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Mathematical Models of Water Quality Parameters for Rivers and Estuaries

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MATHEMATICAL MODELS OF WATER QUALITY PARAMETERS
FOR RIVERS AND ESTUARIES

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FOREWARD

The development of computer models for mass transport in estuaries has been an important engineering activity for the past decade. Initial surveys conducted under this project showed however that only a limited amount of work had been done in modeling two dimensional characteristics in partially stratified estuaries.

As a result the major thrust of this project was directed in this important area. The project effort was coordinated with an existing project dealing with the Houston Ship Channel specifically EPA Research and Development grant 16090 DQW and considerably field data was developed by this EPA project. Additional support was received in the form of a Miles Cox fellowship which partially supported Dr. Young.

This final report presents the results of the literature survey model development and model calibration.

Project personnel over project time period have included Dr. Roy W. Hann, Jr., the current project director, Dr. Donald Schaezler, Dr. Robert Irvine, Dr. P. Jonathan Young, Mr. Richard Withers, Dr. Richard Allison and other staff members of the Estuarine Systems Projects Research group of The Environmental Engineering Division of Texas A&M Universities Civil Engineering Department.

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CHAPTER I

INTRODUCTION AND GOALS

Estuaries represent an economically and biologically important part of our coast. These bodies of water are used as permanent homes or nursery grounds for many of the important forms of marine life. Estuaries also provide protected transportation routes for cargo ships and barges. Because of their biological productivity and their economically advantageous locations, estuaries often attract industrial and commercial development along with associated dense populations.

Because of industrial and metropolitan development, estuaries are often expected to accept large amounts of waste materials. Many estuaries are large in size and give the appearance of being able to assimilate large volumes of this waste material. In actuality, estuaries may act as natural traps for pollutants. The fresh-water flow into some estuaries is relatively small. Once a pollutant enters such an estuary, the material is carried back and forth by the tides and makes only slow progress toward the ocean. These conditions are especially characteristic of Gulf

Coast estuaries. For example, recent studies (92) indicate that the average flushing time (i.e., the time necessary for the volume of fresh water inflow to equal the volume of the estuary) for the upper 14 miles of the Houston Ship Channel is 26 days and the average flushing time for Galveston Bay is 155 days.

In regard to salinity, estuaries vary between two extremes: the homogeneous estuary and the highly stratified. At each section of a homogeneous estuary, there is complete vertical mixing of the salt and fresh water. In a stratified estuary, there are two layers of water: the upper layer contains fresh water flowing toward the sea, and the lower layer contains salt water moving away from the sea. A sharp change in salinity occurs at the interface between the upper and lower layers. The Delaware Estuary is representative of a homogeneous estuary, whereas the Mississippi Delta represents a highly stratified estuary. Many estuaries fall somewhere between these two extremes and include some mixing and some stratification at each section; hence, they are more difficult to analyze than the idealized homogeneous or stratified cases. The Houston Ship Channel is an example of a partially stratified estuary. A comprehensive discussion of the characteristics of these types of estuaries is available in Ippen (51).

GOALS OF THIS STUDY

Although the past decade has produced an extensive amount of

mathematical modeling of estuaries, little effort has gone into modeling of the two dimensional characteristics of partially stratified estuaries. A major goal of this research was to develop computer models which could calculate vertical and horizontal mass transport in partially stratified estuaries. These models were to be applicable for predicting pollutant dispersion and dissolved oxygen distributions.

Two types of finite difference techniques provided the basis for these mathematical models: an explicit method and a Crank-Nicolson implicit method. The second major goal of this research was to compare the accuracy and usefulness of these two techniques. Discussions in literature of this type of comparison are scanty. Where these comparisons do exist for the two-dimensional mass transport equation (28), the equation has been simplified and applied to non-estuary problems with limited success.

A third major goal of this research was to summarize existing one- and two-dimensional mathematical models that have been applied to significant estuary problems.

The final major goal of this study was to demonstrate the applicability of these computer models to the mass transport characteristics of the Houston Ship Channel.

CHAPTER II

CHARACTERISTICS OF THE HOUSTON SHIP CHANNEL

The portion of the Houston Ship Channel being studied in this report is a tributary to upper Galveston Bay (see Figure 2-1). The Houston Ship Channel, shown in Figure 2-2, is a fairly narrow estuary which has been dredged to a depth of about 40 feet to allow the passage of ocean-going vessels. This estuary can be considered laterally homogeneous, especially in the upper 14 miles where there is no pronounced influence from side bays. The channel receives a large amount of industrial and municipal wastes from outfalls which discharge into the channel at many points.

Salinity measurements have been made on a regular basis on the channel since April 1968. Early in the sampling program, salinity was recorded at every ten feet of depth at five points laterally across the channel. These measurements were taken every four miles up the 24-mile long channel. Eventually, it was decided to sample only the centerline of the channel, since the salinity seemed not to vary significantly across the channel. At that time, the sampling grid was tightened to include measurements at every five feet vertically and two miles longitudinally (29).

As can be seen from the raw data and from several Estuarine Systems Projects reports (92, 109), the salinity structure of the

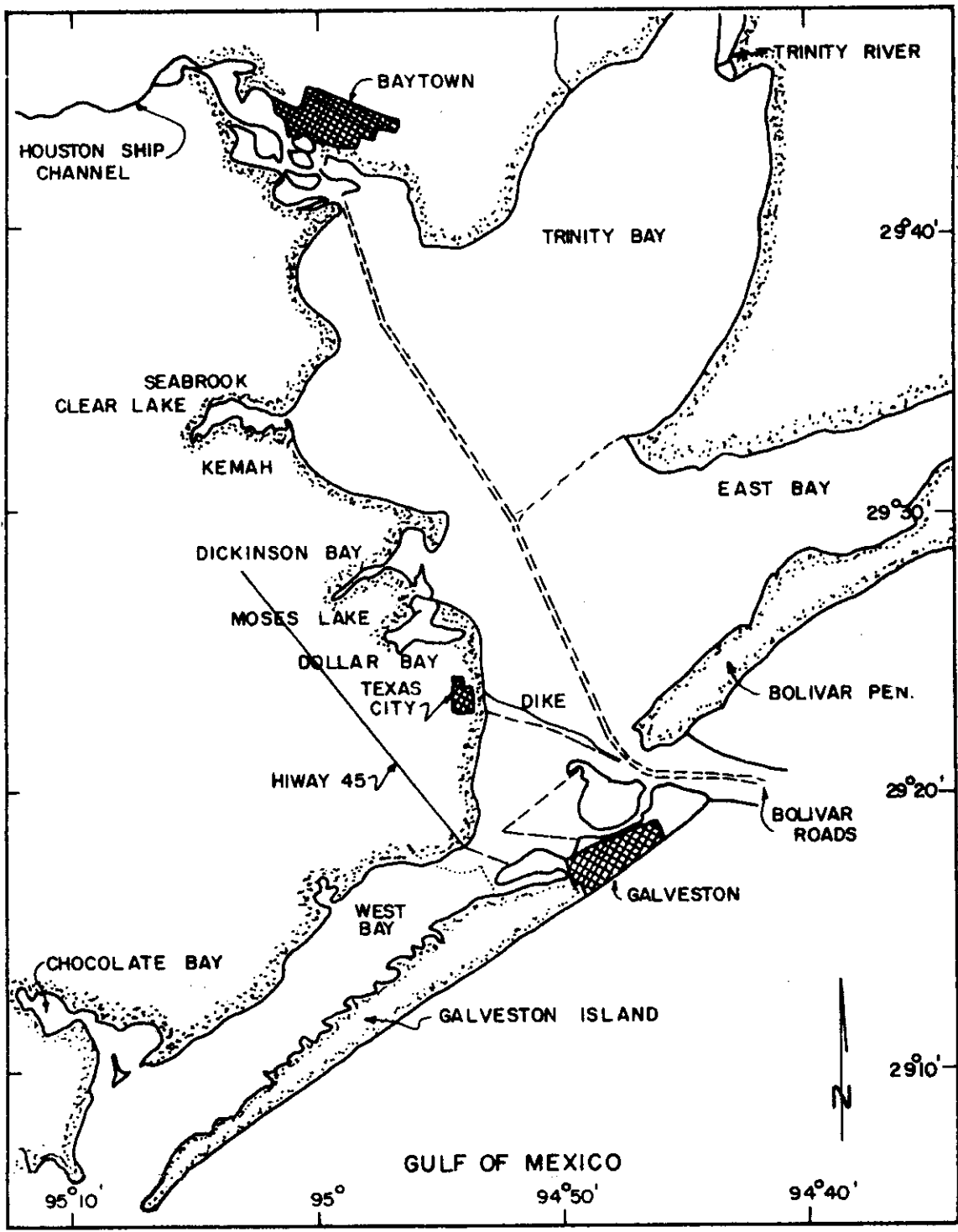


FIGURE 2-1.- GALVESTON BAY AREA

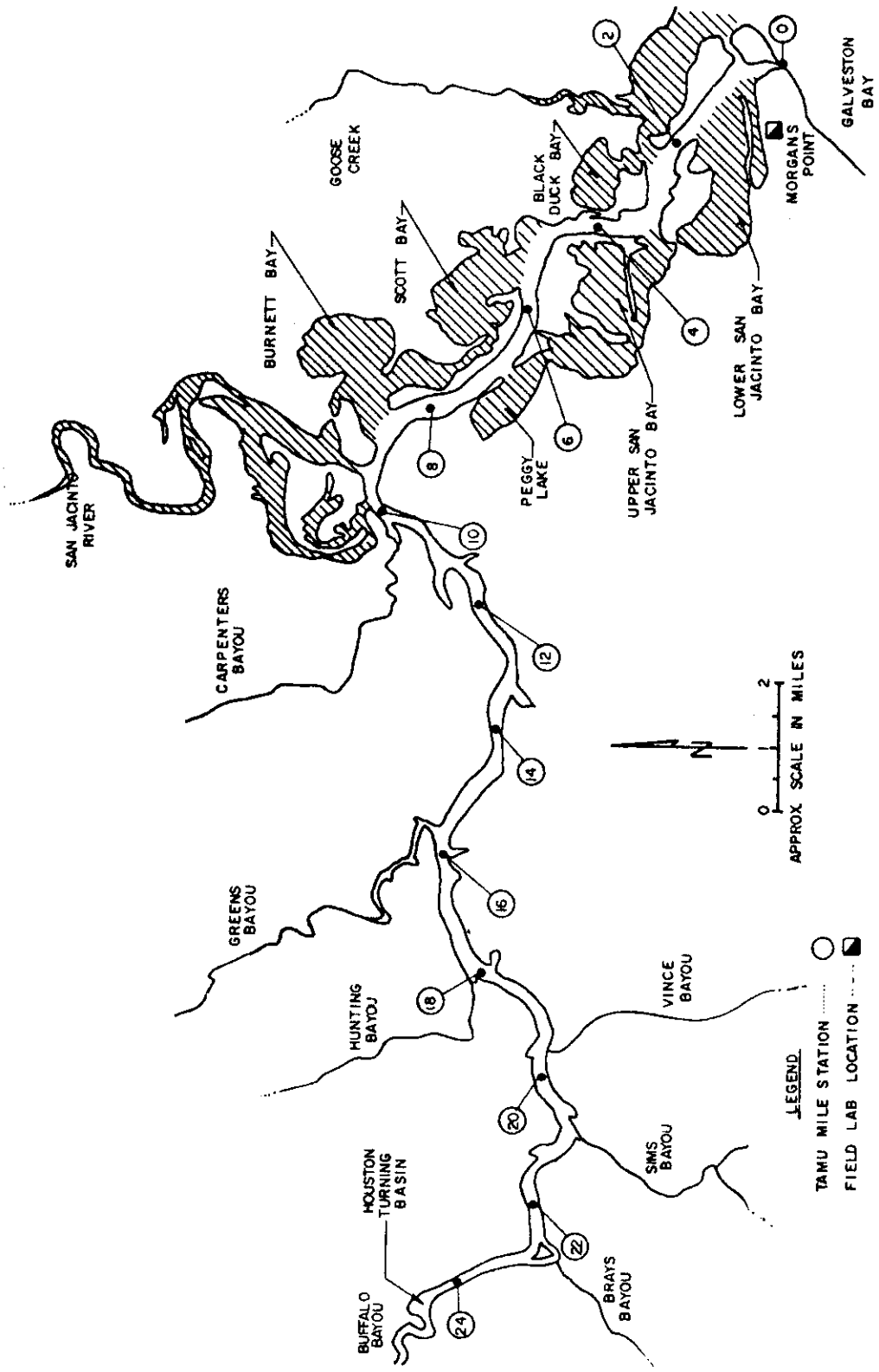


FIGURE 2-2. - HOUSTON SHIP CHANNEL

Houston Ship Channel is extremely variable. During some periods of heavy rainfall, the channel becomes highly stratified. At times of low inflow, the channel may become vertically well-mixed because of tidal agitation. However, the majority of the data indicates that the salinity structure is generally at some intermediate state of stratification, i.e., the salinity varies considerably from top to bottom with no sharp interface between fresh and saline water. Thus, the ship channel can best be classified as being "partially stratified." Calculations using typical values for inflow and tidal prism for the channel indicate that it usually falls within the "partially mixed" classification of previous estuary researchers (51, 107). The terms "partially mixed" and "partially stratified" are used synonymously in this report.

The channel can be adequately modeled by assuming a constant centerline depth and varying width; its behavior is essentially two-dimensional. However, several factors can be listed which keep the channel from being a simple, well-behaved hydraulic system:

1. The downstream end of the channel is a protected bay which is partially stratified and which has changeable salinity.
2. Part of the dredged channel is bordered by large, shallow side bays whose influence on flow patterns and dissolved oxygen patterns is difficult to determine.

3. A large tributary, the San Jacinto River, enters the channel about ten miles from the entrance to upper Galveston Bay and the flow of the river is erratic due to the operation of an upstream dam.
4. Tides in the 40-foot deep channel are erratic and of small amplitude, usually averaging about one foot in height.
5. Many chemical substances are added to the channel waters; their effects on flow patterns, salinity structure, and decay rates are difficult to determine.
6. Ship traffic on the channel has an undetermined effect on flow patterns and reaeration and interrupts the taking of measurements.
7. Benthic deposits and algae have a significant effect on water quality characteristics.

CHAPTER III
DEVELOPMENT OF THE ONE AND TWO DIMENSIONAL
MASS TRANSPORT EQUATIONS

Mass transport is governed by the principle of conservation of matter. For the case of materials dissolved in a fluid, conservation of matter must be satisfied for each constituent. For such a system, the conservation principle can be summarized as follows:

$$\left. \begin{array}{l} \text{TIME RATE OF} \\ \text{ACCUMULATION} \\ \text{OF CONSTITUENT} \\ \text{INSIDE A FLUID} \\ \text{ELEMENT} \end{array} \right\} = \left. \begin{array}{l} \text{INFLOW OF} \\ \text{CONSTITUENT} \\ \text{TO FLUID} \\ \text{ELEMENT} \end{array} \right\} - \left. \begin{array}{l} \text{OUTFLOW OF} \\ \text{CONSTITUENT} \\ \text{FROM FLUID} \\ \text{ELEMENT} \end{array} \right\} + \left. \begin{array}{l} \text{TIME RATE OF PRODUCTION} \\ \text{OF CONSTITUENT BY} \\ \text{CHEMICAL AND BIOLOGICAL} \\ \text{REACTION INSIDE THE} \\ \text{FLUID ELEMENT} \end{array} \right\}$$

This principle can be expressed by the equation

$$\frac{\partial \rho_A}{\partial t} = -\left(\frac{\partial N_x}{\partial x} + \frac{\partial N_y}{\partial y} + \frac{\partial N_z}{\partial z}\right) + r_A \dots \dots \dots (3-1)$$

or in the vector form

$$\frac{\partial \rho_A}{\partial t} = -\nabla \cdot \vec{N}_A + r_A \dots \dots \dots (3-2)$$

where ρ_A is the mass density of the mixture; \vec{N}_A is a mass flux of constituent A across a boundary and is expressed in the units mass / (Length)² (Time); N_x, N_y, N_z are the components of \vec{N}_A ; and r_A is the time rate of production of constituent A. This equation and its various forms are referred to by the names "equation of continuity for a constituent," "equation for conservation of a dissolved constituent," "mass transport equation," "mass balance equation," "diffusion equation," and the "convective dispersion equation."

The mass flux term, \vec{N}_A , represents transport by two phenomena: local fluid velocity and molecular diffusion:

$$\vec{N}_A = -\rho_A D_A \nabla C_A + \rho_A C_A \vec{V} \dots \dots \dots (3-3)$$

where D_A is the molecular diffusion coefficient for constituent A in the fluid, ρ_A is the mass density of the mixture; C_A is the concentration of constituent A; and \vec{V} is the velocity

vector. The negative diffusion term results from the tendency of a constituent in a mixture to move by molecular action in the direction of decreasing concentration of that constituent.

The velocities have both a time-averaged component, V_j , and a perturbation component, V_j' , which represents the random variations in velocity from its average value. The concentrations also have a perturbation component, C_A' . A combination of these terms produces the following form for the turbulent mass flux (14):

$$\text{Turbulent Mass Flux Per Unit Area} = \rho_A \overline{(V_j' C_A')} \dots \dots (3-4)$$

where a time average is denoted by a bar over the appropriate terms.

By analogy with Fick's first law of diffusion, it is often assumed that the turbulent flux is proportional to the gradient of the time-averaged concentration; for example, in the x-direction:

$$\rho_A \overline{(V_x' C_A')} = \rho_A E_x \frac{\partial \bar{C}_A}{\partial x} \dots \dots \dots (3-5)$$

where E_x is the turbulent diffusion coefficient.

If the isotropic molecular diffusion is assumed, these considerations lead to the following vector form of the mass transport equation for a single component, after the A subscripts are

dropped:

$$\frac{\partial C}{\partial t} + \nabla \cdot \vec{CV} = D(\nabla \cdot \nabla C) + \nabla \cdot (E_i \nabla C) + \frac{r}{\rho} \dots \dots \dots (3-6)$$

When continuity for an incompressible fluid is applied, a more useful form results:

$$\begin{aligned} & \frac{\partial C}{\partial t} + V_x \frac{\partial C}{\partial x} + V_y \frac{\partial C}{\partial y} + V_z \frac{\partial C}{\partial z} \\ & = \frac{\partial}{\partial x} \left(E_x \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial y} \left(E_y \frac{\partial C}{\partial y} \right) + \frac{\partial}{\partial z} \left(E_z \frac{\partial C}{\partial z} \right) \\ & + D \left(\frac{\partial^2 C}{\partial x^2} + \frac{\partial^2 C}{\partial y^2} + \frac{\partial^2 C}{\partial z^2} \right) + \frac{r}{\rho} \dots \dots \dots (3-7) \end{aligned}$$

The E_x , E_y , and E_z are the turbulent diffusion coefficients and can be only equal for the case of isotropic turbulence. In a natural system, the turbulent diffusion coefficients are many orders of magnitude larger than the molecular diffusion coefficients; hence it is acceptable to ignore completely the molecular diffusion terms. Of course, the terms on the left side of this equation can be more compactly expressed by the material derivative, DC/Dt .

