND REMOVED		V4	100% VOIDED	

WCS - WIRE CROSS SECTION SCS - STRAND CROSS SECTION

TCS - TENDON CROSS SECTION

SHEATHING DAMAGE DEFECTS ONLY APPLY TO CERTAIN SC SYSTEMS:

S1 - SC4

S2 - SC3

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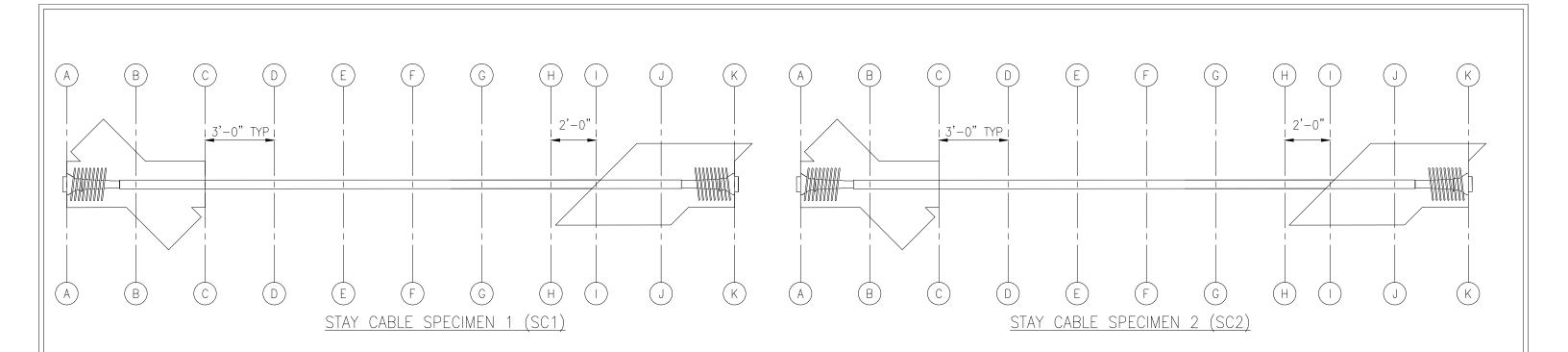
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NDE METHODS	L

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Drawing Title:

G-501 **CONDITION DESCRIPTIONS** 



CONDITION PLACEMENT DIAGRAM AND SCHEDULE—STAY CABLE 1			
LOCATION	CONDITION CODE		
A-B	CT3		
B-C	INTACT		
C-D	INTACT		
D-E	W2		
E-F	CT3		
F-G	V4		
G-H	BT1		
H-I	GS1		
I-J W1/CT1			
J-K	W 1		

STAY CABLE ID	STAY CABLE CONFIGURATIONS
SC1	GROUTED STRANDS IN STEEL PIPE
SC2	GROUTED STRANDS IN HDPE PIPE

CONDITION PLACEMENT DIAGRAM AND SCHEDULE—STAY CABLE 2			
LOCATION	CONDITION CODE		
A-B	INTACT		
В-С	CW1		
C-D	INTACT		
D-E	W2		
E-F	CT3		
F-G	V4		
G-H	BT1		
H-I GS1			
I-J V1/CT1			
J-K	V1		

#### NOTES

- CONSTRUCTION STAGE SHOWN IN G-401 THRU G-405
- SEE S-400 SHEETS FOR FOUNDATION DETAILS
- GRID H-I IS 2'-0", ALL OTHER DEFECT GRIDS ARE 3'-0"