Equation 3-6 can be simplified by dropping the molecular diffusion term, which is generally insignificant, and by eliminating the term $\frac{r}{\rho}$ by assuming that the material is conservative. This leaves the following form of the mass transport equation:

$$\frac{\partial C}{\partial t} = - \frac{\partial(V_x C)}{\partial x} - \frac{\partial(V_y C)}{\partial y} - \frac{\partial(V_z C)}{\partial z} + \frac{\partial}{\partial x} \left(E_x \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial y} \left(E_y \frac{\partial C}{\partial y} \right) + \frac{\partial}{\partial z} \left(E_z \frac{\partial C}{\partial z} \right) \dots (3-8)$$

Equation 3-8 is associated with the general three-dimensional equation of continuity for an incompressible fluid:

$$\frac{\partial V_x}{\partial x} + \frac{\partial V_y}{\partial y} + \frac{\partial V_z}{\partial z} = 0 \dots (3-9)$$

By expansion of a method suggested by Pritchard (78), Equations 3-8 and 3-9 can be treated to produce the two-dimensional and one-dimensional mass transport equations for a laterally homogeneous estuary. During this derivation, we assume that concentrations do not change with width and that the width does not change with time.

Assume that $a(x, z)$ is the left-hand boundary of the estuary and $b(x, z)$ is the right-hand boundary. By integrating Equation 3-8 between these boundaries we have

$$\int_a^b \frac{\partial C}{\partial t} dy = - \int_a^b \frac{\partial(V_x C)}{\partial x} dy - \int_a^b \frac{\partial(V_y C)}{\partial y} dy - \int_a^b \dots \text{etc.} \dots (3-10)$$

By extension of the Leibnitz rule (57), we can write

$$\int_a^b \frac{\partial(V_x C)}{\partial x} dy = \frac{\partial}{\partial x} \int_a^b (V_x C) dy + (V_x C)_a \frac{\partial a}{\partial x} - (V_x C)_b \frac{\partial b}{\partial x}$$

. (3-11)

and

$$\int_a^b \frac{\partial}{\partial x} \left(E_x \frac{\partial C}{\partial x} \right) dy = \frac{\partial}{\partial x} \int_a^b \left(E_x \frac{\partial C}{\partial x} \right) dy + \left(E_x \frac{\partial C}{\partial x} \right)_a \frac{\partial a}{\partial x} - \left(E_x \frac{\partial C}{\partial x} \right)_b \frac{\partial b}{\partial x}$$

. (3-12)

and similarly for the other terms. We can integrate the y terms directly:

$$\int_a^b \frac{\partial(V_y C)}{\partial y} dy = (V_y C)_b - (V_y C)_a \dots \dots \dots (3-13)$$

and

$$\int_a^b \frac{\partial}{\partial y} \left(E_y \frac{\partial C}{\partial y} \right) dy = \left(E_y \frac{\partial C}{\partial y} \right)_b - \left(E_y \frac{\partial C}{\partial y} \right)_a \dots \dots \dots (3-14)$$

Since there can be no mass transfer across the boundaries and since the boundaries do not change with time, the following equations result from analysis of the derivatives:

$$(V_y C)_b = (V_x C)_b \frac{\partial b}{\partial x} + (V_z C)_b \frac{\partial b}{\partial z} \dots \dots \dots (3-15)$$

$$(V_y C)_a = (V_x C)_a \frac{\partial a}{\partial x} + (V_z C)_a \frac{\partial a}{\partial z} \dots \dots \dots (3-16)$$

$$\left(E_y \frac{\partial C}{\partial y} \right)_b = \left(E_x \frac{\partial C}{\partial x} \right)_b \frac{\partial b}{\partial x} + \left(E_z \frac{\partial C}{\partial z} \right)_b \frac{\partial b}{\partial z} \dots \dots \dots (3-17)$$

$$\left(E_y \frac{\partial C}{\partial y} \right)_a = \left(E_x \frac{\partial C}{\partial x} \right)_a \frac{\partial a}{\partial x} + \left(E_z \frac{\partial C}{\partial z} \right)_a \frac{\partial a}{\partial z} \dots \dots \dots (3-18)$$

If equations 3-11 through 3-18 are substituted into Equation 3-10, we are left with

$$\begin{aligned} \frac{\partial}{\partial t} \int_a^b C dy = & - \frac{\partial}{\partial x} \int_a^b (V_x C) dy - \frac{\partial}{\partial z} \int_a^b (V_z C) dy \\ & + \frac{\partial}{\partial x} \int_a^b \left(E_x \frac{\partial C}{\partial x} \right) dy + \frac{\partial}{\partial z} \int_a^b \left(E_z \frac{\partial C}{\partial z} \right) dy \end{aligned} \dots \dots \dots (3-19)$$

Since we have assumed lateral homogeneity, this equation reduces to

$$\begin{aligned} \frac{\partial(WC)}{\partial t} = & - \frac{\partial(WV_x C)}{\partial x} - \frac{\partial(WV_z C)}{\partial z} \\ & + \frac{\partial}{\partial x} \left(E_x W \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial z} \left(E_z W \frac{\partial C}{\partial z} \right) \dots \dots \dots (3-20) \end{aligned}$$

where $W = b - a$, which is the estuary width.

If we apply the Leibnitz rule and the same boundary conditions to Equation 3-9, we find that the two-dimensional continuity equation with regard to width becomes

$$\frac{\partial(WV_x)}{\partial x} + \frac{\partial(WV_z)}{\partial z} = 0 \dots \dots \dots (3-21)$$

Combining Equation 3-20 and Equation 3-21 gives the two-dimensional mass transport equation which is used most frequently in this study:

$$W \frac{\partial C}{\partial t} = -WV_x \frac{\partial C}{\partial x} - WV_z \frac{\partial C}{\partial z} + \frac{\partial}{\partial x} \left(E_x W \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial z} \left(E_z W \frac{\partial C}{\partial z} \right) \dots \dots \dots (3-22)$$

If we wish to derive the one-dimensional mass transport equation, we can apply the same type of analysis to Equation 3-8 using the cross-sectional area, A_x ; the following equation results:

$$A_x \frac{\partial C}{\partial t} = -A_x V_x \frac{\partial C}{\partial x} + \frac{\partial}{\partial x} \left(E_x A_x \right) \frac{\partial C}{\partial x} \dots \dots \dots (3-23)$$

Appropriate source and sink terms can be added to Equations 3-22 and 3-23 for the various substances being transported.

For an estuary of constant depth, the cross-sectional area term can be replaced by a width term, W_x :

$$W_x \frac{\partial C}{\partial t} = -W_x V_x \frac{\partial C}{\partial x} + \frac{\partial}{\partial x} \left(E_x W_x \right) \frac{\partial C}{\partial x} \dots \dots \dots (3-24)$$

CHAPTER IV

RECENT MATHEMATICAL MODELS OF ESTUARIES

Because of increased concern over the degradation of estuaries, advanced techniques are needed to analyze the behavior of pollutants in these bodies of water. When developing a plan for controlling the quality of a body of water, one of the crucial problems is the formulation of a model which accurately simulates the behavior of the system. The most significant work relating to the mathematical modeling of estuaries has been done in the last decade. In the early 1960's, researchers concentrated on applying closed-form, analytical equations to estuaries. As experience has been gained, more complex methods have been developed with a heavier reliance on digital computers. The most recent models generally require a high degree of mathematical sophistication including numerical solution on a computer of complex systems of partial differential equations. Once a simulation model has been developed and verified, techniques of operations research can often be applied to determine the minimum cost solution to the water quality problems.

Most analyses dealing with water quality problems are based on some form of the mass transport equation. Some manipulation of terms in this equation is necessary for constituents which interact, such as biochemical oxygen demand (BOD) and dissolved

oxygen. For parameters which undergo progressive changes, such as in the nitrogen cycle, the mass transport equation may have to be used several times to describe the system adequately. When a steady-state condition in a river is assumed, the diffusion and dispersion terms can be dropped and formulations such as the classical Streeter-Phelps equation (94) can be derived directly.

Determining the success of any model in adequately representing a system is highly dependent upon the amount of data available from that system. Assuming a model has been formulated correctly, the accuracy of the model then depends on choosing the correct values for reaeration, decay, dispersion, salinity gradients, velocity of flow, volume of pollution, and various factors related to tidal action and currents. Attempts to use a model to predict water quality should be made only after that model has been shown to fit adequately a large amount of existing data.

Estuary modeling of particular interest has been done by many American researchers. Important work also has been done in foreign countries; for instance, the Water Pollution Research Laboratory of Great Britain has made a landmark study of the Thames (104) and the Delft Hydraulics Laboratory makes regular studies on estuaries in The Netherlands (35). The state of current research for estuary mathematical models can best be indicated by a brief review of the work of some of these research efforts.

MODELING METHODS OF DONALD J. O'CONNOR

O'Connor is the best known researcher in the field of mathematical modeling of estuaries. His 1960 paper (65) entitled, "Oxygen Balance of an Estuary" was the first comprehensive report on modeling of the behavior of non-conservative substances in these bodies of water. During the past decade, O'Connor has written many additional papers (66, 67, 68, 69) dealing with the modeling of various estuaries.

O'Connor's approach to the modeling of estuaries is generally characterized by steady-state analytical solutions to a one-dimensional form of the estuary mass transport equation. Two of the most useful of these equations will be discussed later in this chapter as Equations 4-5 and 4-6. O'Connor's use of analytical equations is most versatile when several of these equations are used simultaneously. This approach was applied to the East River, New York, where the system was divided into segments, each with its appropriate general solution. The concentration profile for the entire profile was then determined by solving a set of equations which matched at the segment boundaries (68). This approach requires that the variation in cross-sectional area of segments of the estuary be expressed as well-behaved functions of distance. O'Connor often uses expressions for cross-sectional area which are linear, monomial,

or exponential functions of distance. The dispersion term E_x is often used to include the influence of tides as well as eddy diffusion.

O'Connor's methods have been used to obtain steady-state concentration profiles for many estuaries including the New York Harbor, East River, Delaware River, James River, and Raritan River. Time-varying equations also have been developed and applied.

MODELING METHODS OF ROBERT V. THOMANN

A widely used mathematical model for the Delaware Estuary is primarily the work of Thomann (97, 98, 99, 100, 101, 102, 75). Thomann's initial formulation of the model was presented in his doctoral dissertation, "The Use of Systems Analysis to Describe the Time Variation of Dissolved Oxygen in a Tidal Stream" (97). Thomann later became the technical director of the Delaware Estuary Comprehensive Study (DECS) under the Public Health Service and Federal Water Pollution Control Administration (FWPCA); for several years, a large amount of data was collected by DECS and the model was improved. In July, 1966, the FWPCA published a report (24) which recorded the status of the model and the recommendations of that government agency for the

uses of the river. Since that time, the Thomann model has been applied to the Potomac (42) and other estuaries.

A numerical solution to the basic equations in the model was obtained by the DECS from the Re-Entry Systems Department of the General Electric Company. The work at General Electric was supervised by Jeglic (54, 55). A recent digital computer program, called DECS III, is a refined, time-dependent version of the original model. It has been programmed in several versions of FORTRAN IV and can be used by many of the larger digital computers. A steady-state version of the Thomann model was been documented by Bunce and Hetling (9).

The basic form of the Thomann equation for BOD is as follows:

$$\begin{aligned} \frac{dL_k}{dt} = & \frac{Q_{k-1, k}}{VOL_k} \left[\epsilon_{k-1, k} L_{k-1} + (1 - \epsilon_{k-1, k}) L_k \right] \\ & - \frac{Q_{k, k+1}}{VOL_k} \left[\epsilon_{k, k+1} L_k + (1 - \epsilon_{k, k+1}) L_{k+1} \right] \\ & + \frac{E'_{k-1, k}}{VOL_k} (L_{k-1} - L_k) + \frac{E'_{k, k+1}}{VOL_k} (L_{k+1} - L_k) \\ & - d_k L_k \pm J_k \dots \dots \dots (4-1) \end{aligned}$$

In this equation, k ranges from 1 to n , where n = the number of sections, $k-1$ = the upstream section, $k+1$ = the downstream section, L = the ultimate carbonaceous biochemical oxygen demand concentration, Q = the net flow from section to section, VOL = the volume

of the section, ξ = an advective coefficient dependent upon the ratio of the dispersion to the advective forces, E' = an eddy exchange coefficient analogous to the classical eddy diffusion coefficient in non-tidal streams, d = the decay rate of biochemical oxygen demand, and J = the direct sources of biochemical oxygen demand.

It is obvious from this equation that Thomann attempts to separate some of the effects of tides from the eddy exchange coefficient, E' . It should be noted that the dimensions of E' must be L^3/T to make the equation dimensionally correct. Thus, Thomann's E' can not be considered equal to the dispersion or diffusion coefficients (D or E) of other investigations.

The advective coefficient, ξ , varies between a value of 0.5 for an environment dominated almost entirely by tides to a value of 1.0 for a non-tidal stream. For situations between these extremes, ξ is assumed to be some function of Q and E' . In some applications, ξ has been assumed equal to either 0.5 (99) or to the ratio $l_k / (l_k + l_{k-1})$ where l is the length of the appropriate segment (9). If ξ is equal to 0.5, the Thomann equation becomes a one-dimensional finite-difference form of the mass transport equation when E' is set equal to $EA/\Delta x$, where A is the cross-sectional area. A discussion of the problems associated with evaluating the various parameters in the Thomann equation is found in the proceedings of the Stanford ASCE Symposium on Estuarine Pollution (75, 76).

Recently, Hays (38, 39) has applied the steady-state version ($dL_k/dt = 0$) of the Thomann model to the Houston Ship Channel. Hays applied non-linear programming techniques to this one-dimensional model and obtained least-cost solutions for producing certain dissolved oxygen profiles in the channel.

MODELING METHOD OF WATER RESOURCES ENGINEERS, INC.

The mathematical model most often applied to the San Francisco Bay and Delta region was developed for the FWPCA (now the Environmental Protection Agency, EPA) by Water Resources Engineers, Inc., under the supervision of Orlob. The first stage of this model was formulated in 1965 to represent the water quality in the Sacramento-San Joaquin Delta (90, 105). Verification of this phase of the model was achieved by comparison with past salinity and hydrological conditions and with the results of a prototype dye tracer study. During the following year, 1966, the model was extended to Suisun and San Pablo Bay (72, 106). The model in this form was the major tool used in predicting the effects of the proposed San Joaquin master drain (25) on the future water quality of the estuary.

The model uses finite-difference approximations to simplified, one-dimensional forms of the equation of motion, the equation of continuity, and the mass transport equation for conservative materials (106):

$$\frac{\partial V_x}{\partial t} + V_x \frac{\partial V_x}{\partial x} + g \frac{\partial H}{\partial x} + K |V_x| V_x = 0 \quad \dots \dots \dots (4-2)$$

$$\frac{\partial H}{\partial t} + \frac{\partial Q}{\partial x} = 0 \quad \dots \dots \dots (4-3)$$

$$\frac{\partial C}{\partial t} = E_x \frac{\partial^2 C}{\partial x^2} - V_x \frac{\partial C}{\partial x} \quad \dots \dots \dots (4-4)$$

where K represents a frictional resistance term; H is the tidal height; Q is the flow rate; V_x is the velocity; and the remaining terms are defined as in previous sections of this report. These one-dimensional equations were applied to a two-dimensional grid representing the Bay-Delta watercourse. The model used a network of channels which are linked at nodes to describe the area covered by the Delta, Suisun Bay, and San Pablo Bay. This grid system was a major distinction between the San Francisco Bay model and the Delaware Estuary model, which simply divided the Delaware into a series of reaches.

Water Resources Engineers chose to employ an explicit scheme in solving the above equations because of the simplicity of this type of formulation and because the step size that can be used is relatively independent of the size of the system (106). The water quality model uses intra-tidal velocities, and it was found that

the effects of advective transport were much more important than eddy dispersion. Thus, the dispersion term could be ignored and only the velocity term was used in the refined version of the conservative mass transport equation. Some problems developed because of the "numerical mixing" error which was a consequence of the numerical methods used to integrate the mass transport equation and the application of one-dimensional equations to a two-dimensional system. Techniques have been developed to circumvent these problems by applying a weighted-average factor to the concentrations, but generally either accuracy or stability of the solution must be sacrificed to a certain degree (72).

The Water Resources Engineers' model is operated as follows: The hydrodynamic model is operated for several equal tidal cycles until stable intra-tidal velocities are obtained; normally these velocities are obtained for 30-minute increments (72). These velocities are fed into the water quality model which is run for a large number of equal tidal cycles until a stable equilibrium pattern is developed. The rate of convergence of the quality model is somewhat dependent upon the initial concentrations used; thirty tidal cycles is a representative number of repetitions needed before equilibrium is obtained.

In the use of this model by the FWPCA, primary emphasis was placed on total dissolved solids and total nitrogen, where nitrogen was considered as a conservative element. The principal characteristics of the present nitrogen and salinity distributions in the

Bay-Delta have been reproduced by the model with a satisfactory degree of accuracy. Non-conservative substances can be directly represented by the model and several preliminary studies have been conducted which illustrate the capability of the model to handle BOD and DO distributions. However, the model is set up on the assumption of complete mixing at a cross-section and, therefore, cannot handle a vertically stratified body of water.

This model has been applied to other bodies of water, including the San Diego Bay (26) and the Columbia River estuary (10). Recent documentation of the model by Feigner and Harris (26) show the following characteristics: The hydrodynamic model is typically run for 1 to 4 tidal cycles with a resulting execution time of 7 to 23 minutes on a CDC 6600 computer. The water quality model is typically run for 20 to 56 tidal cycles with a resulting execution time of 3 to 14 minutes on the CDC 6600. Execution time for an IBM 360/65 computer was found to be 2 to 3 times greater than those reported for the CDC 6600. In the above applications of the model, the number of junctions ranged between 112 and 830, and the number of channels ranged between 170 and 1050.

MODELING METHOD OF HANN AND EVETT

A mathematical model developed at Texas A&M University under the supervision of Hann has been applied to several Gulf Coast

estuaries including the San Bernard, the Neches, and the Houston Ship Channel. The primary reference for this model is a doctoral dissertation written by Evett (23). A revised edition of this model has been written in a problem-oriented language (POL) format (30).

In comparison with all of the other models discussed thus far, Hann's approach is unique in that it can be applied to partially stratified estuaries. This model uses the following two solutions to a one-dimensional form of the mass transport equation:

$$C = C_0 e^{-K_d x/U} \dots \dots \dots (4-5)$$

and

$$C = C_0 e^{jx} \dots \dots \dots (4-6)$$

where

$$j = \frac{U}{2E} \left(1 \pm \sqrt{1 + \frac{4EK_d}{U^2}} \right)$$

For these equations, E = the dispersion coefficient, C = the waste concentration, C₀ = the concentration at the outfall, x = the horizontal distance, U = the advective velocity, and K_d = the removal coefficient.

Equation 4-6 represents concentration of a non-conservative

pollutant in a vertically mixed (homogeneous) estuary where the optional minus sign is used for the downstream direction. Equation 4-5 represents the concentration below an outfall in a river with no horizontal dispersion. Equation 4-5 is used in the upper, fresh-water layer of a stratified estuary where it is assumed that diffusion is insignificant in comparison to advective transport. Allowances also are made to account for salt water flow upward from the saline wedge. The development of both the homogeneous and stratified cases assume a steady-state condition.

The methods of modeling a homogeneous estuary or a highly stratified estuary proceed in essentially the same manner. Initially, the estuary is divided into segments. The lengths of these segments can vary; usually one-mile segments are most convenient. A calculation is then made using Equations 4-5 and 4-6 to determine the concentration that would appear in each segment under steady-state conditions as a result of injecting a unit waste load of one pound per day into a particular segment. After this calculation is repeated for each segment, a matrix of "unit loading coefficients" results. The appropriate unit loading coefficients can then be multiplied by the actual waste load being put into each segment and the waste distribution in the estuary is obtained.

The unit loading coefficients are adjusted for use in a channel with varying area by the following technique. Assume that a unit load (one pound of BOD per day) is introduced into segment 5 and a concentration of C_5 results. The quality in the downstream

segment 6 is then calculated assuming that it has a volume equal to segment 5. The calculation is made using Equation 4-6 for a homogeneous estuary and Equation 4-5 for the top layer of a highly stratified estuary. The next step is to modify the quality parameter in segment 6 to reflect the changed volume by means of the following formula:

$$C_{6, \text{ modified}} = \frac{VOL_5}{VOL_6} C_6 \dots \dots \dots (4-7)$$

where VOL_5 and VOL_6 equal the volumes of segments 5 and 6, respectively. This procedure is then repeated using the modified C_6 value to calculate C_7 and so on down the estuary. Similar calculations are made in the upstream direction in the case of a homogeneous estuary.

The procedure used by Hann and Evett to analyze a partially mixed estuary is based on the assumption that the waste concentration in each segment will fall at some value between that for a highly stratified estuary and that for a homogeneous estuary. The actual value will depend upon the amount of stratification existing in the estuary. For each segment, a calculation is made to determine a "degree of stratification", DS. The calculation of this term requires the following segment input parameters: average top salinity, A; average bottom salinity, B; and salinity of the ocean or bay outside the estuary, C. The "degree of stratification" is then calculated by the following formula:

$$DS = \frac{B-A}{C-A} \dots \dots \dots (4-8)$$

Thus, to determine the waste pattern in a partially stratified estuary, a calculation is made for a homogeneous estuary and for a completely stratified estuary, and then each calculation is weighted in accordance with the above "degree of stratification." Evett (23) applied linear programming techniques to this model to determine the maximum waste discharges which could be applied to the channel while satisfying certain dissolved oxygen criteria.

ANALYTICAL METHODS OF HARLEMAN AND IPPEN

Harleman and Ippen have produced some of the most significant American research in relating experimental findings to the behavior of actual estuaries. Their work is summarized briefly in this section to demonstrate an alternative approach to the numerical analysis techniques employed by other researchers. Likewise, the experiments of Harleman and Ippen give significant insight toward determining some of the important input parameters for mass transport models.

Most of the estuary studies performed by these investigators since 1960 were initiated by the U. S. Army Corps of Engineers Committee on Tidal Hydraulics. The experimental data were obtained in the rectangular salinity flume of the Waterways Experiment Station at Vicksburg and in a smaller flume at the Hydrodynamics

Laboratory of the Massachusetts Institute of Technology. The scope of these investigations covered the following four phases and were reported in the indicated references:

1. The extent of salinity intrusion and the mean salinity distribution in an idealized estuary (50, 51);
2. The vertical mixing of fresh and salt water and the resulting vertical salinity distribution (36);
3. The vertical distribution of current velocities as affected by salinity distribution (36); and
4. The movement and deposition of sediments as affected by the density current phenomena (37).

The estuary research of Harleman and Ippen has led to a number of unique parameters which they feel are important in analyzing estuaries. Much of their work has been presented in a non-dimensional format so that results can be applied to estuaries of widely varying dimensions and behavior.

Most of the work done by Harleman and Ippen has been correlated with a parameter known as the "stratification number." The use of this parameter was concisely explained in these authors' analysis of the Rotterdam waterway (35): "The stratification number, G/J , is defined as the following ratio.

$$\frac{G}{J} = \frac{\text{rate of energy dissipation per unit mass of fluid}}{\text{rate of gain of potential energy per unit mass of fluid}}$$

..... (4-9)

The numerator was evaluated from an analysis of the damping of the tidal wave in the channel. The denominator reflects the gain in potential energy due to increasing specific weight as the water becomes saline in moving down the estuary to the ocean. A large value of stratification number (of the order of 100 or more) indicates a well mixed estuary condition. Lower values indicate an increasing tendency toward stratification with a two-layer saline wedge as a limiting case. Although the stratification number was a useful parameter in correlating the Vicksburg Waterway Experiment Station (W.E.S.) channel tests, it is inconvenient to evaluate in actual estuaries. The difficulty is primarily due to the necessity of evaluating the rate of energy dissipation from tidal data."

During a reanalysis of the W.E.S. data it was found that a dimensionless parameter containing the tidal prism, Froude number (based on the maximum tidal velocity), fresh water discharge, and the tidal period was uniquely related to the stratification number. For convenience in later discussion this combination will be called "estuary number." In comparison with the stratification number, the estuary number is relatively easy to evaluate in actual estuaries. The estuary number is defined as follows:

$$\text{estuary number} = \frac{P_t F_o^2}{Q_f T} \dots \dots \dots (4-10)$$

where P_t = tidal prism, the volume of sea water entering the

estuary on the flood tide, $F_o = \text{Froude number} = u_o/\sqrt{gh}$, u_o is the maximum flood tide velocity at $x = 0$, and h is the mean depth at $x = 0$, $Q_f = \text{fresh water discharge}$, and $T = \text{tidal period}$. The relation between the stratification number and the estuary number is shown in Figure 4-1 for the W.E.S. tests.

In their one-dimensional treatment of estuary analysis, Harleman and Ippen use what they term an "apparent diffusion coefficient D'_x " (50, 35, 51). Through this term, they attempt to take into consideration the effects of large scale internal circulation estuaries and the net effects of vertical transport. The ratio of D'_x to the actual longitudinal coefficient of turbulent diffusion (D_t) approaches unity for the well-mixed estuaries and approaches an order of 1000 for the highly stratified (51). The D'_x/D_t ratio ranged between 20 and 180 for the W.E.S. studies (50).

To solve the one-dimensional form of the mass transport equation, it is necessary to express the diffusion coefficient in terms of x . Harleman and Ippen assumed that the apparent diffusion coefficient decreased linearly with x in the upland direction. This approach seemed to work well for a salinity flume with a constant cross-section. Since salinity at low water slack cannot equal "ocean" salinity, the variation of D'_x was extended seaward a distance B where ocean salinity was assumed to exist. The equation for the apparent diffusion coefficient becomes:

$$D'_x = \frac{D'_o B}{x + B} \dots \dots \dots (4-11)$$

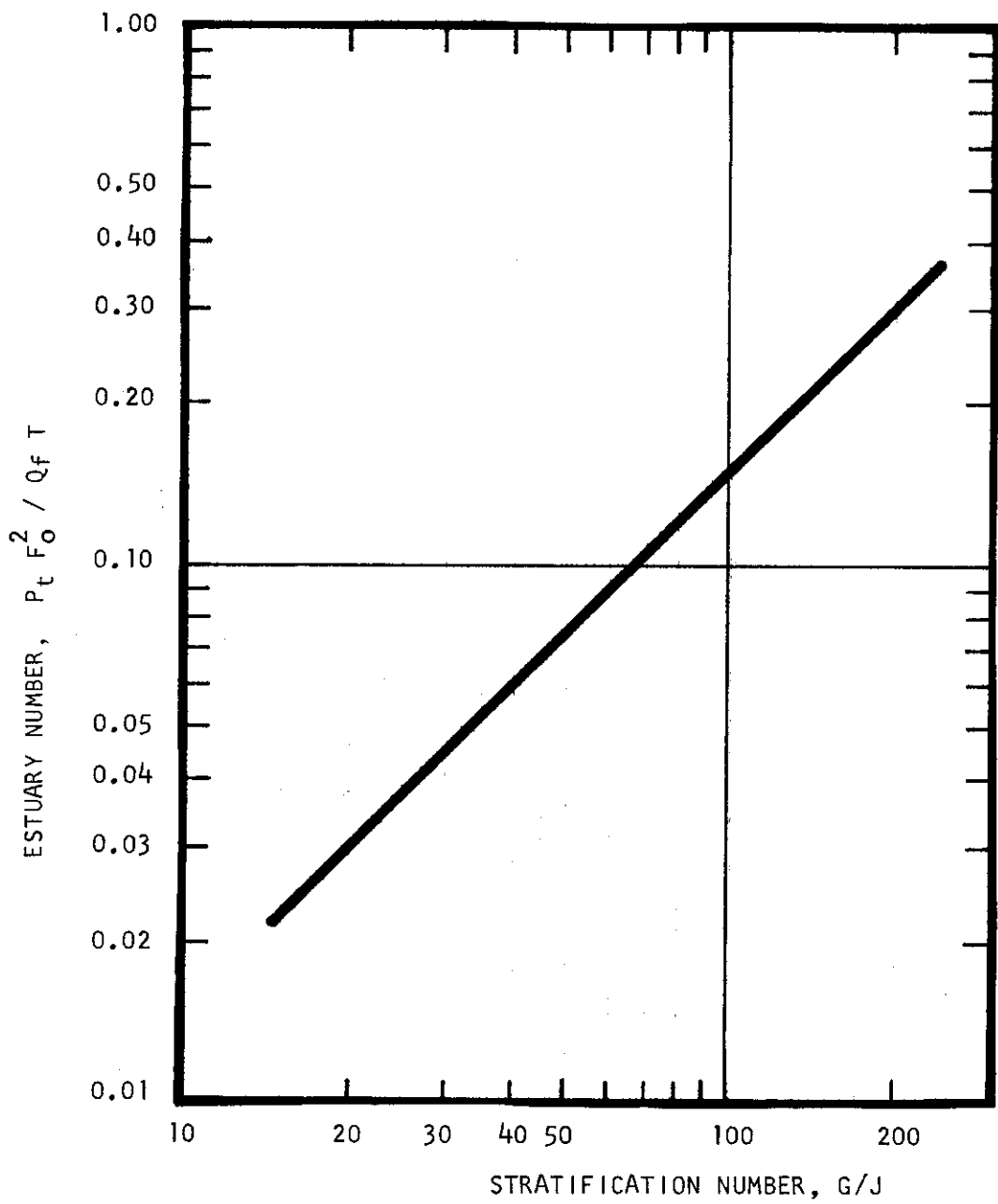


FIGURE 4 - 1. - RELATIONSHIP BETWEEN STRATIFICATION NUMBER AND ESTUARY NUMBER

where $D'_x = D'_0$ at $x = 0$, the mouth of the channel. Thus, the governing equation for salinity in the one-dimensional analysis becomes:

$$\frac{\bar{s}_{lws}}{s_0} = \exp \left(- V_f (x + B)^2 / 2 D'_0 B \right) \dots \dots \dots (4-12)$$

where \bar{s}_{lws} = local salinity at low water slack, s = ocean salinity, and V_f is the velocity due to fresh water discharge. If salinity is known at low water slack for at least two points, then the parameters D'_0 and B can be determined.

Other parameters which Harleman and Ippen feel are important in one-dimensional analysis are $D'_0 / V_f B$, $2 B / u_0 T$, and $Q_f T / P_t$; all the parameters in these ratios have been defined previously. Analysis using these parameters has been applied to the Rotterdam Waterway by Harleman and Abraham (35). Several characteristics of the waterway were defined; unfortunately, most of the Vicksburg flume data falls outside the range of the waterway data.

Most recent studies of the two-dimensional behavior of estuaries have been performed by Harleman and Ippen to analyze vertical transport (36) and shoaling (37). A large amount of information was collected on tidal time-averaged values of vertical velocity, horizontal velocity, salinity profiles, and vertical dispersion in an idealized estuary of constant cross section. Vertical velocities were found to be in the downward

direction in the seaward half of the intrusion region and upward in the landward half (36). This finding is in contradiction to some other researchers who assume that a net vertical motion from bottom to surface exists throughout the intrusion length. In addition, certain parameters were found which proved to be useful in the qualitative analysis of estuary shoaling.

TWO-DIMENSIONAL MODELS OF GALVESTON BAY

Several studies of Galveston Bay, Texas, have been made recently to evaluate the bay's hydrodynamic and mass transport characteristics. Reid and Bodine modeled the behavior of the bay for storm surge conditions (84). Masch and Shankar extended this work to include mass transport considerations (61, 62, 88, 89). The Galveston Bay Study, through the consulting firm Tracor, Inc., is applying similar models to Galveston Bay (21, 22). In addition, the Texas Water Development Board is extending the use of the Masch/Shankar models to other estuaries along the Texas coast (96).

The work of Reid and Bodine is based on a vertically integrated form of the equations of motion and continuity. These equations allow for rainfall, wind stress, and quadratic bottom friction but ignore terms dealing with momentum advection and the Coriolis force:

$$\frac{\partial U}{\partial t} + gZ \frac{\partial H}{\partial x} = KW^2 \cos \psi - fQUZ^{-2} \dots \dots \dots (4-13)$$

$$\frac{\partial V}{\partial t} + gZ \frac{\partial H}{\partial y} = Kw^2 \sin \psi - fQVZ^{-2} \dots \dots \dots (4-14)$$

$$\frac{\partial H}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = R \dots \dots \dots (4-15)$$

where U and V are the vertically integrated x and y horizontal components, respectively, of transport per unit width; H is the water level elevation relative to the local mean sea level datum; Z is the depth of water at position x, y, at time t; Q is equal to the vector average of transport per unit width and is obtained from the positive root of the radical $Q = \sqrt{U^2 + V^2}$; R equals the rainfall rate; f is a dimensionless bed resistance coefficient; W is the wind speed 10 meters above the water; ψ is the angle between the wind velocity vector and the x-axis; K is the dimensionless Van Dorn coefficient for wind stress and is considered to be a function of W. Reid and Bodine solve Equations 4-13, 4-14, and 4-15 for U, V, and H, using a finite-difference recursion equation (84). The two-dimensional grid for Galveston Bay uses a two-nautical-mile spacing. The model has been verified for Hurricanes Carla of 1961 and Cindy of 1963.

Masch and his associates (61) have made certain revisions to the Reid/Bodine hydrodynamics model in order to make the model more applicable to conditions other than storm surges. Part of this refinement included changing the spacing to a one-mile grid. Shankar (88, 89, 62) added supplemental versatility to

the model by coupling it with a two-dimensional salinity model.

Shankar applies the following form of the mass transport equation, where the terms are defined as in previous examples:

$$\frac{\partial C}{\partial t} = \frac{\partial}{\partial x} \left(E_x \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial y} \left(E_y \frac{\partial C}{\partial y} \right) - U \frac{\partial C}{\partial x} - V \frac{\partial C}{\partial y} \dots (4-16)$$

The Bay was represented according to a repeating grid of cells shown in Figure 4-2. In this grid, the dispersion terms and concentration are cell-centered, whereas the velocities are defined at the cell boundaries. Thus, the following explicit finite-difference equation was developed (88, 89) to approximate Equation 4-16:

$$\begin{aligned} \frac{C_{i,j}^{(k+1)} - C_{i,j}^{(k)}}{\Delta t} = & \frac{1}{2(\Delta x)^2} \left[E_{x_{i,j}} C_{i+1,j}^{(k)} - 2E_{x_{i,j}} C_{i,j}^{(k)} \right. \\ & + E_{x_{i,j}} C_{i-1,j}^{(k)} + E_{x_{i+1,j}} \left(C_{i+1,j}^{(k)} - C_{i,j}^{(k)} \right) \\ & \left. - E_{x_{i-1,j}} \left(C_{i,j}^{(k)} - C_{i-1,j}^{(k)} \right) \right] \\ & + \frac{1}{2(\Delta y)^2} \left[E_{y_{i,j}} C_{i,j+1}^{(k)} - 2E_{y_{i,j}} C_{i,j}^{(k)} \right. \\ & \left. + E_{y_{i,j}} C_{i,j-1}^{(k)} + E_{y_{i,j+1}} \left(C_{i,j+1}^{(k)} - C_{i,j}^{(k)} \right) \right] \end{aligned}$$

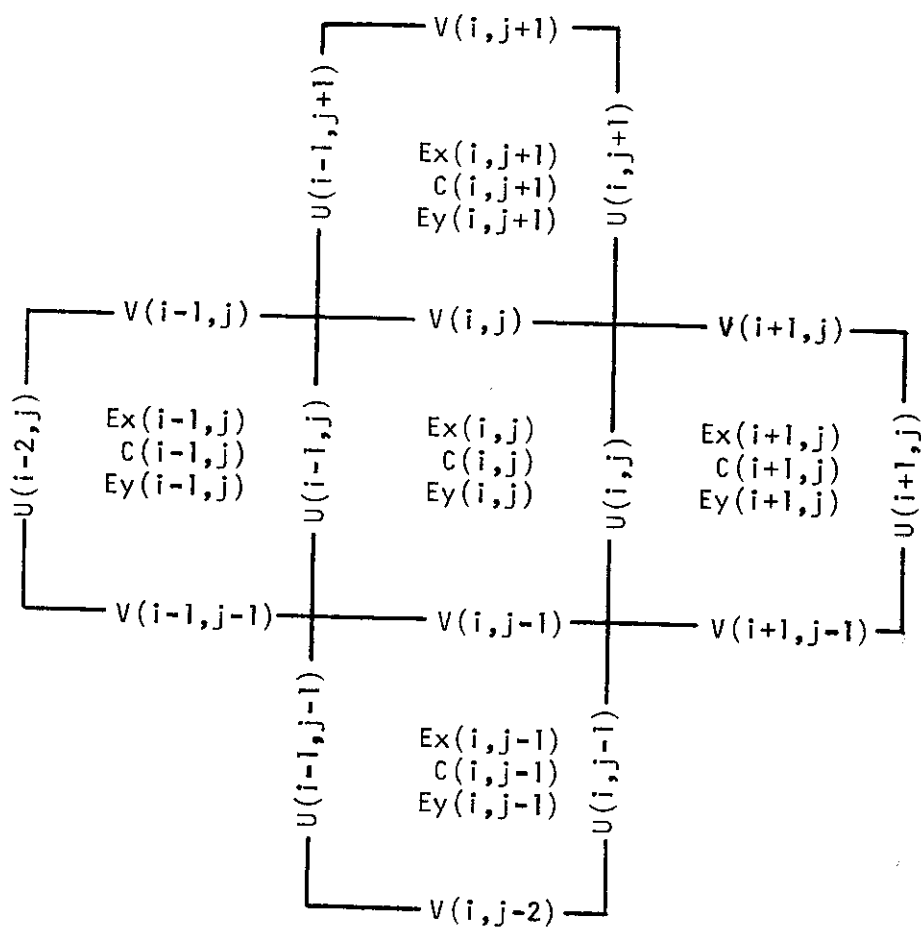


FIGURE 4-2. - CELL STRUCTURE FOR SALINITY MODEL OF SHANKAR AND MASCH

$$\begin{aligned}
 & - E_{y_{i,j-1}} \left(C_{i,j}^{(k)} - C_{i,j+1}^{(k)} \right) \Big] \\
 & - \left(\frac{U_{i,j} + U_{i-1,j}}{2} \right) \left[\frac{C_{i+1,j}^{(k)} - C_{i-1,j}^{(k)}}{2\Delta x} \right] \\
 & - \left(\frac{V_{i,j} + V_{i,j-1}}{2} \right) \left[\frac{C_{i,j+1}^{(k)} - C_{i,j-1}^{(k)}}{2\Delta y} \right] \\
 & \dots \dots \dots (4-17)
 \end{aligned}$$

This equation can be rearranged to compute the unknown salinities, $C_{i,j}^{(k+1)}$, at time $t + \Delta t$ from the known salinities, $C^{(k)}$, at time t .

Tracor and the Galveston Bay Study (GBS) use a hydrodynamic model for the Bay which is basically the same as that used by Masch (21). Likewise, the one-mile grid used by the GBS for mass transport is defined in a manner similar to that of Shankar (Figure 4-2), except that the GBS expresses U , V , and C at the cell center and dispersion at the cell walls (22). In addition, the GBS model includes terms for various sources and sinks. Tracor has applied the GBS model extensively to predict salinity, BOD, temperature, and nutrient patterns in Galveston Bay. Tracor has recently edited a noteworthy report which assesses the state-of-the-art of estuary modeling techniques (20) additional information on the Galveston Bay model can be found in that report.

Recently, Leendertse (59, 60) has also applied finite

difference schemes to hydrodynamic and water quality phenomena.