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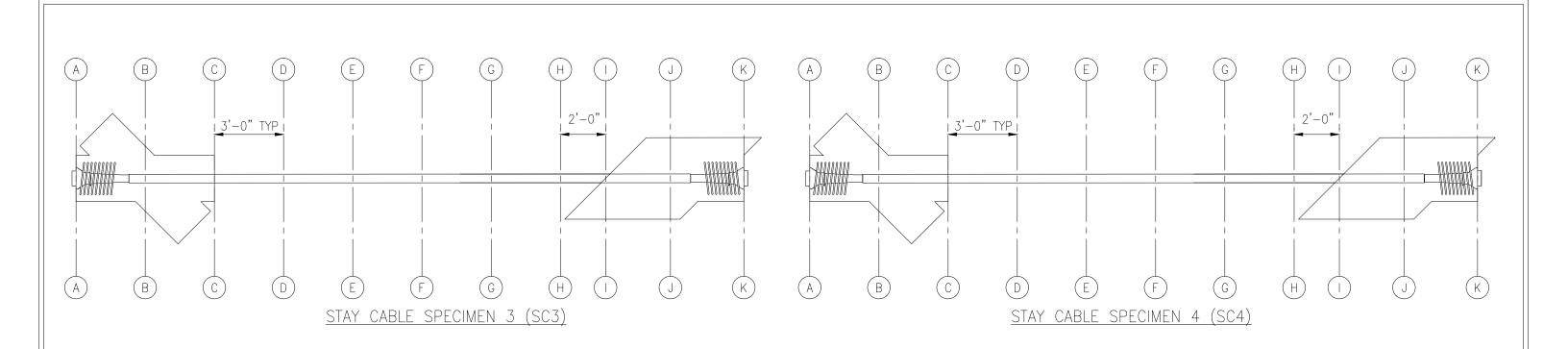
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CONDITION ASSESSMENT OF BRIDGE
PT AND STAY CABLE SYSTEMS USING
NDE METHODS

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TEXAS A&M TRANSPORTATION INSTITUTE 3135 TAMU COLLEGE STATION, TX 77843-3135 Drawing Title:

G-514
CONDITION PLACEMENT
SCHEDULE



CONDITION PLACEMENT DIAGRAM AND SCHEDULE—STAY CABLE 3			
LOCATION	CONDITION CODE		
A-B	CT2		
B-C	INTACT		
C-D	CS1		
D-E	BS1		
E-F	CT3		
F-G	BT1		
G-H	INTACT		
H-I S2			
I-J INTACT			
J-K CS2			

STAY CABLE ID	STAY CABLE CONFIGURATIONS
SC3	UNGROUTED MONOSTRANDS GREASED & SHEATHED IN HDPE PIPE
SC4	UNGROUTED EPOXY COATED STRANDS IN HDPE PIPE

CONDITION PLACEMENT DIAGRAM AND SCHEDULE—STAY CABLE 4				
LOCATION	CONDITION CODE			
A-B	CT3			
B-C	CS2			
C-D BS1				
D-E CT3				
E-F	BT1			
F-G	CW1			
G-H	INTACT			
H-I S1				
I-J INTACT				
J-K	CS1			

- CONSTRUCTION STAGE SHOWN IN G-401 THRU G-405
   SEE S-400 SHEETS FOR FOUNDATION DETAILS
- GRID H-I IS 2'-0", ALL OTHER DEFECT GRIDS ARE 3'-0"

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CONDITION ASSESSMENT OF BRIDGE PT AND STAY CABLE SYSTEMS USING NDE METHODS

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G-515 **CONDITION PLACEMENT SCHEDULE** 

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#### DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR BUILDING DEPICTED HEREON HAS BEEN DESIGNED ACCORDING TO THE 2012 EDITION OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS (6TH ED).

THE STRUCTURE HAS BEEN DESIGNED TO RESIST THE FOLLOWING SUPERIMPOSED LOADS.

LIVE LOAD: 75 PSF

DEAD LOAD: ONLY SELF-WEIGHT (150 PCF)

#### DESIGN APPROACH:

THE STRUCTURE WAS DESIGNED USING LOAD AND RESISTANCE FACTOR DESIGN (LRFD) AS OUTLINED IN THE FOLLOWING DESIGN STANDARDS:

- AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 6TH EDITION, 2012. (CHECKED 2013 REVISIONS)
- PCI BRIDGE DESIGN MANUAL, 3RD EDITION, 2011.

#### CONSTRUCTION:

ALL CONSTRUCTION DRAWINGS ARE PRESENTED AS AN AID TO THE CONTRACTOR AND NOT CONSIDERED WORKING DRAWINGS. ALL DETAILS OF THE CONTRACTOR'S ERECTION SCHEME AND CALCULATIONS TO BE PRESENTED TO THE EOR FOR APPROVAL PRIOR TO IMPLEMENTATION.