VERTICAL MODEL BY PRITCHARD AND WILSON

A two-layered, segmented model was suggested by Pritchard (81) in 1969. His approach has been applied recently by Wilson (108) at the Chesapeake Bay Institute under Pritchard's supervision. This analysis was directed towards the flushing of pollutants from a partially mixed estuary in which the flow is strongly two-layered. The segmentation method used by Wilson is shown in Figure 4-3. Horizontal dispersion was ignored on the assumption that mixing could be represented properly by the combined use of vertical mixing and horizontal velocity.

A set of equations was developed to represent the mass balances between the segments and these equations were integrated with time by a Hamming predictor-corrector scheme. This approach was applied to the Northwest Branch of the Baltimore Harbor.

Wilson (108) concluded that the two-layered, segmented model could describe adequately the gross features of advection and diffusion within the Northwest Branch. Use of the model led to estimates of steady-state concentrations; these estimates were considered to be in good accord with field data.

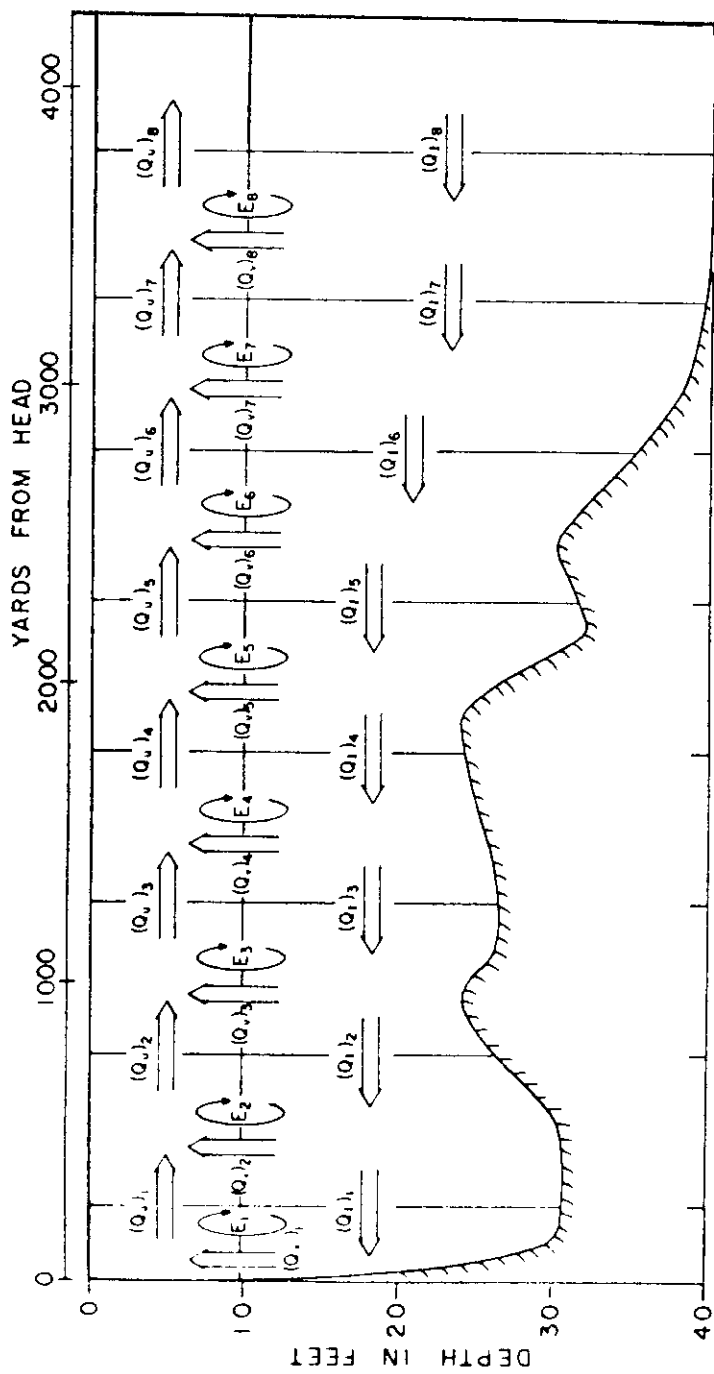


FIGURE 4-3. - SCHEMATIC FOR THE TWO-DIMENSIONAL MODEL OF WILSON AND PRITCHARD INDICATING SEGMENTATION AND PARAMETERS USED TO CHARACTERIZE FLOW. (Q_u , Q_l , and Q_v ARE VOLUME RATES OF NET ADVECTIVE FLOW; E IS THE COEFFICIENT OF VERTICAL DIFFUSIVE EXCHANGE.) (108)

CHAPTER V

DEVELOPMENT OF FINITE DIFFERENCE SCHEMES

As the advantages of high speed digital computers are becoming more widely recognized in the field of mathematical modeling, finite difference techniques are becoming more popular. Finite difference methods are based on the assumption that derivatives can be approximated by using the values of functions at points which are separated by finite increments of space or time. The finite difference approximations that are used to represent partial derivatives can be derived by truncating a Taylor series expansion of a function at a point or, more simply, by visualizing the relationships between slopes and values of a function at separated points. For instance, the following relationships are often used to approximate first and second-order derivatives:

$$\frac{\partial C}{\partial x} = \frac{C_{x+1,t} - C_{x,t}}{\Delta x} + O(\Delta x) \dots \dots \dots (5-1)$$

$$\frac{\partial C}{\partial x} = \frac{C_{x,t} - C_{x-1,t}}{\Delta x} + O(\Delta x) \dots \dots \dots (5-2)$$

$$\frac{\partial C}{\partial x} = \frac{C_{x+1,t} - C_{x-1,t}}{2\Delta x} + O[(\Delta x)^2] \dots \dots \dots (5-3)$$

$$\frac{\partial^2 C}{\partial x^2} = \frac{C_{x-1,t} - 2C_{x,t} + C_{x+1,t}}{(\Delta x)^2} + O[(\Delta x)^2] \dots \dots \dots (5-4)$$

The x and t subscripts denote location of the value C in a space grid and time grid, respectively, and the $O(\Delta x)$ symbol represents the order of magnitude of the error. The Δx represents a dimensionless distance fraction. Equation 5-1 is a forward form of the finite difference representation and Equation 5-2 is a backward form; Equations 5-3 and 5-4 are central difference forms. By using more neighboring points, an unlimited number of other finite difference approximations can be obtained. However, the above forms are the most compact and were judged to be the most useful ones for this study.

The mass transport equation which we are dealing with in this research is a parabolic partial differential equation. When developing a numerical procedure to approximate this equation, a choice must often be made between an "explicit" formulation and an "implicit" formulation. An approach which expresses one unknown pivotal value directly in terms of known pivotal values is called an "explicit" formulation. An approach in which several unknowns are related to one or several unknowns by an equation is called an "implicit" formulation. The implicit approach requires the solution of a set of simultaneous equations, whereas the explicit approach requires the solution of only one equation for each point.

ONE DIMENSIONAL EQUATIONS WITH CONSTANT COEFFICIENTS

These concepts can be demonstrated by showing the derivation

of the finite difference equations for the one-dimensional, constant coefficient case. The partial differential equation can be expressed as

$$\frac{\partial C}{\partial t} = E \frac{\partial^2 C}{\partial x^2} - U \frac{\partial C}{\partial x} - K_d C \dots \dots \dots (5-5)$$

where C is concentration (ppm), E is a constant dispersion (ft²/sec), U is a constant velocity (ft/sec), and K_d is a constant decay rate (/sec).

Explicit Formulation. In terms of the appropriate finite difference equations, an explicit form of this equation is as follows:

$$\begin{aligned} \frac{C_{x,t+1} - C_{x,t}}{\Delta t} = & E \left(\frac{C_{x+1,t} - 2C_{x,t} + C_{x-1,t}}{(\Delta x)^2} \right) \\ & - U \left(\frac{C_{x+1,t} - C_{x-1,t}}{2\Delta x} \right) - K_d (C_{x,t}) \dots \dots \dots (5-6) \end{aligned}$$

After rearranging terms, the following equation results:

$$\begin{aligned} C_{x,t+1} = & \frac{E\Delta t}{(\Delta x)^2} (C_{x+1,t} - 2C_{x,t} + C_{x-1,t}) \\ & - \frac{U\Delta t}{2\Delta x} (C_{x+1,t} - C_{x-1,t}) \\ & - \Delta t K_d (C_{x,t}) + C_{x,t} \dots \dots \dots (5-7) \end{aligned}$$

This equation is used repeatedly at each time step until all the

new values for the t+1 set of points are calculated. Since all the concentrations with a t subscript are known before the equation is applied, the calculation of the t+1 concentrations is straightforward except at the boundaries. At the boundaries, certain procedures must be followed to obtain a solution. These boundary conditions are discussed in a later chapter of this report.

Implicit Formulation. Using a more general approach, the mass transport equation can be represented by the following weighted average approximation (after Smith, 91):

$$\begin{aligned} \frac{C_{x,t+1} - C_{x,t}}{\Delta t} = & \frac{E}{(\Delta x)^2} \left\{ \theta \left(C_{x+1,t+1} - 2C_{x,t+1} + C_{x-1,t+1} \right) \right. \\ & \left. + (1 - \theta) \left(C_{x+1,t} - 2C_{x,t} + C_{x-1,t} \right) \right\} \\ & - \frac{U}{2\Delta x} \left\{ \theta \left(C_{x+1,t+1} - C_{x-1,t+1} \right) \right. \\ & \left. + (1 - \theta) \left(C_{x+1,t} - C_{x-1,t} \right) \right\} \\ & - K_d \left\{ \theta \left(C_{x,t+1} \right) + (1 - \theta) \left(C_{x,t} \right) \right\} \dots \quad (5-8) \end{aligned}$$

This equation uses the same approximations for the derivatives as were used previously for the explicit formulation. If we substitute $\theta = 0$, we obtain Equation 5-7 of the explicit scheme. If we substitute $\theta = 1/2$, we obtain what is known as the Crank-Nicolson

implicit formulation. If we substitute $\theta = 1$, we obtain a fully implicit, backward difference formula. These formulations are compared schematically in Figures 5-1, 5-2, and 5-3. For any value of θ greater than zero, a set of simultaneous equations must be solved.

Using $\theta = 1/2$ in Equation 5-8, and collecting terms, the following Crank-Nicolson representation is obtained:

$$\begin{aligned} & -A_i \left(C_{x-1,t+1} \right) + B_i \left(C_{x,t+1} \right) - D_i \left(C_{x+1,t+1} \right) \\ & = A_i \left(C_{x-1,t} \right) - B'_i \left(C_{x,t} \right) + D_i \left(C_{x+1,t} \right) \dots \dots \dots (5-9) \end{aligned}$$

where

$$A_i = \left(\frac{E\Delta t}{2(\Delta x)^2} + \frac{U\Delta t}{4\Delta x} \right) \dots \dots \dots (5-10)$$

$$B_i = \left(\frac{E\Delta t}{(\Delta x)^2} + \frac{K_d\Delta t}{2} + 1 \right) \dots \dots \dots (5-11)$$

$$B'_i = B_i - 2$$

$$D_i = \left(\frac{E\Delta t}{2(\Delta x)^2} - \frac{U\Delta t}{4\Delta x} \right) \dots \dots \dots (5-12)$$

for $i = 2, 3, 4, \dots, N - 3$, where N is the number of points.

For N points, there are $N - 2$ equations. By arranging the knowns and unknowns for a series of distance steps, the following

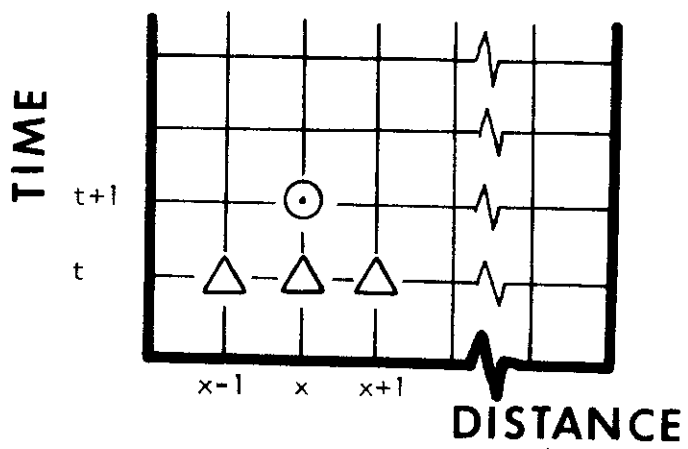


FIGURE 5-1. - EXPLICIT ($\theta = 0$)

\triangle = KNOWN VALUES
 \odot = UNKNOWN VALUES

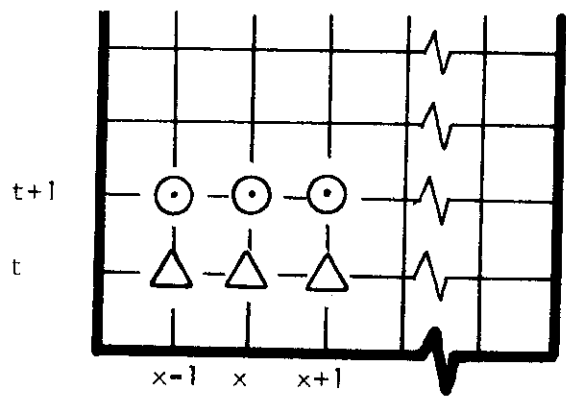


FIGURE 5-2. - CRANK-NICOLSON ($\theta = 1/2$)

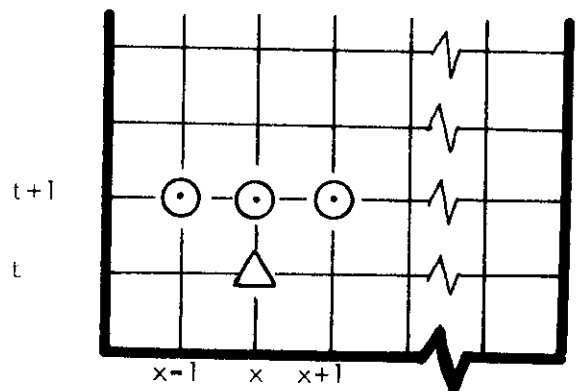


FIGURE 5-3. - FULLY IMPLICIT ($\theta = 1$)

set of simultaneous equations is obtained:

$$\begin{aligned}
 B_1 C_{2,t+1} - D_1 C_{3,t+1} &= W_1 \\
 -A_2 C_{2,t+1} + B_2 C_{3,t+1} - D_2 C_{4,t+1} &= W_2 \\
 -A_3 C_{3,t+1} + B_3 C_{4,t+1} - D_3 C_{5,t+1} &= W_3 \\
 \vdots & \\
 \vdots & \\
 \vdots & \\
 \vdots & \\
 \vdots & \\
 -A_{N-2} C_{N-2,t+1} + B_{N-2} C_{N-1,t+1} &= W_{N-2} \\
 \dots\dots\dots & \dots\dots\dots (5-13)
 \end{aligned}$$

The W_i terms represent the collection of known concentrations and their coefficients, as shown by the right side of Equation 5-9. The values of the terms B_1 , D_1 , W_1 , A_{N-2} , B_{N-2} , and W_{N-2} are dependent upon the boundary conditions which are applied to the problem; these considerations will be discussed in a later chapter of this report.

Fortunately, the coefficients of these simultaneous equations form a tridiagonal matrix which can be solved by a relatively simple Gauss elimination method, also known as the "Thomas Algorithm." If more than three unknowns are used in each equation, a more time-consuming matrix solution technique would be needed; this discourages the use of formulas which require more than three

points to approximate derivatives.

The Gauss method begins by using the first equation to eliminate $C_{2,t+1}$ from the second equation. Then the new second equation is used to eliminate $C_{3,t+1}$ from the third equation, and so on. Finally, the new last-but-one equation can be used to eliminate $C_{N-2,t+1}$ from the last equation. This leaves one equation with only one unknown, $C_{N-1,t+1}$. Thus, the unknown $C_{i,t+1}$ values can be found by back-substitution. This procedure is summarized in the following steps (11, 91):

1. Define all A_i , B_i , D_i , and W_i values;
2. Define and calculate alpha's (α_i);

$$\alpha_1 = B_1 \dots \dots \dots (5-14)$$

$$\alpha_i = B_i - \frac{A_i D_{i-1}}{\alpha_{i-1}} \dots \dots \dots (5-15)$$

for $i = 2, 3, \dots, N-2$

3. Define and calculate S's

$$S_1 = W_1 \dots \dots \dots (5-16)$$

$$S_i = W_i + \frac{A_i S_{i-1}}{\alpha_{i-1}} \dots \dots \dots (5-17)$$

for $i = 2, 3, \dots, N-2$

4. Calculate C's starting with $C_{N-1,t+1}$

$$C_{N-1,t+1} = \frac{S_{N-2}}{\alpha_{N-2}} \dots \dots \dots (5-18)$$

$$C_{i,t+1} = \frac{1}{\alpha_i} S_i + D_i C_{i+1,t+1} \dots \dots \dots (5-19)$$

for $i = N-2, N-3, \dots 2$.

ONE DIMENSIONAL EQUATIONS WITH VARYING COEFFICIENTS

The finite difference equations become more complicated when the coefficients are allowed to vary with time and distance. The appropriate partial differential equation which permits varying coefficients and varying width is the following:

$$W \frac{\partial C}{\partial t} = - W_x V \frac{\partial C}{\partial x} + \frac{\partial}{\partial x} \left(E_x W \frac{\partial C}{\partial x} \right) - W_x K D_x C + (\text{Sources} - \text{Sinks})$$

\dots \dots \dots (5-20)

The terms in this equation are as defined in Equation 3-24.

Explicit Formulation. - The explicit formulation for this equation uses the approximations listed in Equations 5-1, 5-3, and 5-4. In the following finite difference equations, a z subscript is used in order to be consistent with the two-dimensional formulations; this z is always equal to 1 in the one-dimensional formulas. The one-dimensional, explicit, finite difference approximation to Equation 5-20 is shown in Figure 5-4, as well as

Partial Differential Equation For One-Dimensional Mass Transport:

$$W_x \frac{\partial C}{\partial t} = - W_x V_x \frac{\partial C}{\partial x} + \frac{\partial}{\partial x} \left(E_x W_x \frac{\partial C}{\partial x} \right) - W_x KD_x C + (\text{Sources} - \text{Sinks}) \dots \dots \dots (5-20)$$

Explicit Finite Difference Formulation:

$$\begin{aligned} \frac{W_x}{\Delta t} (C_{x,z,t+1} - C_{x,z,t}) &= - \frac{W_x V_x C_{x,z}}{2\Delta x} (C_{x+1,z,t} - C_{x-1,z,t}) \\ &+ \left(\frac{W_{x+1} EX_{x+1,z} + W_x EX_{x,z}}{2\Delta x} \right) \left(\frac{C_{x+1,z,t} - C_{x,z,t}}{\Delta x} \right) \\ &- \left(\frac{W_x EX_{x,z} + W_{x-1} EX_{x-1,z}}{2\Delta x} \right) \left(\frac{C_{x,z,t} - C_{x-1,z,t}}{\Delta x} \right) \\ &- W_x KD_x C_{x,z,t} + (\text{Sources} - \text{Sinks}) \dots \dots \dots (5-21) \end{aligned}$$

FIGURE 5-4. - EXPLICIT FINITE DIFFERENCE EQUATIONS FOR ONE-DIMENSIONAL MASS TRANSPORT WITH VARYING COEFFICIENTS

(Continued on next page)

Rearranged Explicit Equation:

$$\begin{aligned}
 C_{x,z,t+1} = & - C_{x-1,z,t} \left(\frac{\Delta t}{W_x} \right) \left(\frac{W_x V X_{x,z}}{2\Delta x} + \frac{W_x EX_{x,z} + W_{x-1} EX_{x-1}}{2(\Delta x)^2} \right) \\
 & - C_{x,z,t} \left(\frac{\Delta t}{W_x} \right) \left(\frac{W_{x+1} EX_{x+1,z} + W_x EX_{x,z} + \frac{W_x EX_{x,z} + W_{x-1} EX_{x-1,z}}{2(\Delta x)^2}}{2(\Delta x)^2} \right) \\
 & + C_{x+1,z,t} \left(\frac{\Delta t}{W_x} \right) \left(- \frac{W_x V X_{x,z}}{2\Delta x} + \frac{W_{x+1} EX_{x+1,z} + W_x EX_{x,z}}{2(\Delta x)^2} \right) \\
 & + C_{x,z,t} - KD_x \Delta t C_{x,z,t} + (\text{Sources} - \text{Sinks}) \dots \dots \dots (5-22)
 \end{aligned}$$

(End of Figure 5-4)

a more useful rearranged version. The Equation 5-22 is solved repeatedly for each time step as in the explicit equation with constant coefficients discussed previously.