# TEXAS A&M TRANSPORTATION INSTITUTE

## TEXAS A&M UNIVERSITY

NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM (NCHRP)
PROJECT 14-28

FOR THE CONSTRUCTION OF THE MOCK-UP SPECIMENS SPECIFIED WITHIN THE UPDATED INTERIM REPORT II

#### FOUNDATIONS:

SHALLOW FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 psf. THIS PRESSURE WAS VERIFIED IN THE REPORT "THE NATIONAL GEOTECHNICAL EXPERIMENTATION SITES AT TEXAS A&M UNIVERSITY: CLAY AND SAND" BY JEAN-LOUIS BRIAUD IN 1997. THE BUILDING PAD SHALL BE COMPACTED TO 95% MODIFIED PROCTOR DENSITY DETERMINED IN ACCORDANCE WITH ASTM D-1557.

FORM WORK AND SHORING:

DESIGN, ERECTION AND REMOVAL OF ALL FORM WORK, SHORES AND RESHORES SHALL MEET REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301. NO FORMWORK SHALL BE STRIPPED UNTIL STRUCTURAL CONCRETE HAS REACHED AT LEAST TWO—THIRDS OF THE 28 DAY DESIGN STRENGTH.

#### MATERIALS:

### CONCRETE:

CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE STRENGTHS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

- FOUNDATIONS AND SLABS ON GRADE: 3000 psi @ 28 DAYS
- ALL STRUCTURAL COMPONENTS: 5500 psi @ 28 DAYS
- ALL STRUCTURAL COMPONENTS: 4400 psi @ TIME OF INITIAL PRESTRESSING

MINIMUM REQUIRED COVER FOR REINFORCING AND PRESTRESSING STEEL:  $1\frac{1}{2}$  IN.

CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C-94 FOR MEASURING, MIXING, TRANSPORTING, ETC. MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS.

#### CONCRETE REINFORCING:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SHOP DRAWINGS DEPICTING QUANTITY AND INTENDED PLACEMENT LOCATION OF REINFORCING STEEL ARE DOCUMENTED HEREIN.

#### POST-TENSIONING STRANDS:

FOR LOCATION OF POST-TENSIONED TENDONS, SEE GENERAL SHEETS.

ALL PT STRANDS ARE 0.6" DIA. GRADE 270 LOW RELAXATION STRANDS

ASSUMED YIELD STRENGTH: 243 ksi

ALL TENDONS TO BE STRESSED FROM ESE SIDE.

#### DESIGN PARAMETERS:

- FRICTION COEFFICIENT: 0.23
- WOBBLE COEFFICIENT: 0.0002
- ANCHORAGE SET: 0.375 IN.
- APPARENT MODULUS TO BE VERIFIED BY MILL REPORT FOR CALCULATIONS OF ELONGATION

#### POST-TENSIONING ANCHORAGES:

PERFORMANCE OF THE ANCHORAGE DEVICE AND FURNISHING OF ANY SUPPLEMENTARY REINFORCEMENT REQUIRED IN THE "LOCAL ZONE" SHALL BE THE RESPONSIBILITY OF THE MATERIAL SUPPLIER. THE "LOCAL ZONE" IS THE REGION IMMEDIATELY SURROUNDING EACH ANCHORAGE DEVICE. IT SHALL BE CONSIDERED A PRISM WITH TRANSVERSE DIMENSIONS EQUAL TO THE SUM OF THE PROJECTED SIZE OF THE BEARING PLATE PLUS THE MANUFACTURER'S SPECIFIED MINIMUM EDGE COVER. THE LENGTH OF THE LOCAL ZONE EXTENDS FOR THE LENGTH OF THE ANCHORAGE DEVICE PLUS AN ADDITIONAL DISTANCE IN FRONT OF THE ANCHOR EQUAL TO THE MAXIMUM LATERAL DIMENSIONS OF THE ANCHOR.

#### STRESSING SCHEDULE:

SEE G-209 FOR STRESSING SCHEDULE.

#### GROUTING INFORMATION:

ALL TENDONS TO BE GROUTED.

SEE S-601 FOR GROUTING INFORMATION.