Implicit Formulation. - The Crank-Nicolson implicit finite difference formulation for the one-dimensional mass transport equation with varying coefficients is shown in Figure 5-5; the z subscript is equal to 1 in this one-dimensional framework and terms of the form " - - NEXT" refer to values at time $t+1$.

This set of equations can be arranged into a tridiagonal matrix. For easier programming, the indexing system is different than the previous one-dimensional, implicit array:

$$B(2)C_{2,z,t+1} + G(2)C_{3,z,t+1} = D(2)$$

$$A(3)C_{2,z,t+1} + B(3)C_{3,z,t+1} + G(3)C_{4,z,t+1} = D(3)$$

$$A(4)C_{3,z,t+1} + B(4)C_{4,z,t+1} + G(4)C_{5,z,t+1} = D(4)$$

$$\begin{array}{ccc} \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot \end{array}$$

$$A(N-1)C_{N-2,z,t+1} + B(N-1)C_{N-1,z,t+1} = D(N-1)$$

..... (5-24)

This set of $N-2$ equations is used for a grid with N horizontal points. The values for $B(2)$, $G(2)$, $D(2)$, $A(N-1)$, $B(N-1)$, and

$D(N-1)$ depend upon the boundary conditions of the problems. The values for other $D(I)$ are found by collecting all the C terms in Equation 5-23 with a subscript of t and their associated coefficients. This yields the values for the arrays A , B , G , and D shown in Figure 5-6. The tridiagonal matrix for this formulation is solved by the Gauss elimination scheme outlined previously in Equations 5-14 through 5-19.

Partial Differential Equation for One Dimensional Mass Transport:

$$W_X \frac{\partial C}{\partial t} = - W_X V_X \frac{\partial C}{\partial X} + \frac{\partial}{\partial X} \left(E_X W_X \frac{\partial C}{\partial X} \right) - W_X K D_X C + (\text{Sources} - \text{Sinks}) \dots \dots \dots (5-20)$$

Implicit Crank-Nicolson Finite Difference Formulation:

$$\begin{aligned} \frac{W_X}{\Delta t} \left(C_{X,Z,t+1} - C_{X,Z,t} \right) = & - \frac{W_X V_{X,Z}}{2} \left\{ \frac{C_{X+1,Z,t+1} - C_{X-1,Z,t+1}}{2\Delta X} \right\} \\ & - \frac{W_X V_{X,Z}}{2} \left\{ \frac{C_{X+1,Z,t} - C_{X-1,Z,t}}{2\Delta X} \right\} \\ & + \frac{1}{2} \left(\frac{W_X^{EXNEXT}{}_{X+1,Z} + W_X^{EXNEXT}{}_{X,Z}}{2\Delta X} \right) \left\{ \frac{C_{X+1,Z,t+1} - C_{X,Z,t+1}}{\Delta X} \right\} \\ & + \frac{1}{2} \left(\frac{W_X^{EX}{}_{X+1,Z} + W_X^{EX}{}_{X,Z}}{2\Delta X} \right) \left\{ \frac{C_{X+1,Z,t} - C_{X,Z,t}}{\Delta X} \right\} \\ & - \frac{1}{2} \left(\frac{W_X^{EXNEXT}{}_{X,Z} + W_X^{EXNEXT}{}_{X-1,Z}}{2\Delta X} \right) \left\{ \frac{C_{X,Z,t+1} - C_{X-1,Z,t+1}}{\Delta X} \right\} \end{aligned}$$

FIGURE 5-5. - IMPLICIT CRANK-NICOLSON FINITE DIFFERENCE EQUATION FOR ONE-DIMENSIONAL MASS TRANSPORT WITH VARYING COEFFICIENTS

(Continued on next page)

$$\begin{aligned}
 & - \frac{1}{2} \left(\frac{W_x^{EX}}{X} + \frac{W_{x-1}^{EX}}{2\Delta X} \right) \left\{ \frac{C_{x,z,t} - C_{x-1,z,t}}{\Delta X} \right\} \\
 & - \frac{1}{2} W_x^{KD} \left\{ C_{x,z,t+1} \right\} - \frac{1}{2} W_x^{KD} \left\{ C_{x,z,t} \right\} + (\text{Sources} - \text{Sinks}) \\
 & \dots\dots\dots (5-23)
 \end{aligned}$$

(End of Figure 5-5)

$$A(x) = \left(\frac{\Delta t}{W_x}\right) \left(-\frac{W_x V_{XNEXT} x,z}{4\Delta x} - \frac{W_x EXNEXT x,z + W_{x-1} EXNEXT x-1,z}{4(\Delta x)^2} \right) \dots \dots \dots (5-25)$$

$$B(x) = 1 + \left(\frac{\Delta t}{W_x}\right) \left\{ \left(\frac{W_{x+1} EXNEXT x+1,z + 2W_x EXNEXT x,z + W_{x-1} EXNEXT x-1,z}{4(\Delta x)^2} + \frac{W_x KD_x}{2} \right) \right\} \dots \dots \dots (5-26)$$

$$G(x) = \left(\frac{\Delta t}{W_x}\right) \left(\frac{W_x V_{XNEXT} x,z}{4\Delta x} - \frac{W_{x+1} EXNEXT x+1,z + W_x EXNEXT x,z}{4(\Delta x)^2} \right) \dots \dots \dots (5-27)$$

$$D(x) = C_{x,z,t} + \left(\frac{\Delta t}{W_x}\right) \left(-\frac{W_x V_{X,z}}{4\Delta x} \right) \left\{ C_{x+1,z,t} - C_{x-1,z,t} \right\} \\ + \left(\frac{\Delta t}{W_x}\right) \left(\frac{W_{x+1} EX_{x+1,z} + W_x EX_{x,z}}{4(\Delta x)^2} \right) \left\{ C_{x+1,z,t} - C_{x,z,t} \right\} \\ - \left(\frac{\Delta t}{W_x}\right) \left(\frac{W_x EX_{x,z} + W_{x-1} EX_{x-1,z}}{4(\Delta x)^2} \right) \left\{ C_{x,z,t} - C_{x-1,z,t} \right\} - \frac{KD_x}{2} \left\{ C_{x,z,t} \right\} \dots \dots \dots (5-28)$$

FIGURE 5-6. - COEFFICIENTS FOR TRIDIAGONAL MATRIX

TWO DIMENSIONAL EQUATIONS WITH VARYING COEFFICIENTS

The most complicated form of the mass transport equation which has been programmed in this research allows for two dimensional convective-dispersion with variable coefficients:

$$\begin{aligned}
 W_x \frac{\partial C}{\partial t} &= \frac{\partial}{\partial x} \left(E_x W_x \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial z} \left(E_z W_x \frac{\partial C}{\partial z} \right) \\
 &= W_x V_x \frac{\partial C}{\partial x} - W_x V_z \frac{\partial C}{\partial z} - W_x K D_x C + (\text{Sources} - \text{Sinks}) . \quad (5-29)
 \end{aligned}$$

This equation was derived and discussed in Chapter III.

Explicit Formulation. The two dimensional explicit formulation takes the same approach as Equations 5-21 and 5-22 of the one-dimensional explicit method, but in addition includes terms in the vertical z direction. The two-dimensional explicit finite difference equation is shown in Figure 5-7. A more convenient rearranged version is shown also.

The Equation 5-30 can be visualized as the computational molecule shown in Figure 5-8.

Partial Differential Equation for Two Dimensional Mass Transport:

$$W_x \frac{\partial C}{\partial t} = - W_x V_x \frac{\partial C}{\partial x} - W_x V_z \frac{\partial C}{\partial z} + \frac{\partial}{\partial x} \left(E_x W_x \frac{\partial C}{\partial t} \right) + \frac{\partial}{\partial z} \left(E_z W_x \frac{\partial C}{\partial z} \right) - W_x K D_x C + (\text{Sources} - \text{Sinks})$$

..... (5-29)

Explicit Finite Difference Formulation:

$$\begin{aligned} \frac{W_x}{\Delta t} \left\{ C_{x,z,t+1} - C_{x,z,t} \right\} = & - \frac{W_x V_x}{2\Delta x} \left\{ C_{x+1,z,t} - C_{x-1,z,t} \right\} - \frac{W_x V_z}{2\Delta z} \left\{ C_{x,z+1,t} - C_{x,z-1,t} \right\} \\ & + \left(\frac{W_{x+1} E_x}{2\Delta x} + \frac{W_x E_x}{2\Delta x} \right) \left\{ \frac{C_{x+1,z,t} - C_{x,z,t}}{\Delta x} \right\} \\ & - \left(\frac{W_x E_x}{2\Delta x} + \frac{W_{x-1} E_x}{2\Delta x} \right) \left\{ \frac{C_{x,z,t} - C_{x-1,z,t}}{\Delta x} \right\} \\ & + \left(\frac{W_x E_z}{2\Delta z} + \frac{W_x E_z}{2\Delta z} \right) \left\{ \frac{C_{x,z+1,t} - C_{x,z,t}}{\Delta z} \right\} \end{aligned}$$

FIGURE 5-7. - EXPLICIT FINITE DIFFERENCE EQUATIONS FOR TWO-DIMENSIONAL MASS TRANSPORT WITH VARYING COEFFICIENTS

(Continued on next page)

$$\begin{aligned}
 & - \left(\frac{W_{X,EZ} C_{X,Z,t} + W_{X,E} C_{X,Z,t-1}}{2\Delta z} \right) \left\{ \frac{C_{X,Z,t} - C_{X,Z,t-1}}{\Delta z} \right\} \\
 & - W_{X,KD} C_{X,Z,t} + (\text{Sources} - \text{Sinks}) \dots \dots \dots (5-30)
 \end{aligned}$$

Rearranged Explicit Equation:

$$\begin{aligned}
 C_{X,Z,t+1} = & C_{X-1,Z,t} \left(\frac{\Delta t}{W_X} \right) \left(\frac{W_{X,VX} C_{X,Z} + W_{X,EX} C_{X-1,Z}}{2\Delta x} + \frac{W_{X,EX} C_{X,Z} + W_{X-1,EX} C_{X-1,Z}}{2(\Delta x)^2} \right) \\
 & - C_{X,Z,t} \left(\frac{\Delta t}{W_X} \right) \left(\frac{W_{X+1,EX} C_{X+1,Z} + W_{X,EX} C_{X,Z}}{2(\Delta x)^2} + \frac{W_{X,EX} C_{X,Z} + W_{X-1,EX} C_{X-1,Z}}{2(\Delta x)^2} \right) \\
 & + \frac{W_{X,EZ} C_{X,Z,t+1} + W_{X,EZ} C_{X,Z}}{2(\Delta z)^2} + \frac{W_{X,EZ} C_{X,Z} + W_{X,EZ} C_{X,Z-1}}{2(\Delta z)^2} \\
 & + C_{X+1,Z,t} \left(\frac{\Delta t}{W_X} \right) \left(- \frac{W_{X,VX} C_{X,Z}}{2\Delta x} + \frac{W_{X+1,EX} C_{X+1,Z} + W_{X,EX} C_{X,Z}}{2(\Delta x)^2} \right) \\
 & + C_{X,Z-1,t} \left(\frac{\Delta t}{W_X} \right) \left(\frac{W_{X,VZ} C_{X,Z}}{2\Delta z} + \frac{W_{X,EZ} C_{X,Z} + W_{X,EZ} C_{X,Z-1}}{2(\Delta z)^2} \right)
 \end{aligned}$$

FIGURE 5-7. - (Continued)

$$+ C_{X,Z+1,t} \left(\frac{\Delta t}{W_X} \right) \left(- \frac{W_X V Z_{X,Z}}{2 \Delta z} + \frac{W_X E Z_{X,Z+1}}{2 (\Delta z)^2} + \frac{W_X E Z_{X,Z}}{2 (\Delta z)^2} \right)$$

$$+ C_{X,Z,t} - \Delta t K D_X C_{X,Z,t} + (\text{Sources} - \text{Sinks}) \dots \dots \dots (5-31)$$

(End of Figure 5-7)

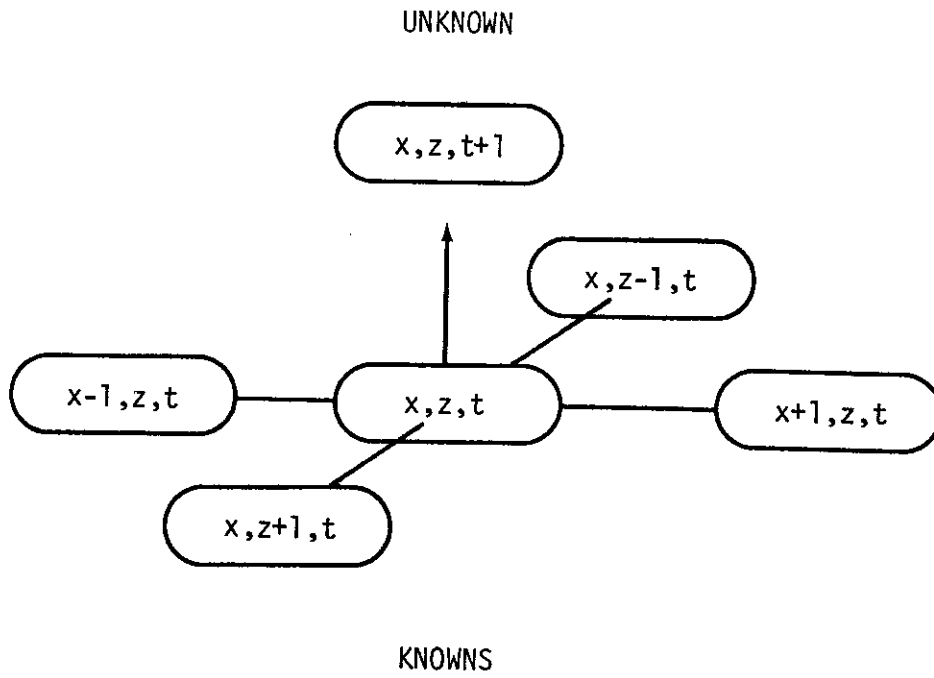


FIGURE 5-8. - TWO-DIMENSIONAL EXPLICIT COMPUTATION MOLECULE

Implicit Formulation. In order to avoid solving a large, two-dimensional matrix by a time-consuming iterative technique, a procedure known as the "implicit alternating-direction method" can be used. This approach, as discussed by Peaceman and Rachford (74), and later modified by Douglas (18), permits the application of the efficient tridiagonal matrix solution explained earlier in this report to a two-dimensional, Crank-Nicolson implicit approximation to the mass transport equation. Additional discussions of this approach are found in other sources (11, 28, 91). This method requires a two-phase procedure. During the initial phase, a Crank-Nicolson scheme is applied only to concentrations in the x-direction. The new concentrations, called CSTAR, which result from the solution of this system of equations are used in a second phase where the Crank-Nicolson scheme is applied to concentrations in the z-direction. The values of CSTAR can be considered as initial approximations which are used to find a more accurate solution during the second phase. This procedure can be expressed schematically as follows:

First Phase.

$$\begin{aligned} & \frac{W_x}{\Delta t} \left(\text{CSTAR}_{x,z} - C_{x,z,t} \right) \\ & = \frac{1}{2} \left(\text{Derivatives and coefficients of } C \text{ in } x\text{-direction at time} \right. \\ & \quad \left. t+1 \right) \end{aligned}$$

$$\begin{aligned}
 & + \frac{1}{2} \text{ (Derivatives and coefficients of } C \text{ in } x\text{-direction at time } t) \\
 & + \text{ (Derivatives and coefficients of } C \text{ in } z\text{-direction at time } t) \\
 & \dots \dots \dots (5-32)
 \end{aligned}$$

Second Phase.

$$\begin{aligned}
 & \frac{W_x}{\Delta t} (C_{x,z,t+1} - C_{x,z,t}) \\
 & = \frac{1}{2} \text{ (Derivatives and coefficients of } C_{STAR} \text{ in } x \text{ direction).} \\
 & + \frac{1}{2} \text{ (Derivatives and coefficients of } C \text{ in } x\text{-direction at time } t) \\
 & + \frac{1}{2} \text{ (Derivatives and coefficients of } C \text{ in } z\text{-direction at time } t+1) \\
 & + \frac{1}{2} \text{ (Derivatives and coefficients of } C \text{ in } z\text{-direction at time } t) \\
 & \dots \dots \dots (5-33)
 \end{aligned}$$

This two-phase approach requires the solution of two tri-diagonal matrices. The arrays are of the same form as Equation

5-24. The equations for these arrays are shown in Figures 5-9 and 5-10. As in the previous implicit cases, values for $B(2)$, $G(2)$, $D(2)$, $A(N-1)$, $B(N-1)$, and $D(N-1)$ are dependent upon the conditions at the boundaries. The computational molecule for the two-dimensional Crank-Nicolson scheme is shown in Figure 5-11. An example of a two-dimensional grid is shown in Figure 5-12. Values for dispersion, velocity, decay, and other variables must be defined for each grid point.

where NX equals the number of points in the x-direction and for X = 3, 4, 5, . . . NX-2:

$$A(X) = \left(\frac{\Delta t}{W_X}\right) \left(-\frac{W_X V_{XNEXT} X,Z}{4\Delta X} - \frac{W_X EX_{NEXT} X,Z}{4(\Delta X)^2} + \frac{W_X EX_{NEXT} X-1,Z}{4(\Delta X)^2} \right) \dots \dots \dots (5-34)$$

$$B(X) = 1. + \left(\frac{\Delta t}{W_X}\right) \left(\frac{W_X EX_{NEXT} X+1,Z}{4(\Delta X)^2} + \frac{W_X EX_{NEXT} X,Z}{4(\Delta X)^2} + \frac{W_X EX_{NEXT} X-1,Z}{4(\Delta X)^2} \right) \dots \dots \dots (5-35)$$

$$G(X) = \left(\frac{\Delta t}{W_X}\right) \left(\frac{W_X V_{XNEXT} X,Z}{4\Delta X} - \frac{W_X EX_{NEXT} X+1,Z}{4(\Delta X)^2} + \frac{W_X EX_{NEXT} X,Z}{4(\Delta X)^2} \right) \dots \dots \dots (5-36)$$

$$D(X) = C_{X,Z,t} + \left(\frac{\Delta t}{W_X}\right) \left[-\frac{W_X V_{X,Z}}{4\Delta X} \left\{ C_{X+1,Z,t} - C_{X-1,Z,t} \right\} - \frac{2W_X V_{Z,X,Z}}{4\Delta Z} \left\{ C_{X,Z+1,t} - C_{X,Z-1,t} \right\} + \left(\frac{W_X EX_{X+1,Z}}{4(\Delta X)^2} + \frac{W_X EX_{X,Z}}{4(\Delta X)^2} \right) \left\{ C_{X+1,Z,t} - C_{X,Z,t} \right\} - \left(\frac{W_X EX_{X,Z}}{4(\Delta X)^2} + \frac{W_X EX_{X-1,Z}}{4(\Delta X)^2} \right) \left\{ C_{X,Z,t} - C_{X-1,Z,t} \right\} + 2 \left(\frac{W_X EZ_{X,Z+1}}{4(\Delta Z)^2} + \frac{W_X EZ_{X,Z}}{4(\Delta Z)^2} \right) \right]$$