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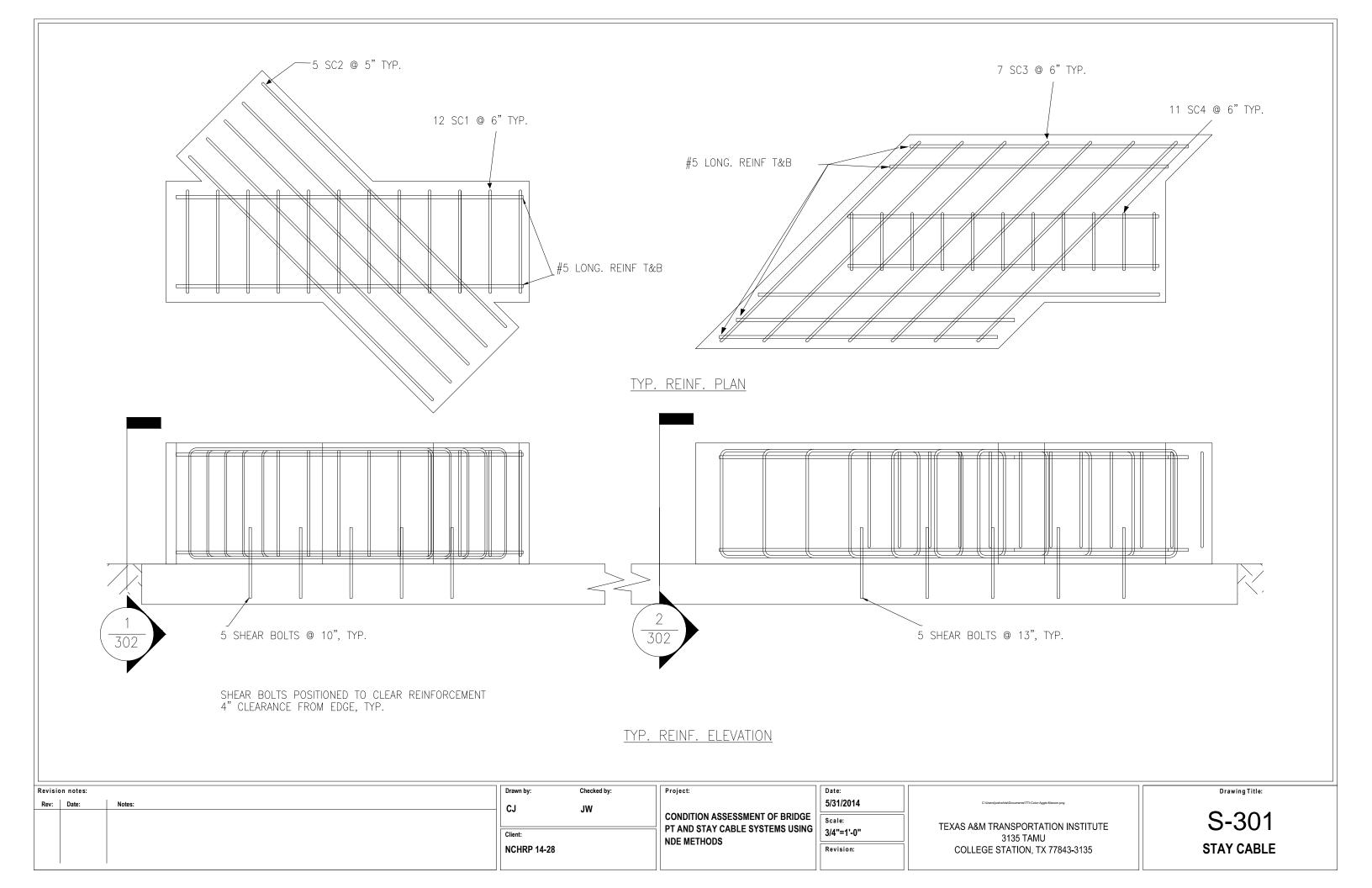
CONDITION ASSESSMENT OF BRIDGE PT AND STAY CABLE SYSTEMS USING NDE METHODS

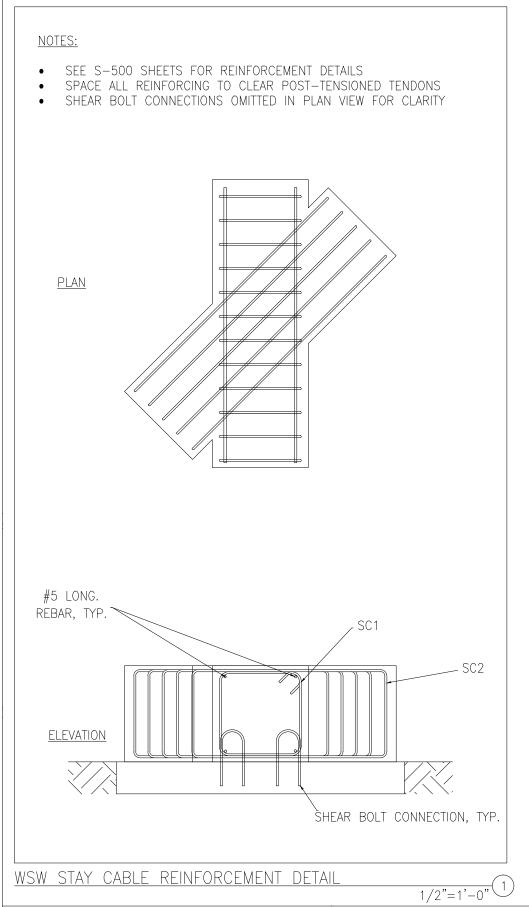
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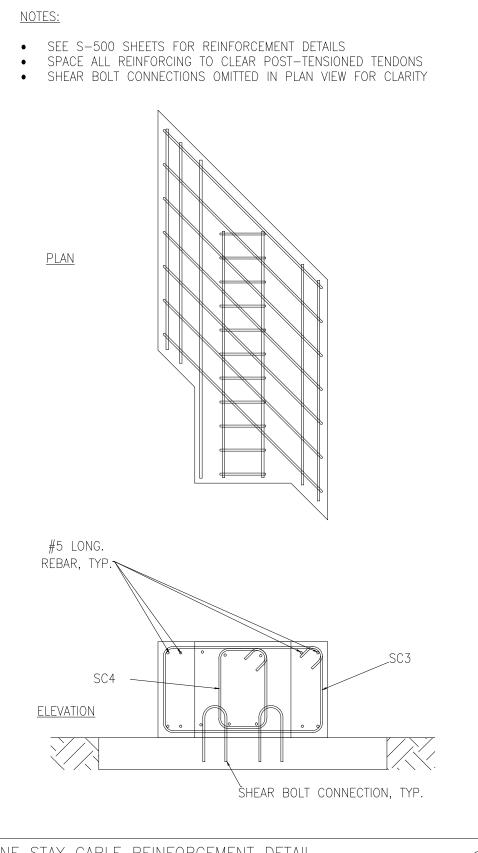
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S-101 STRUCTURAL NOTES







ENE STAY CABLE REINFORCEMENT DETAIL

Revision notes: Rev: Date:

Drawn by: Checked by: CJ JW NCHRP 14-28

CONDITION ASSESSMENT OF BRIDGE PT AND STAY CABLE SYSTEMS USING NDE METHODS

Project:

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TEXAS A&M TRANSPORTATION INSTITUTE 3135 TAMU COLLEGE STATION, TX 77843-3135

S-302 **STAY CABLE DETAILS** 

## CONSTRUCTION PHASE 4

TABLE OF ESTIMATED QUANTITIES				
BAR	SIZE	NO.	LENGTH (FT-IN)	TOTAL WEIGHT (LBS)
STRAIGHT	5	102	5 - 8	603
REINFORCING STEEL LBS 1644				1644
CLASS "C" CONCRETE CY				18.7

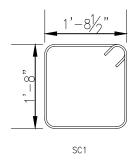
TABLE OF ESTIMATED QUANTITIES				
PIPE	NO.	CURVATURE RAD. (FT)	LENGTH (FT-IN)	TOTAL WEIGHT (LBS)
1	2	15	5 – 1	80
2	8	15	5 - ½	79

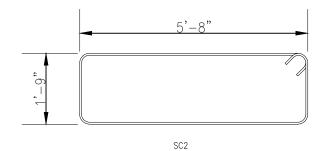
#### NOTES

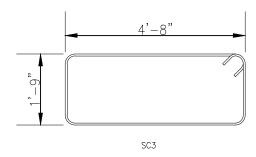
- ALL PIPES TO EXTEND 6" PAST DIAPHRAGM/DEVIATOR FACE
- (12) STEEL PIPES IN DIAPHRAGM TO BE MEASURED BASED ON TRUMPET SIZE