FIGURE 5-9. - ARRAY VALUES FOR THE FIRST PHASE OF THE IMPLICIT PROCEDURE

(Continued on next page)

$$\left\{ C_{x,z+1,t} - C_{x,z,t} \right. \\ \left. - 2 \left(\frac{W_x E_z}{4(\Delta z)^2} + \frac{W_x E_z}{4(\Delta z)^2} \right) \left\{ C_{x,z,t} - C_{x,z-1,t} \right\} \dots \dots \dots \right] \dots \dots \dots \quad (5-37)$$

(End of Figure 5-9)

Where NZ equals the number of points in the z-direction and for Z = 3, 4, 5, . . . NZ-2:

$$A(Z) = \left(\frac{\Delta t}{W_X}\right) \left(- \frac{W_X VZNEXT_{X,Z}}{4\Delta Z} - \frac{W_X EZNEXT_{X,Z} + W_X EZNEXT_{X,Z-1}}{4(\Delta z)^2} \right) \dots \dots \dots (5-38)$$

$$B(Z) = 1. + \left(\frac{\Delta t}{W_X}\right) \left(\frac{W_X EZNEXT_{X,Z+1} + W_X EZNEXT_{X,Z}}{4(\Delta z)^2} + \frac{W_X EZNEXT_{X,Z-1} + W_X EZNEXT_{X,Z}}{4(\Delta z)^2} \right) \dots \dots \dots (5-39)$$

$$G(Z) = \left(\frac{\Delta t}{W_X}\right) \left(\frac{W_X VZNEXT_{X,Z}}{4\Delta Z} - \frac{W_X EZNEXT_{X,Z+1} + W_X EZNEXT_{X,Z}}{4(\Delta z)^2} \right) \dots \dots \dots (5-40)$$

$$D(Z) = C_{X,Z,t} + \left(\frac{\Delta t}{W_X}\right) \left[- \frac{W_X VXNEXT_{X,Z}}{4\Delta X} \left\{ CSTAR_{X+1,Z} - CSTAR_{X-1,Z} \right\} \right. \\ \left. - \frac{W_X VX_{X,Z}}{4\Delta X} \left\{ C_{X+1,Z,t} - C_{X-1,Z,t} \right\} - \frac{W_X VZ_{X,Z}}{4\Delta Z} \left\{ C_{X,Z+1,t} - C_{X,Z-1,t} \right\} \right] \\ + \left(\frac{W_X EXNEXT_{X+1,Z} + W_X EXNEXT_{X,Z}}{4(\Delta x)^2} \right) \left\{ CSTAR_{X+1,Z} - CSTAR_{X,Z} \right\} \\ - \left(\frac{W_X EXNEXT_{X,Z} + W_X EXNEXT_{X-1,Z}}{4(\Delta x)^2} \right) \left\{ CSTAR_{X,Z} - CSTAR_{X-1,Z} \right\} \quad \text{(Continued on next page)}$$

FIGURE 5-10. - ARRAY VALUES FOR THE SECOND PHASE OF THE IMPLICIT PROCEDURE

$$\begin{aligned}
& + \left(\frac{W_{x+1} EX_{x+1,z} + W_x EX_{x,z}}{4(\Delta x)^2} \right) \left\{ C_{x+1,z,t} - C_{x,z,t} \right\} - \left(\frac{W_x EX_{x,z} + W_{x-1} EX_{x-1,z}}{4(\Delta x)^2} \right) \\
& \left\{ C_{x,z,t} - C_{x-1,z,t} \right\} \\
& + \left(\frac{W_x EZ_{x,z+1} + W_x EZ_{x,z}}{4(\Delta z)^2} \right) \left\{ C_{x,z+1,t} - C_{x,z,t} \right\} - \left(\frac{W_x EZ_{x,z} + W_x EZ_{x,z-1}}{4(\Delta z)^2} \right) \\
& \left\{ C_{x,z,t} - C_{x,z-1,t} \right\} \dots \dots \dots (5-41)
\end{aligned}$$

For mass transport problems with decay and other source/sink terms, this additional equation is used after the solution of the tridiagonal matrix:

$$\text{FINAL } C_{x,z,t+1} = C_{x,z,t+1} - \left(\frac{\Delta t K D X}{2} \right) \left\{ C_{x,z,t+1} + C_{x,z,t} \right\} + (\text{Sources} - \text{Sinks}) \dots \dots \dots (5-42)$$

(End of Figure 5-10)

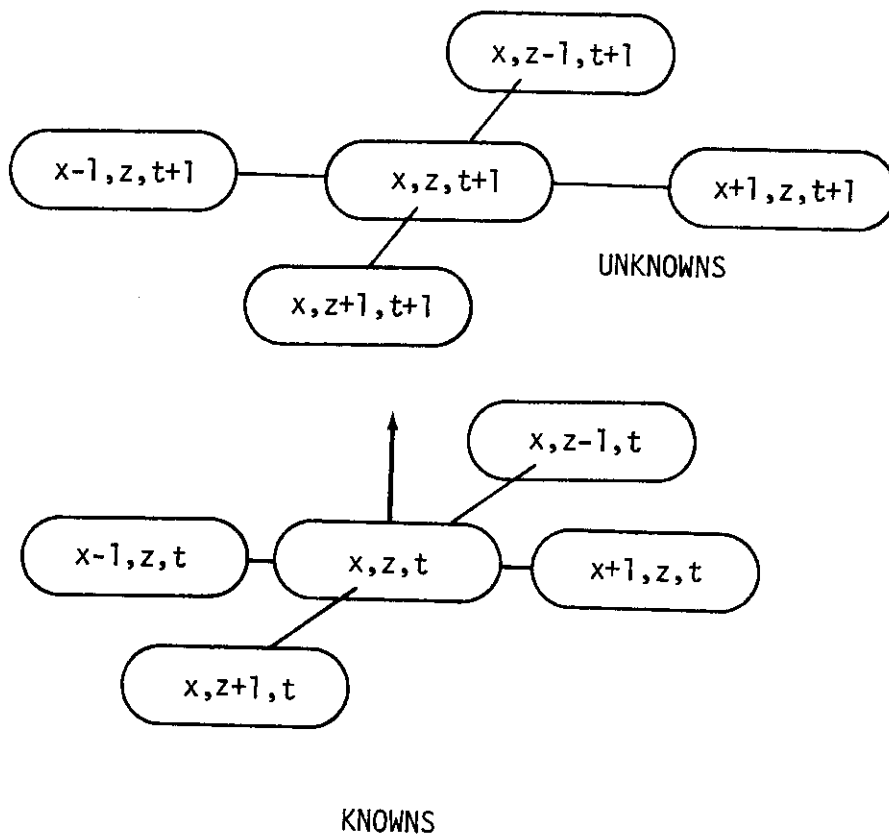


FIGURE 5-11. - TWO-DIMENSIONAL IMPLICIT COMPUTATIONAL MOLECULE

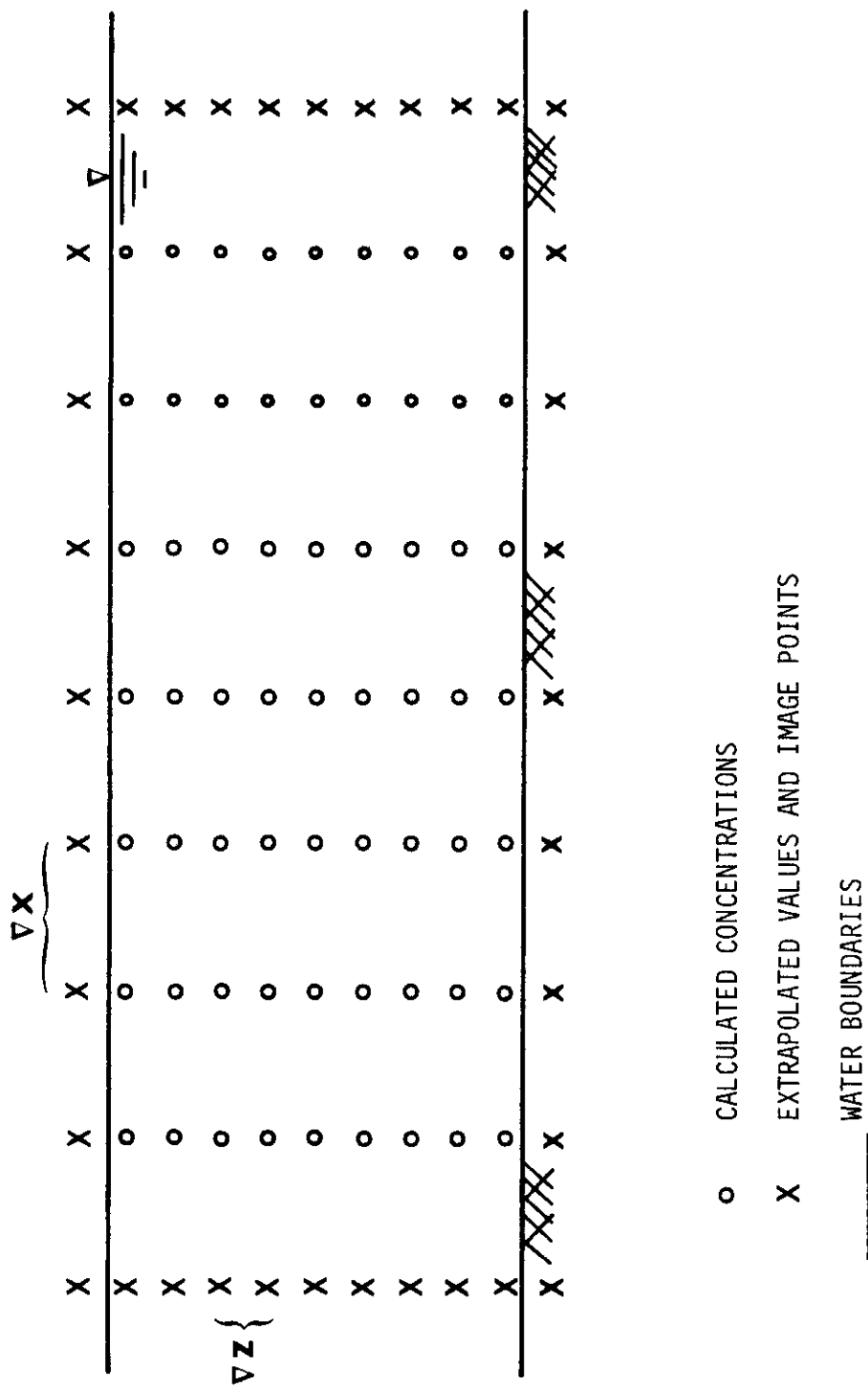


FIGURE 5-12. - TWO-DIMENSIONAL VERTICAL GRID

CHAPTER VI

STABILITY CONSIDERATIONS

Instability is an important problem which must be dealt with when applying finite difference techniques. Instability refers to the tendency of errors to grow without limit as the computation progresses. These errors can be generated by inexact initial conditions, inexact boundary conditions, oscillations generated in early time steps, or round-off errors. Stability, on the other hand, is the tendency to damp out errors as the computation progresses. If a solution is unstable, it becomes hopelessly inaccurate. On the other hand, a stable solution is not necessarily an accurate solution. The accuracy of a solution must be investigated after the stability criteria has been established.

EXPLICIT METHOD

Several sophisticated techniques are available for analyzing the stability of explicit finite difference formulations. However, in this study, the following simple rule was found to be the most convenient: stability is assured when the coefficient for each concentration value is greater than zero. Thus, examination of the Equation 5-31 leads to the following inequalities if variations with distance are considered to be small, and if the subscripts x , z , and t are dropped:

$$EX \frac{\Delta t}{(\Delta x)^2} - VX \frac{\Delta t}{2\Delta x} > 0 \dots \dots \dots (6-1)$$

$$EZ \frac{\Delta t}{(\Delta z)^2} - VZ \frac{\Delta t}{2\Delta z} > 0 \dots \dots \dots (6-2)$$

$$1 - 2EX \frac{\Delta t}{(\Delta x)^2} - 2EZ \frac{\Delta t}{(\Delta z)^2} - \Delta t KD > 0 \dots \dots \dots (6-3)$$

According to Equations 6-1 and 6-2, dispersion coefficients in a certain direction can not be set equal to zero when a velocity exists in that direction. Solution of these equations, in order, leads to the following stability criteria:

$$\Delta x < \frac{2EX}{VX} \dots \dots \dots (6-4)$$

$$\Delta z < \frac{2EZ}{VZ} \dots \dots \dots (6-5)$$

$$\Delta t < \frac{(\Delta x)^2 (\Delta z)^2}{2EX(\Delta z)^2 + 2EZ(\Delta x)^2 + KD(\Delta x)^2 (\Delta z)^2} \dots \dots \dots (6-6)$$

The corresponding stability criteria for one dimensional analysis is

$$\Delta x < \frac{2EX}{VX} \dots \dots \dots (6-7)$$

$$\Delta t < \frac{(\Delta x)^2}{2EX + (\Delta x)^2 KD} \dots \dots \dots (6-8)$$

One-dimensional stability restrictions can be examined rapidly by plotting coaxial graphs on log-log paper for Equations 6-7 and 6-8. Figures 6-1 and 6-2 display such graphs for a useful range of velocity and dispersion values.

When applying these criteria to the data in this study, the minimum numerators and maximum denominators were used. For example, when applying Equation 6-6, the maximum values for EX, EZ, and KD were used; likewise, when applying Equation 6-4, the minimum value for EX and the maximum value for VX were used. This approach leads to a conservative estimate of stability criteria, since maximum velocities and minimum dispersions rarely occur at the same point.

IMPLICIT METHOD

The Crank-Nicolson implicit method is not limited by the same stability criteria as the explicit formulation. However, literature and computation experience demonstrate that severe oscillations and inaccuracies can occur when the time increment exceeds twice the time criteria of the explicit approach. Thus, for the Crank-Nicolson formulation, the time increment should not exceed the following value when a new range of data is being analyzed:

$$\Delta t < \frac{2(\Delta x)^2 (\Delta z)^2}{2EX(\Delta z)^2 + 2EZ(\Delta x)^2 + KD(\Delta x)^2(\Delta z)^2} \dots \dots \dots (6-9)$$

The corresponding one-dimensional limit is

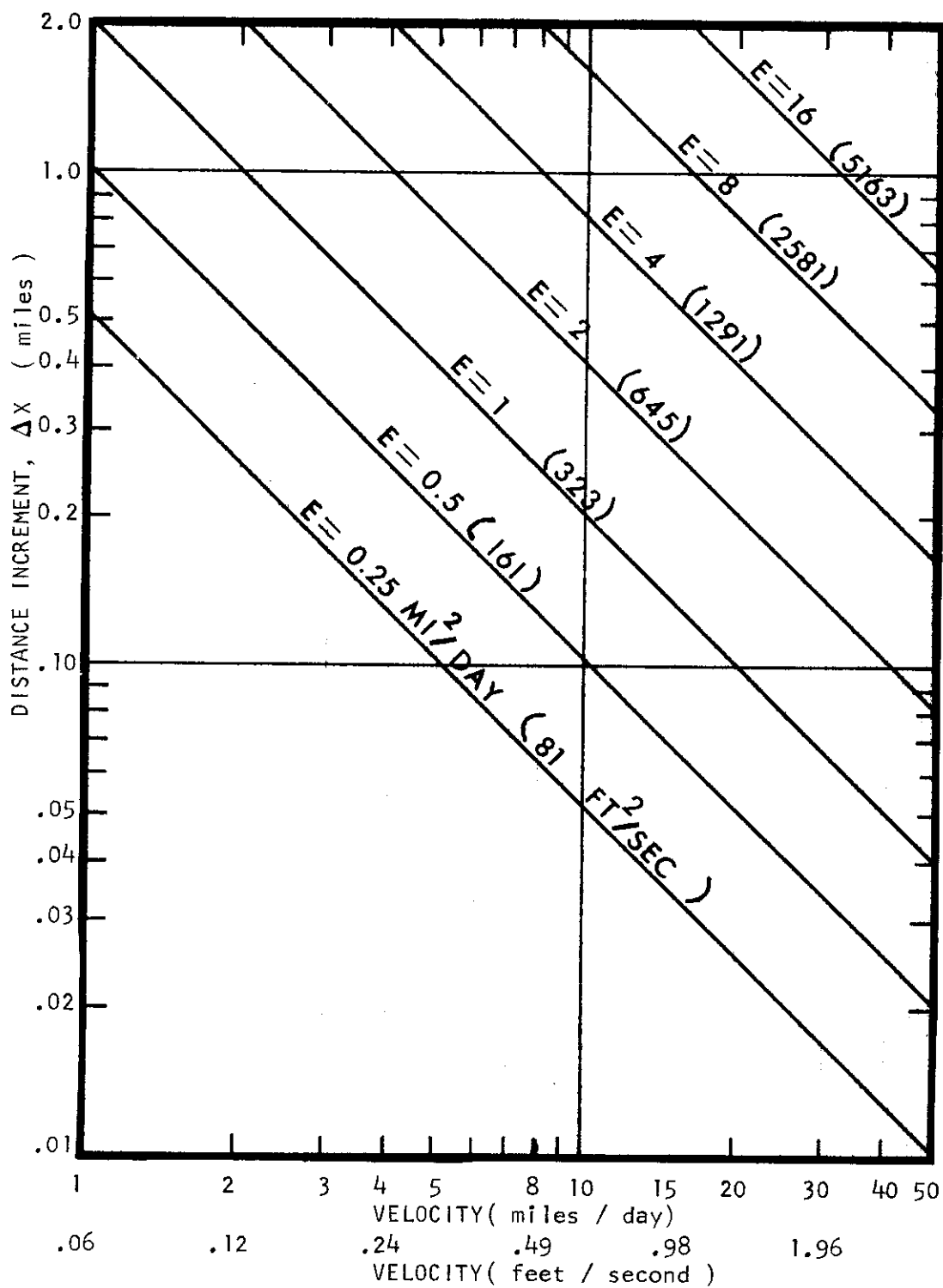


FIGURE 6-1. - DISTANCE INCREMENTS FOR STABILITY

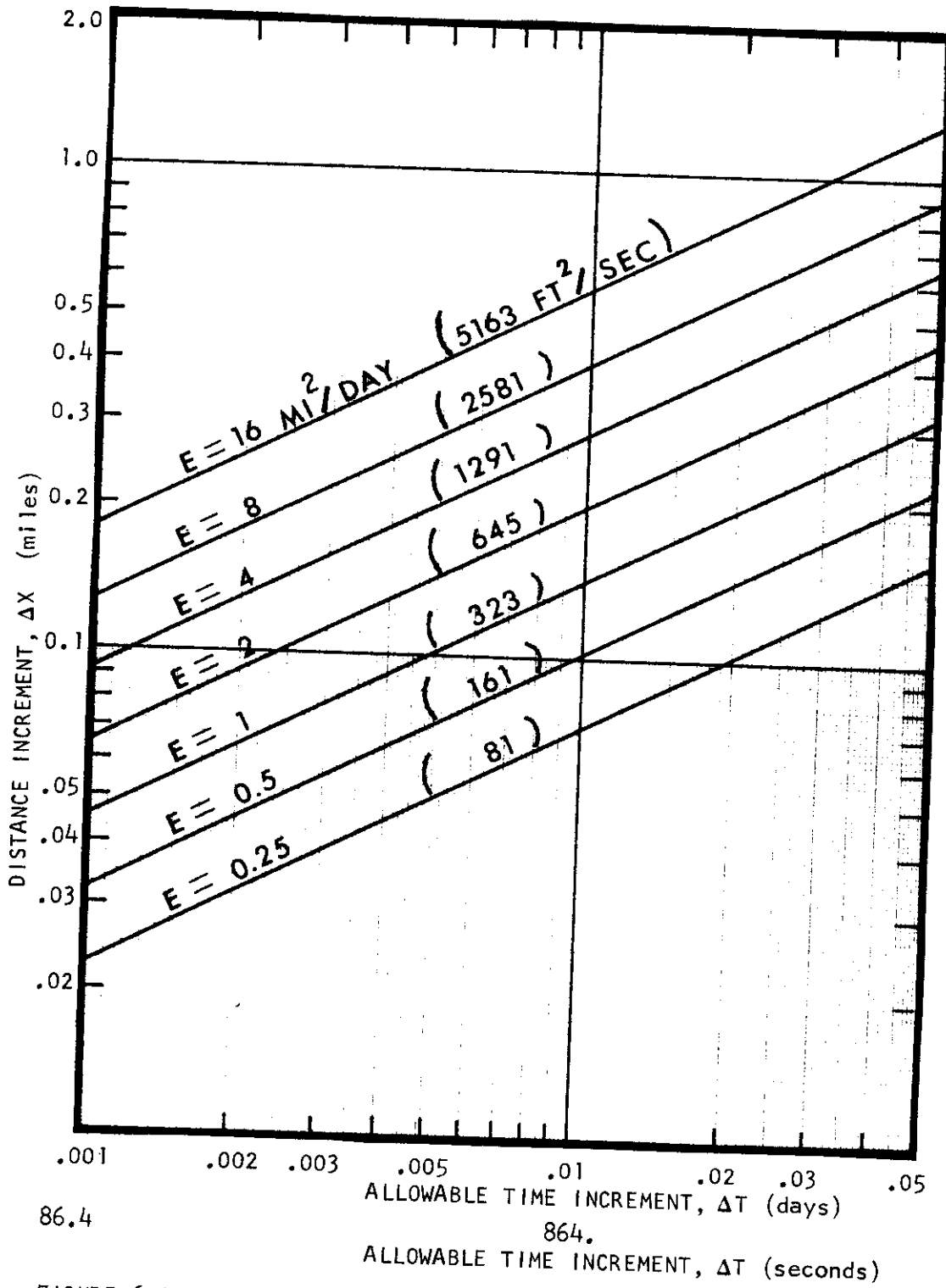


FIGURE 6-2. - TIME INCREMENTS FOR STABILITY

$$\Delta t < \frac{2(\Delta x)^2}{2EX + (\Delta x)^2_{KD}} \dots \dots \dots (6-10)$$