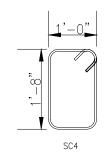
## STAY CABLE CONSTRUCTION

TABLE OF ESTIMATED QUANTITIES (INCLUDES ALL STAY CABLE SPECIMENS)				
BAR	SIZE	NO.	LENGTH (FT-IN)	TOTAL WEIGHT (LBS)
SC01	4	48	7 - 10	252
SC02	4	20	15 - 10	212
SC03	4	28	13 - 10	259
SC04	4	44	6 - 4	187
STRAIGHT	5	16	5 - 9	96
STRAIGHT	5	32	4 - 7	153
REINFORCING STEEL LBS			1159	
CLASS "C" CONCRETE CY 11.3			11.3	









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CONDITION ASSESSMENT OF BRIDGE PT AND STAY CABLE SYSTEMS USING NDE METHODS

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TEXAS A&M TRANSPORTATION INSTITUTE 3135 TAMU COLLEGE STATION, TX 77843-3135 Drawing Title:

S-504
REINFORCEMENT DETAILS

## TEXAS A&M TRANSPORTATION INSTITUTE

## TEXAS A&M UNIVERSITY

#### 1.0 GENERAL NOTES

THIS GROUTING PLAN IS PROVIDED PER PTI 4.6.1 PRIMARY GROUTING PERSONNEL: JOSHUA WHITE, ASBI GROUTING TRAINING CERTIFICATE CASEY JONES, ASBI GROUTING TRAINING CERTIFICATE KATLYN MCCOY

#### 2.0 GROUT SELECTION

TYPE C PREPACKAGED THIXOTROPIC GROUT TO BE USED, TO CONFORM TO PTI M55 TYPE C GROUT TO BE USED BECAUSE MAXIMUM HEAD IS KNOWN, NO VERTICAL TENDONS TO BE GROUTED

#### 3.0 DUCT/VENT PREPARATION

POSITIVE SHUT-OFF GROUT PORTS TO BE USED ALL DUCT COUPLERS WILL BE HEAT SHRINK SLEEVES ATTACHED AT CONNECTIONS AIR PRESSURE TESTING UP TO 30 PSI WILL BE PERFORMED ON ALL TENDONS PRIOR TO GROUTING ANY AIR LEAKS WILL BE SEALED USING EPOXY PRIOR TO GROUTING

NO FLUSHING OF DUCTS WITH WATER WILL BE DONE PRELIMINARY CLEANING OF DUCTS USING OIL-FREE AND WATER-FREE COMPRESSED AIR WILL BE DONE A MAXIMUM OF 48 HOURS PRIOR TO GROUTING GROUTING MUST BE DONE WITHIN 7 DAYS FROM INSTALLING THE TENDONS

#### 4.0 EQUIPMENT/MATERIALS/TESTING

COLLODIAL MIXER TO BE USED

INSPECTION OF ALL MATERIALS & EQUIPMENT TO BE DONE PRIOR TO EACH DAY OF

TYPE C THIXOTROPIC GROUT TO BE STORED ON SITE FOR NO MORE THAN ONE MONTH PRIOR TO USE

GROUT TO BE STORED IN LARGE CONTAINER FOR PROTECTION FROM SUN/MOISTURE ONLY POTABLE WATER WILL BE USED, CHLORIDE LEVELS TO BE CHECKED PRIOR TO EACH DAY OF GROUTING AND MUST MEET THE REQUIREMENTS OF ASTM C1602 WATER CONTENT OF GROUT MIXTURES MUST BE WITHIN 1% OF MANUFACTURER'S

THE FOLLOWING TESTS WILL BE PERFORMED ON THE GROUT:

WET DENSITY TEST (ANSI/API MUD BALANCE TEST)

REQUIRED EQUIPMENT:

1. MUD BALANCE SCALE

TEST FREQUENCY = EVERY NEW MIX OF GROUT

EXPECTED RESULT = 1.8-2.2

MODIFIED FLOW CONE (ASTM C939 MODIFIED)

REQUIRED EQUIPMENT:

- 1. STANDARD GROUT FLOW CONE
- 2. RECEIVING CONTAINER (MIN 2000 mL)
- 3. RING STAND FOR FLOW CONE
- 4. LEVEL

Revision notes

5. STOP WATCH

TEST FREQUENCY = EVERY NEW MIX OF GROUT EXPECTED RESULT = 7-30 SECONDS

WICK-INDUCED BLEED TEST (ASTM 940)

REQUIRED EQUIPMENT

- 1. TRANSPARENT TUBE (APPROX. 1000 mm TALL, NOMINAL DIAMETER OF 80
- 2. 1000 mm CONDITIONED, CLEANED SEVEN-WIRE STRAND
- 3. DUCT/ELECTRICAL TAPE

TEST FREQUENCY = 1/DAY

EXPECTED RESULT = 0.0% AT THREE HOURS

SCHUPACK PRESSURE BLEED TEST (ASTM C1741)

REQUIRED EQUIPMENT

- 1. GELMAN FILTER WITH STAINLESS STEEL SCREEN
- 2. TYPE A FIBERGLASS FILTER
- 3. AIR PRESSURE SUPPLY
- 4. 10 mL GRADUATED CYLINDER
- 5. TRANSPARENT TUBE

TEST FREQUENCY = 1/DAY

EXPECTED RESULT = MAX. 4% @ 20 PSI

GROUT STRENGTH

REQUIRED EQUIPMENT

- 1. SCALES, MASSES, SIEVES, GLASS GRADUATES, AND THREE GANG MOLDS
- FOR 2-IN CUBE SPECIMENS
- 2. PLATES TO COVER MOLDS
- 3. WEIGHT TO HOLD DOWN COVER PLATES

TEST FREQUENCY = 1/DAY

EXPECTED RESULT = MIN. 3 KSI @ 7 DAYS, MIN. 5 KSI @ 28 DAYS IF THE TEST RESULTS DO NOT COMPLY TO REQUIRED VALUES PER PTI M55 CH. 4, PROPERLY DISPOSE OF GROUT AND MIX AGAIN, RECHECK TEMPERATURES AND MEASUREMENTS, WATER SHALL NOT BE USED AT GREATER THAN 90 DEGREES F EACH SHIFT WILL CONSIST OF 4 WORKERS:

- 1: MIXING GROUT/RUNNING THE PUMP
- 2: ASSISTING WITH GROUT PUMP/MIXER
- 3: CLOSING VENT VALVES
- 4: RUNNING GROUT TESTS

A GROUT LOG WILL ALSO BE KEPT INCLUDING AT MINIMUM THE FOLLOWING INFORMATION:

- 1. DATE/TIME OF GROUTING
- 2. ELAPSED TIME
- 3. DAILY NOTES
- 4. DATA/RESULTS OF PORT CHECK
- 5. AMOUNT OF GROUT USED
- 6. TEMPERATURES OF WATER ADDED/GROUT MIXES
- 7. FINAL VOLUMES OF GROUT PUMPED
- 8. EFFLUX TIME
- 9. TENDON IDS
- 10. SIGNATURE OF OPERATOR

A PREGROUTING MEETING WILL OCCUR A MAXIMUM OF 48 HOURS PRIOR TO EACH GROUTING SESSION

#### 5.0 PUMPING

GROUT WILL BE MIXED IN COLLODIAL MIXER, PUMPED AT DEFINED PRESSURES. AND VALVE CLOSING SEQUENCES DEPENDS ON WHICH TENDON IS BEING GROUTED, AS SHOWN IN VALVE CLOSING SEQUENCE

MAXIMUM PRESSURES AT INLETS:

EXTERNAL HDPE DUCTS- 145 PSI

INTERNAL CIRCULAR DUCTS - 245 PSI

NORMAL OPERATIONS PRESSURE RANGE: 50-75 PSI

IF PUMP CANNOT STAY WITHIN THIS RANGE, STOP THE GROUTING OPERATION

ALL GROUTING WILL USE ONE-WAY FLOW, AS ILLUSTRATED BY VALVE CLOSING SEQUENCES A MINIMUM OF ONE GALLON OF GROUT WILL POUR OUT OF EACH VALVE PRIOR TO CLOSING AS IN RECOMMENDED PRACTICE

Project:

THIS GROUT SHALL BE DISCARDED, NOT REUSED

SEQUENCE OF CLOSING VALVES LISTED BELOW. REFER TO SECTION G6 FOR VALVE LABELING:

TENDON 1: 1C, 1B, 1A (1A INLET)

TENDON 2: 2C, 2B, 2A (2A INLET)

TENDON 3: 3C, 3B, 3D, 3A (3A INLET)

TENDON 4: 4C, 4B, 4D, 4A (4A INLET)

TENDON 5: 5C, 5B, 5D, 5A (5A INLET)

TENDON 6: 6B, 6A, 6C (6C INLET)

TENDON 7: 7B, 7A, 7C (7C INLET)