CHAPTER VII

BOUNDARY CONDITIONS

Both the explicit and the implicit methods applied in this research use central difference formulas to represent first and second derivatives. As shown in Equations 5-3 and 5-4, these formulas require grid concentrations to the left and right of the point being analyzed. This requirement presents certain difficulties at the grid boundaries; concentration profiles must be extrapolated one point past the boundary to allow for the application of the central difference formula to a boundary concentration. Since inaccurate boundary concentrations can severely distort the entire concentration profile, care must be taken to choose the proper extrapolation technique. Profile distortion becomes a significant problem when concentration values are changing rapidly near the boundary or when initial concentration profiles are placed near the borders of the grid. One solution to these problems is to enlarge the grid to such a size that the concentrations at the boundary have an insignificant effect on the whole profile; this is the best solution when unlimited computer time is available. When economy requires that the grid must be as compact as possible, more care must be taken to choose a proper extrapolation technique.

Numerous extrapolation techniques were investigated during

the development of computer programs for this research; the most useful techniques are reviewed below.

BOUNDARIES ALLOWING NO TRANSFER

A common condition encountered in estuary modeling is a situation where no transfer of mass is allowed across the air-water surface or through the channel bottom. This condition is handled easily by including a set of "image" points at the border of the grid. At the air-water interface, these image concentrations are set equal to the corresponding concentrations at the surface. At the bottom of the channel, the image concentrations are set equal to the corresponding bottom-most concentrations.

For the two-dimensional explicit formulation, the upper image concentration is labelled $C_{x,1,t+1}$, and the surface concentration is labelled $C_{x,2,t+1}$. Thus, after Equation 5-31 has been applied to all the internal points, $C_{x,1,t+1}$ is set equal to the just-calculated value for $C_{x,2,t+1}$. The same procedure is followed for the bottom concentrations.

For the two-dimensional implicit formulation, a corresponding change must be made to the coefficient matrix represented by Equations 5-38 through 5-40. At the z-boundaries, the appropriate equations for a grid with NZ grid points would be

$$A(2) C_{x,1,t+1} + B(2) C_{x,2,t+1} + G(2) C_{x,3,t+1} = D(2) \dots (7-1)$$

$$A(NZ-1) C_{x,NZ-2,t+1} + B(NZ-1) C_{x,NZ-1,t+1} + G(NZ-1) C_{x,NZ,t+1} = D(NZ-1) \dots \dots \dots (7-2)$$

Since $C_{x,1,t+1}$ and $C_{x,NZ,t+1}$ are image concentrations, these equations become

$$[A(2) + B(2)] C_{x,2,t+1} + G(2) C_{x,3,t+1} = D(2) \dots \dots \dots (7-3)$$

$$A(NZ-1) C_{x,NZ-2,t+1} + [B(NZ-1) + G(NZ-1)]$$

$$C_{x,NZ-1,t+1} = D(NZ-1) \dots \dots \dots (7-4)$$

Equations 7-3 and 7-4 become the first and last equations for the tridiagonal matrix.

These image concentrations block the dispersion of mass across a boundary. Loss of mass by advection is prevented by setting the vertical velocity term equal to zero at the surface and the bottom.

CONSTANT SLOPE EXTRAPOLATION

A convenient method of extrapolating profiles is to assume that a straight line can be extended from the two concentrations nearest the boundary. This approach is fairly accurate for profiles which have flattened out; it is extremely inaccurate for profiles whose slopes are changing rapidly near the boundaries. This

extrapolation technique is frequently used for the Crank-Nicolson implicit formulation because, unfortunately, only the two adjacent concentrations can be manipulated if the form of the tridiagonal matrix is to be preserved.

For the explicit method, concentrations are extended in the horizontal direction according to the following equations:

$$C_{1,z,t+1} = 2 C_{2,z,t+1} - C_{3,z,t+1} \dots \dots \dots (7-5)$$

$$C_{NX,z,t+1} = 2 C_{NX-1,z,t+1} - C_{NX-2,z,t+1} \dots \dots \dots (7-6)$$

Similar equations are applied if extrapolation in the vertical direction is needed.

The coefficient Equations 5-34 through 5-36 for the implicit method are represented as follows at the boundaries in the x-direction:

$$\begin{aligned}
 & [2 A(2) + B(2)] C_{2,z,t+1} + [G(2) - A(2)] C_{3,z,t+1} \\
 & = D(2) \dots \dots \dots (7-7)
 \end{aligned}$$

$$\begin{aligned}
 & [A(NX-1) - G(NX-1)] C_{NX-2,z,t+1} + [B(NX-1) + 2G(NX-1)] \\
 & C_{NX-1,z,t+1} = D(NX-1) \dots \dots \dots (7-8)
 \end{aligned}$$

Extrapolations in the z-direction are represented by similar equations.

EXPONENTIAL EXTRAPOLATIONS

In an idealized estuary, where dispersion and velocity are constant throughout, the following two-dimensional equation can be applied to the instantaneous release of a slug of pollutant (16):

$$C_{x,z,t} = \left(\frac{m}{4\pi t \sqrt{EX \cdot EZ}} \right) \exp \left[- \frac{(x-VX \cdot t)^2}{4EX \cdot t} - \frac{(z-VZ \cdot t)^2}{4EZ \cdot t} \right]. \quad (7-9)$$

where m equals amount per unit depth, and x and z are the distances from the source point. By comparing this equation for concentrations at $x - \Delta x$, $x + \Delta x$, and x, the following relationships are found:

$$C_{x-\Delta x,z,t} = C_{x,z,t} \cdot \exp \left[\frac{2x\Delta x - (\Delta x)^2 - 2\Delta x \cdot VX \cdot t}{4EX \cdot t} \right]. \quad (7-10)$$

$$C_{x+\Delta x,z,t} = C_{x,z,t} \cdot \exp \left[- \frac{2x\Delta x - (\Delta x)^2 + 2\Delta x \cdot VX \cdot t}{4EX \cdot t} \right]. \quad (7-11)$$

Similar expressions can be derived for concentrations at $z+\Delta z$ and $z-\Delta z$.

Equations 7-10 and 7-11 have obvious potential for extrapolating water quality profiles, especially if an idealized estuary is being modeled. For an estuary where average velocities and

dispersion coefficients are used throughout, this extrapolation provides the exact relationship between the final two concentrations on the grid at the appropriate boundaries. For an estuary with varying characteristics, these equations can give good approximations if the proper velocities and dispersion coefficients are provided.

For the explicit method, Equations 7-10 and 7-11 can be applied directly. For the implicit method, these equations can be easily represented in the coefficient matrix. For example, in the x-direction, the first line and last line of the tridiagonal matrix can be represented by Equations 7-12 and 7-13:

$$\left[-A(2) \cdot \exp(\text{EXPON1}) + B(2) \right] C_{2,z,t+1} + G(2) C_{3,z,t+1} = D(2) \dots \dots \dots (7-12)$$

$$A(NX-1) C_{NX-2,z,t} + \left[B(NX-1) + G(NX-1) \cdot \exp(\text{EXPON2}) \right] C_{NX-1,z,t+1} = D(NX-1) \dots \dots \dots (7-13)$$

where EXPON1 stands for the bracketed expression in Equation 7-10 and EXPON2 stands for the bracketed expression in Equation 7-11.

Similar expressions can be inserted for extrapolation in the z-direction.

These equations can be applied only to instantaneous slug releases; formulas could be devised also for other types of profiles, such as a steady-state profile from a constant source.

CONTINUED FRACTIONS AND INVERTED DIFFERENCES

Several interpolation techniques are available in the IBM Scientific Subroutine package (48). With small modifications, these programs can be used for extrapolation. Three of these techniques were investigated during this study: the Aitkens scheme of Lagrange interpolation (ALI); the Aitkens scheme of Hermite interpolation (AHI); and the continued fractions and inverted differences scheme (ACFI). All of these techniques are explained in Hildebrand (43). These techniques were designed to interpolate equations containing low powers of the variable; thus, they are very accurate when applied to gradually changing profiles. However, serious inaccuracies can result when the slope of the profile is changing rapidly near the boundary: extrapolated concentrations can become negative or can become greater than the concentration at the adjacent point.

The continued fractions and inverted differences scheme (ACFI) was found to be the most reliable of these methods and was modified to be used as an extrapolation technique for the explicit

formulation. An advantage of this technique is that the programmer can choose the number of internal points to be included in the extrapolation formula.

EXTRAPOLATION BY PROPORTIONS

A final extrapolation method that was found to be useful was the simple proportion:

$$\frac{C_{3,z,t+1}}{C_{2,z,t+1}} = \frac{C_{2,z,t+1}}{C_{1,z,t+1}} \dots \dots \dots (7-14)$$

For example, when this formula is rearranged to extrapolate in the x-direction, it becomes

$$C_{1,z,t+1} = \frac{(C_{2,z,t+1})^2}{C_{3,z,t+1}} \dots \dots \dots (7-15)$$

This technique can be fairly accurate for extrapolating a curve whose slope is changing rapidly. The proportion technique and the continued fractions technique were both applied to the explicit method; neither was applied to the implicit method.

CHAPTER VIII

MATHEMATICAL MODELS AND SUPPORTING COMPUTER PROGRAMS

The estuary mass transport equation was programmed in finite difference form at several levels of complexity as part of this research. The simplest form deals with an idealized, one-dimensional estuary with constant values for dispersion, velocity, decay, and cross-section. The most complicated form is applicable to an estuary with characteristics that can be varied in two dimensions throughout the estuary. This progression toward increasing complexity brought about a more economical use of computer time. The numerical methods that are used in this study require considerable computational experience in order to establish accuracy and stability guidelines; this experience is more easily gained by working with less complex programs which take less computer time.

The basic computer programs that have been developed during this study are outlined below; they are written in FORTRAN IV and are discussed in detail in the sections which follow:

One-Dimensional Numerical Analysis With Constant Coefficients. - Two finite difference models were developed for evaluating mass transport in an idealized, one-dimensional estuary:

1. IDEAL - I : explicit method;
2. IDEAL - II: implicit method.

One- and Two-Dimensional Numerical Analysis With Varying

Coefficients. - Three finite difference models were developed for evaluating mass transport and oxygen transport in an estuary with varying characteristics:

1. MASSTRANS - I : mass transport by the explicit method;
2. MASSTRANS - II: mass transport by the implicit method;
3. OXTRANS - I : mass and oxygen transport by the explicit method.

Auxiliary Programs. - Several auxiliary computer programs were developed for accuracy and stability analysis:

1. STABLE - I : one-dimensional stability analysis;
2. STABLE - II : two-dimensional stability analysis;
3. EXACT - I : one-dimensional, time-changing, analytical solutions;
4. EXACT - II : two-dimensional, time-changing, analytical solutions for instantaneous releases;
5. PROFILE - I : steady-state, one-dimensional solutions for non-conservative substances; and
6. PROFILE - II : steady-state, one-dimensional solutions for BOD and dissolved oxygen profiles.

IDEAL - I

This program applies an explicit finite difference method to analyze mass transport in a one-dimensional estuary. The model is useful in evaluating transport in a flume or well-mixed estuary where velocity, dispersion, decay, and cross-sectional area can be

considered constant throughout.

The primary use of this program is to test the accuracy of the explicit finite difference method as compared with analytical, closed-form solutions for the estuary mass transport equation. The program can analyze instantaneous releases or steady-state profiles.

The basis of computation is Equation 5-7. IDEAL-I uses an exponential extrapolation or constant slope extrapolation at the boundaries. The program calculates and points out the appropriate stability criteria according to Equations 6-7 and 6-8.

IDEAL - II

This program has the same purpose and application as IDEAL - I except that the Crank-Nicolson implicit finite differences method is applied.

The basis of computation is Equations 5-9 through 5-19. IDEAL - II uses an exponential extrapolation or constant slope extrapolation at the boundaries. The program calculates and prints out the appropriate criteria to avoid severe oscillations; this criteria is based on Equation 6-10.

MASSTRANS - I

This program was developed to analyze estuaries whose characteristics do not vary significantly with width. The width of the estuary may vary throughout but the concentrations of

dissolved materials at each cross-section are considered to be unchanging in the lateral direction. All physical and hydrodynamic characteristics may vary with time and distance in the longitudinal and vertical directions. The program can be applied with equal ease to the two horizontal directions, allowing depth to vary rather than width.

Concentration profiles can be calculated by this program for continuous or instantaneous releases. The input data may include grid dimensions, distance increments, time increments, widths, loading parameters, velocities, dispersion coefficients, decay rates, sedimentation rates, and other source and sink terms. The time increment can be increased or decreased at any time during the calculation of the concentration profile. The distances between grid points can be increased at any time; a routine within the program will choose the appropriate values from the previously calculated profile and will place these values in the desired locations in the new grid system. The number of grid points may be increased or decreased at any time. Likewise, at any time during the calculation, a new set of data for physical and hydrodynamic conditions may be read into the program.

MASSTRANS-I applies an explicit finite difference method to analyze mass transport in an estuary. A user of this program must be familiar with the limitations on accuracy and stability inherent in this type of numerical procedure. A subroutine within the program prints out the proper increments to insure stability and

terminates the program if this criteria is violated by the input parameters. Another subroutine extrapolates concentrations at the boundaries; several methods can be used for these extrapolations depending on the type of profile being analyzed and the choice of the user. A subroutine is also included which prints out error messages and terminates the program if certain inconsistencies occur in the input data.

This program was developed primarily to analyze partially stratified estuaries which have been dredged out to a fairly constant depth at the centerline of the channel; these estuaries are common in the Gulf Coast region. Application of this program to partially stratified estuaries with variable depths would require moderate revisions to the program and would make the program estuary-dependent.

MASSTRANS-I also can be applied to estuaries which are well-mixed in the vertical direction. This option allows for varying width or varying depth and uses most of the routines available to the two-dimensional analysis.

The basis for one-dimensional applications is Equation 5-22. The basis for two-dimensional applications is Equation 5-31. Stability criteria is defined by Equations 6-4 through 6-8.

MASSTRANS-II

The essential difference between this program and MASSTRANS-I

is that a Crank-Nicolson implicit finite difference method is used to solve the equations for estuary mass transport. Otherwise, the programs operate and are applied in the same manner.

The basis for one-dimensional applications is Equations 5-20 through 5-28. The basis for two-dimensional applications is Equations 5-32 through 5-42. Criteria to avoid severe oscillations is generated through the use of Equations 6-9 and 6-10, but the program is not terminated if this criteria is violated.

OXTRANS-I

This program analyzes the relationship between a primary pollutant and a dissolved gas in a partially stratified estuary. The principle application of OXTRANS-I is to the interaction of biochemical oxygen demand (BOD) and dissolved oxygen (DO).

OXTRANS-I performs two basic calculations during each time step. The first calculation is to determine the profile for the primary pollutant, such as BOD, by the same methods available in MASSTRANS-I. The second calculation uses information from the first calculation to determine the values for the dissolved gas.

The input data for OXTRANS-I is the same as for MASSTRANS-I except that additional data is required for reaeration coefficients, aerobic decay rates, anaerobic decay rates, and other oxygen demands or sources. During the calculations for BOD and DO, the program automatically switches to anaerobic decay rates

whenever the concentration of oxygen is zero. Reaeration terms can be applied to any point on the grid and, thus, the effects of mechanical aeration at any point or points can be investigated.

At the option of the user, the secondary calculations can be bypassed and the program can be operated as MASSTRANS-I.

STABLE-I and STABLE-II

These programs allow for a quick analysis of stability for the explicit finite difference method; they can process a large number of data combinations for dispersion, velocity, and decay with each run.

STABLE-I is applied to one-dimensional stability calculations through the use of Equations 6-7 and 6-8. STABLE-I generates the stability criteria for a large number of combinations of velocity and dispersion after one data set is read in.

STABLE-II is applied to two-dimensional stability calculations through the use of Equations 6-4 through 6-6. STABLE-II computes the stability criteria for each set of data that is read into the program.

EXACT-I and EXACT-II

These programs calculate concentration profiles which change with time according to known solutions to the partial differential equation for mass transfer in an estuary. These solutions can be

applied only to an idealized estuary where velocities, dispersion coefficients, decay rate, and cross-sectional area are constant throughout. EXACT-I and EXACT-II were used extensively in this study to evaluate the accuracy of the finite difference methods.

Solutions for three types of loading conditions were programmed for EXACT-I: a constant concentration defined at a source point; an instantaneous release; a continuous discharge at a point.

Constant Concentration. - The following solution is applicable to a condition where the concentration, C_0 , is constant at and above a certain point in the estuary at time zero (51):

$$\frac{C}{C_0} = \frac{1}{2} \exp\left(\frac{u \cdot x}{E}\right) \operatorname{erfc}\left(\frac{x + u \cdot t}{2\sqrt{E \cdot t}}\right) + \frac{1}{2} \operatorname{erfc}\left(\frac{x - u \cdot t}{2\sqrt{E \cdot t}}\right) \dots (8-1)$$

where erfc is the coerror function, u is net velocity, E is the dispersion coefficient, x is distance, and t is time.