TENDON 8: 8B, 8A, 8C (8C INLET) TENDON 9: 9B, 9A, 9C (9C INLET)

TENDON 10: 10B, 10A, 10C (10C INLET)

TENDON 11: 10B, 10A, 10C (10C INLET)

TENDON 12: 11B, 11A, 11C (11C INLET)

TENDON 13: 13C. 13B. 13A (13A INLET)

TENDON 14: 14C, 14B, 14A (14A INLET) TENDON 15: 15A, 15B, 15C (15C INLET)

TENDON 16: 16A, 16B, 16C (16C INLET)

TENDON 17: 17D, 17A, 17B, 17C (17C INLET)

TENDON 18: 18D, 18A, 18B, 18C (18C INLET)

TENDON 19: 19D. 19A. 19B. 19C (19C INLET) TENDON 20: 20D, 20A, 20B, 20C (20C INLET)

TENDONS WILL BE GROUTED ONE AT A TIME DUE TO EQUIPMENT RESTRICTIONS EACH TENDON WILL REQUIRE APPROX. 42 GALLONS GROUT (INCLUDING TESTS) IF ANY BLOCKAGES, SYSTEM MALFUNCTIONS, OR MAJOR SPILLS OCCUR, IMMEDIATELY STOP THE GROUTING OPERATION

IDENTIFY THE PROBLEM, FIX THE ISSUE, AND TEST THE ENTIRE SYSTEM PRIOR TO RESTARTING THE GROUTING OPERATION

#### 6.0 GENERAL GROUTING PROCEDURE

- 1. GATHER ALL MATERIALS NECESSARY FOR GROUTING, CLEAN-UP, AND ANY POSSIBLE GROUT DISPOSAL
- 2. INSPECT ALL MATERIALS AND EQUIPMENT (MONITOR WATER TEMP.)
- 3. START THE MIXER
- 4. WEIGH OUT GROUT BAGS AND WATER, POUR INTO MIXER
- 5. MIX FOR 3-5 MINUTES 6. PERFORM PRE-PUMP WET DENSITY AND PUMPABILITY TESTS
- 7. IF GROUT MIX PASSES THE TESTS IN STEP 6, MAKE SURE ALL VENTS ARE OPEN AND START PUMP (DO NOT SHUT OFF MIXER)
- 8. CLOSE VENTS IN CORRECT ORDER
- 9. AS SOON AS POSSIBLE AFTER CLOSING THE LAST VENT, STOP THE PUMP AND SHUT OFF THE VALVE FROM THE HOSE
- 10. DISCONNECT THE HOSE FROM THE INLET
- 11. POUR EXCESS GROUT INTO GREASED TRASH CAN TO DRY
- 12. CLEAN-UP: RUN WATER ENTIRELY THROUGH THE SYSTEM UNTIL IT IS CLEAR, RUN IN HOPPER FOR EXTENDED TIME

#### 7.0 POST-GROUTING PROCEDURES

VALVES, CAPS, AND PIPES AT ALL INLETS/OUTLETS SHALL NOT BE REMOVED UNTIL THE GROUT IS SET

NOT LESS THAN 24 HOURS AFTER GROUTING, INSPECT LEVEL OF GROUT IN OUTLETS AND CAPS, TOP WITH FRESHLY MIXED GROUT AS NECESSARY

DO NOT SUBJECT THE FILLED DUCTS TO VIBRATION FOR A MIN. OF 24 HOURS INTERNAL TENDONS SHALL HAVE INLET AND OUTLET OPENINGS REMOVED AT LEAST 1 IN. BELOW THE CONCRETE SURFACE AND FILLED USING FRESHLY MIXED GROUT FOR EXTERNAL TENDONS, ALL OUTLET AND INLET OPENINGS STALL BE PERMANENTLY

REPAIR SPLITS. HOLES. OR OTHER DAMAGE TO EXPOSED DUCTS

Rev:	Date:	Notes:

Drawn by: Checked by: CJ JW Client: NCHRP 14-28

CONDITION ASSESSMENT OF BRIDGE PT AND STAY CABLE SYSTEMS USING NDE METHODS

5/31/2014 Scale N/A" Revision

TEXAS A&M TRANSPORTATION INSTITUTE 3135 TAMU COLLEGE STATION, TX 77843-3135

S-601 **GROUTING PLAN**