A second solution is applicable when the velocity equals zero:

$$\frac{C}{C_0} = \operatorname{erfc}\left(\frac{x}{2\sqrt{E \cdot t}}\right) \dots (8-2)$$

Instantaneous Release. - The following equation represents the concentration resulting from an instantaneous release of amount m per unit cross-sectional area (16).

$$C = \left(\frac{m}{\sqrt{4\pi \cdot E \cdot t}}\right) \exp\left[-\frac{(x - u \cdot t)^2}{4 \cdot E \cdot t}\right] - K_d \cdot t \dots (8-3)$$

This equation was used extensively to test the accuracy of IDEAL-I and IDEAL-II.

Continuous Release. - The following equation is applicable to a conservative substance released at a rate m per unit cross-sectional area per unit time into an estuary with no net velocity (13).

$$C = \left(\frac{m \cdot t^{\frac{1}{2}}}{\sqrt{\pi \cdot E}} \right) \exp \left[- \frac{x^2}{4 \cdot E \cdot t} \right] - \left(\frac{mx}{2E} \right) \operatorname{erfc} \left[\frac{x}{2\sqrt{E \cdot t}} \right] \dots (8-4)$$

EXACT-II was used extensively to test the accuracy of MASSTRANS-I and MASSTRANS-II. The appropriate equation for two-dimensional convective-dispersion from an instantaneous release of amount m per unit depth is the following:

$$C = \left(\frac{m}{4\pi t \sqrt{E_x \cdot E_y}} \right) \exp \left[- \frac{(x - V_x t)^2}{4 \cdot E_x \cdot t} - \frac{(y - V_y t)^2}{4 \cdot E_x \cdot E_y} - K_d \cdot t \right] \dots (8-5)$$

where x and y represent perpendicular axes, V is velocity, and E is dispersion.

PROFILE I and PROFILE II

PROFILE-I calculates the steady-state profile for a non-conservative substance which is continuously released into an idealized estuary at the rate of W pounds per day. PROFILE-II calculates the steady-state profiles for BOD, oxygen deficit, and

dissolved oxygen (DO). These programs were used to test steady-state applications of MASSTRANS-I, MASSTRANS-II, and OXTRANS-I.

The following equations were programmed (67):

$$\text{BOD} = C_o \cdot \exp \left[\frac{u \cdot x}{2E} (1 \pm m_1) \right] \dots \dots \dots (8-6)$$

$$\text{DEFICIT} = \frac{K_d \cdot W}{(K_2 - K_d)Q} \left\{ \frac{1}{m_1} \exp \left[\frac{u \cdot x}{2E} (1 \pm m_1) \right] - \frac{1}{m_2} \exp \left[\frac{u \cdot x}{2E} (1 \pm m_2) \right] \right\} \dots \dots \dots (8-7)$$

$$\text{DO} = C_{\text{SAT}} - \text{DEFICIT} \dots \dots \dots (8-8)$$

in which

$$m_1 = \sqrt{1 + \frac{4K_d \cdot E}{u^2}} \dots \dots \dots (8-9)$$

$$m_2 = \sqrt{1 + \frac{4K_2 \cdot E}{u^2}} \dots \dots \dots (8-10)$$

$$C_o = \frac{W}{Qm_1} \dots \dots \dots (8-11)$$

and Q equals flowrate in pounds per day, K_d equals the decay rate,

K_2 equals the reaeration rate, u is the net velocity, and C_{SAT} is the saturation value for dissolved oxygen. The positive sign refers to values of $x < 0$ and the negative sign refers to values of $x > 0$. Care must be taken to insure consistency of units. An equation similar to Equation 8-7 was applied for the special case where $K_d = K_2$.

CHAPTER IX

ANALYSIS OF ACCURACY

Establishing the accuracy of a modeling technique is of utmost importance before that technique can be used as a basis for engineering decisions. The most straightforward and appropriate manner in which to test the accuracy of finite difference estuary models is to compare their results with exact solutions to the partial differential equations for mass transport in an estuary; these solutions are readily available for certain idealized types of estuary behavior. Such a comparison should be a requirement for any type of estuary model before the model is applied to the oftentimes inexact data of real estuaries.

The finite difference models were tested for the following cases for which exact, analytical solutions are available: one-dimensional, steady-state profiles for non-conservative substances and dissolved oxygen; one-dimensional, instantaneous release of a slug load; and two-dimensional, instantaneous release of a slug load. The accuracy characteristics of the two-dimensional models were found to be the same as the one-dimensional models and, therefore, most of the testing was done on the one-dimensional models in order to conserve computer time.

STEADY-STATE PROFILES

Steady-state profiles are obtained in the finite difference

models by setting a concentration at a point and then iterating the calculations until an unchanging profile develops. This is an inefficient way of obtaining such a profile but, as it turns out, a very accurate way.

The exact analytical solutions for steady-state profiles were obtained through the computer programs PROFILE-I and PROFILE-II which apply Equations 8-6 through 8-8.

Profiles for Non-Conservative Substances. - Excellent accuracy for steady-state profiles can be obtained by finite difference methods, even when large distance increments are used. Comparison between an exact solution and the profile calculated by the explicit method is shown in Table 9-1. The input values were as follows: dispersion coefficient = $10.0 \text{ mi}^2/\text{day}$; velocity = 1.0 mile / day ; decay rate (base e) = $0.25/\text{day}$. For the explicit program, IDEAL-I, the distance increment was 2.0 miles and the time increment was 0.01 days. The implicit solution for the same increments differed from the explicit solution by only one unit in the fourth decimal place. A constant slope extrapolation was applied at the downstream boundary and this appears to significantly affect the accuracy of only the last four concentrations.

In the Table 9-1, two columns are included which refer to the time at which steady-state was achieved in the finite difference model at different points. The first of these columns refers to the time it took for the solution to stabilize to four

TABLE 9-1.-COMPARISON BETWEEN EXACT AND FINITE DIFFERENCE SOLUTIONS

Distance (miles)	Exact (ppm)	Finite Difference (ppm)	Days till steady-state	
			4 places	1 place
0	10.0000	10.0000		
2	7.9321	7.9398	23.40	4.96
4	6.2919	6.3040	24.30	7.76
6	4.9908	5.0052	25.25	8.81
8	3.9588	3.9740		
10	3.1402	3.1552	27.60	11.86
12	2.4908	2.5051		
14	1.9758	1.9888		
16	1.5672	1.5789		
18	1.2431	1.2534		
20	0.9861	0.9948	30.95	
22	0.7822	0.7893		
24	0.6204	0.6258		
26	0.4921	0.4956		
28	0.3904	0.3915		
30	0.3096	0.3078		
32	0.2456	0.2396		
34	0.1948	0.1830	32.15	
36	0.1548	0.1342		
38	0.1226	0.0894		
40	0.0972	0.0446	32.65	

decimal points. The second of these columns refers to the time needed to reach a point in the calculations where the values remain essentially unchanged to one decimal point. Since the time increment was 0.01 days, one day in these columns would represent 100 time iterations.

The implicit method tended to take 1.75 times as much computer time as the explicit method for the same time and distance increments. When a time step of 0.1 days was used, the steady-state solutions were only slightly less accurate for both finite difference methods than for a time increment of 0.01 days.

BOD - DO Profiles. - OXTRANS-I, the explicit finite difference model for dissolved oxygen, was investigated to determine its ability to calculate steady-state profiles for BOD and dissolved oxygen. Table 9-2 shows a comparison between the exact solution and the finite difference calculation for BOD and DO profiles based on the following input data: dispersion coefficient = $400 \text{ ft}^2/\text{sec}$. ($1.24 \text{ mi}^2/\text{day}$); velocity = $0.2 \text{ ft}/\text{sec}$ ($3.27 \text{ miles}/\text{day}$); decay rate (base e) = $0.23/\text{day}$; reaeration rate = $0.1/\text{day}$; $C_{\text{SAT}} = 8.0 \text{ ppm}$; and $C_0 = 8.817$. OXTRANS-I used a distance increment of 0.5 miles and a time increment of 0.05 days.

OXTRANS-I provided excellent accuracy downstream from the source point at Mile 3.5. The extrapolation at the downstream boundary was based on a continued fractions and inverted differences scheme and this method provided outstanding accuracy at the final

TABLE 9-2.-BOD AND DO PROFILES

Mile	BOD (ppm)		DO (ppm)	
	Exact	OXTRANS-I	Exact	OXTRANS-I
0.0	0.001	0.000	8.00	8.00
0.5	0.003	0.001	8.00	8.00
1.0	0.010	0.003	8.00	8.00
1.5	0.039	0.014	7.99	8.00
2.0	0.152	0.068	7.98	7.99
2.5	0.588	0.345	7.93	7.96
3.0	2.276	1.745	7.81	7.85
3.5	8.817	8.817	7.56	7.55
4.0	8.520	8.520	7.27	7.27
4.5	8.233	8.233	7.00	7.00
5.0	7.956	7.956	6.75	6.74
5.5	7.688	7.688	6.50	6.50
6.0	7.429	7.430	6.27	6.26
6.5	7.179	7.179	6.05	6.04
7.0	6.937	6.938	6.84	5.84
7.5	6.704	6.704	5.64	5.64
8.0	6.478	6.478	5.46	5.45
8.5	6.260	6.260	5.28	5.28
9.0	6.049	6.050	5.12	5.11
9.5	5.845	5.846	4.96	4.96
10.0	5.649	5.649	4.81	4.81
10.5	5.458	5.459	4.67	4.67
11.0	5.275	5.275	4.54	4.54
11.5	5.097	5.098	4.42	4.42
12.0	4.925	4.926	4.31	4.30
12.5	4.759	4.760	4.20	4.19
13.0	4.599	4.600	4.10	4.09
13.5	4.444	4.445	4.00	4.00
14.0	4.295	4.295	3.92	3.91
14.5	4.150	4.151	3.84	3.83
15.0	4.010	4.011	3.76	3.76
15.5	3.875	3.876	3.69	3.69
16.0	3.745	3.746	3.63	3.63
16.5	3.619	3.619	3.57	3.57
17.0	3.497	3.498	3.52	3.51
17.5	3.379	3.380	3.47	3.47
18.0	3.265	3.266	3.43	3.42
18.5	3.156	3.156	3.39	3.38

point, Mile 18.5. The steady-state solution at Mile 4.0 stabilized after 60 time iterations and the solution at Mile 18.0 stabilized after 180 time iterations.

The profile upstream of the source point is fairly inaccurate, but this problem did not adversely affect the downstream profile. The inaccuracy in the upstream profile was caused by attempting to use a large distance increment in an area where the slope of the concentration profile was changing rapidly with distance. This rapid change in the slope of the steady-state profile is produced when the velocity is in the opposite direction from dispersion. This inaccuracy can be corrected by using a smaller distance increment in areas where concentration profiles have steep slopes. Tables 9-3 and 9-4 demonstrate the change in accuracy when the distance increment is changed from 0.5 miles to 0.1 miles. The time increment was changed to 0.0025 days to insure stability. Even better accuracy would be attained if the distance increment were decreased even further.

These results demonstrate that the choice of a distance increment has a limiting effect on the accuracy which can be obtained. For steady-state profiles, several distance increments should be tried in order to insure that an accurate profile has been calculated.

ONE-DIMENSIONAL INSTANTANEOUS RELEASE OF A SLUG LOAD

An important application of the finite difference models that

TABLE 9-3.-ACCURACY FOR BOD PROFILES FOR SEVERAL DISTANCE INCREMENTS

Mile	Exact	OXTRANS-I $\Delta x = 0.5$ $\Delta t = 0.05$	OXTRANS-I $\Delta x = 0.1$ $\Delta t = 0.0025$
0.0	0.001	0.000	-
0.5	0.003	0.001	-
1.0	0.010	0.003	-
1.5	0.039	0.014	0.029
2.0	0.152	0.068	0.140
2.5	0.588	0.345	0.571
3.0	2.276	1.745	2.252
3.5	8.817	8.817	8.817

TABLE 9-4.-ACCURACY FOR DISSOLVED OXYGEN PROFILES FOR SEVERAL DISTANCE INCREMENTS

Mile	Exact (ppm)	OXTRANS-I $\Delta x = 0.5$ $\Delta t = 0.05$	OXTRANS-I $\Delta x = 0.1$ $\Delta t = 0.0025$
0.0	8.00	8.00	-
0.5	8.00	8.00	-
1.0	8.00	8.00	-
1.5	7.99	8.00	8.00
2.0	7.98	7.99	7.98
2.5	7.93	7.96	7.94
3.0	7.81	7.85	7.83
3.5	7.56	7.55	7.55

have been developed in this study will be to calculate the concentration profiles resulting from slug loads released into an estuary. Such loading might result from spills of polluting materials or dye releases. Therefore, a considerable amount of time has been spent during this study to evaluate the ability of the models to predict accurately the concentration profiles produced by slug loads.

Finite difference methods are generally regarded as being accurate for diffusion predictions, as in heat transfer problems. However, when velocity is also included as a parameter, a phenomenon known as "numerical dispersion" results. This phenomenon caused significant difficulties in the finite difference methods used in the San Francisco Bay-Delta model (72). Numerical dispersion is related to the fact that the distance increment used in a finite difference model is rarely a multiple of the velocity at which the material is moving.

In this study, numerical dispersion was found to produce distortions and severe inaccuracies if improper choices were made for distance increments and time increments. However, excellent accuracy was achieved and the effects of numerical dispersion were minimized when a small enough distance increment was chosen; for the dispersion coefficients and velocities used in this study, good accuracy was obtained for horizontal distance increments between 0.1 miles and 0.25 miles.

When comparing finite difference solutions with exact analytical solutions, initial concentration values for the model must be chosen to represent accurately the mass of the slug load. These initial values can be chosen in two ways: either by obtaining an initial distribution of values at several points from the analytical solution or by calculating an equivalent "slug" value for a single starting value in the model. The analytical solution, as represented by Equation 8-3, calculates the concentration profile for a mass m per unit cross-sectional area. For a single initial concentration, the finite difference model "sees" this mass as a triangle, as shown in Figure 9-1, below:

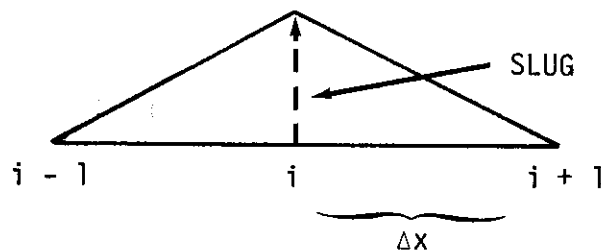


FIGURE 9-1.-SLUG LOAD "SEEN" BY FINITE DIFFERENCE MODEL

The value for SLUG is equal to $m/\Delta x$, where the m used for accuracy investigations was 10^7 . Thus, for a distance increment of 0.25 miles (1,320 feet), $SLUG = 10^7/1320 = 7575.8$. For a distance increment of 0.10 miles (528 feet), $SLUG = 10^7/528 = 18939.39$. The Δx must be expressed in feet.

Analysis of Dispersion Without Velocity. - When using a single

value of a finite difference model, distortions develop in the early calculations of the profile. When a sufficiently small distance increment is chosen along with a proper time increment, these distortions are rapidly damped out. Tables 9-5 and 9-6 demonstrate the ability of the models to converge on the correct solution. For this example, the dispersion coefficient = $1 \text{ mi}^2/\text{day}$ ($322.7 \text{ ft}^2/\text{sec}$), the time increment = 0.01 days, the distance increment = 0.25 miles, and SLUG = 7575.8 at time = 0.0 days. The velocity and decay rate were set equal to zero. The profiles were essentially symmetrical; therefore only half of the profiles are shown.

Effect of Velocity on the Concentration Profile from a Slug Load.

- Table 9-7 shows the effects of a velocity of 5 miles/day ($.306 \text{ ft}/\text{sec}$) on the distribution described in the previous example. Velocity distorts the concentration profiles; however, this distortion is minor when proper time and distance increments are chosen, except at the leading and trailing edges. A comparison between the exact profile after one day elapsed time and the profile predicted by the implicit method is shown in Figure 9-2, for a distance increment of 0.25 miles. Although the explicit method is more accurate for dispersion alone, the implicit method appears to be more accurate when velocity is included.

Error Analysis for a Slug Load. - The finite difference models

TABLE 9-5.-DISTRIBUTION FROM A SLUG LOAD AT TIME = 0.2 DAYS

Mile	Exact	Explicit	Implicit
0.00	1194.7	1196.1	1219.9
0.25	1104.9	1105.7	1120.4
0.50	874.0	873.8	870.6
0.75	591.4	590.5	577.0
1.00	342.3	341.6	329.8
1.25	169.4	169.3	164.5
1.50	71.75	71.88	72.46
1.75	25.98	26.15	28.49
2.00	8.05	8.15	10.10
2.25	2.13	2.17	3.26
2.50	0.48	0.49	0.96
2.75	0.09	0.09	0.26
3.00	0.02	0.02	0.04

TABLE 9-6.-DISTRIBUTION FROM A SLUG LOAD AT TIME = 1.0 DAYS

Mile	Exact	Explicit	Implicit
0.00	534.3	534.3	536.2
0.25	526.0	526.1	527.8
0.50	501.9	502.0	503.3
0.75	464.2	464.2	464.9
1.00	416.1	416.1	416.1
1.25	361.5	361.5	360.9
1.50	304.4	304.4	303.3
1.75	248.5	248.4	247.0
2.00	196.6	196.5	195.1
2.25	150.7	150.6	149.3
2.50	112.0	112.0	110.8
2.75	80.66	80.63	79.77
3.00	56.31	56.29	55.69

TABLE 9-7.-ONE DIMENSIONAL CONVECTIVE-DISPERSION OF A SLUG LOAD AFTER ONE DAY

Mile	Exact	IDEAL-II	IDEAL-I	
		(Implicit)	(Explicit)	(Explicit)
		$\Delta T = .01$ Days	$\Delta T = .01$	$\Delta T = .0025$
-5.00 (Input point)	1.03	0.54	0.40	0.51
-4.75	1.90	1.11	0.82	1.03
-4.50	3.38	2.16	1.62	2.02
-4.25	5.84	4.07	3.07	3.81
-4.00	9.79	7.34	5.61	6.89
-3.75	15.88	12.73	9.87	12.00
-3.50	25.00	21.21	16.74	20.08
-3.25	38.10	33.97	27.35	32.32
-3.00	56.31	52.34	43.05	50.03
-2.75	80.66	77.55	65.30	74.54
-2.50	112.0	110.6	95.46	106.9
-2.25	150.7	151.9	134.5	147.7
-2.00	196.6	201.0	182.7	196.7
-1.75	248.5	256.1	239.2	252.5
-1.50	304.4	315.7	301.9	312.6
-1.25	361.5	375.1	364.5	373.4
-1.00	416.1	430.5	431.4	430.8
-0.75	464.2	477.6	488.4	480.2
-0.50	501.9	512.4	533.4	517.3
-0.25	526.0	532.1	561.9	539.0
-0.00 (Peak)	534.3	535.1	571.1	543.4
0.25	526.0	521.6	560.1	530.4
0.50	501.9	492.9	530.1	501.5
0.75	464.2	452.1	484.3	459.6
1.00	416.1	402.5	427.0	408.4
1.25	361.5	348.2	363.5	352.1
1.50	304.4	292.9	298.7	294.7
1.75	248.5	239.6	237.1	239.5
2.00	196.6	190.8	181.7	189.1
2.25	150.7	147.9	134.5	145.2
2.50	112.0	111.7	96.15	108.4
2.75	80.66	81.95	66.71	78.58
3.00	56.31	57.21	46.57	54.86

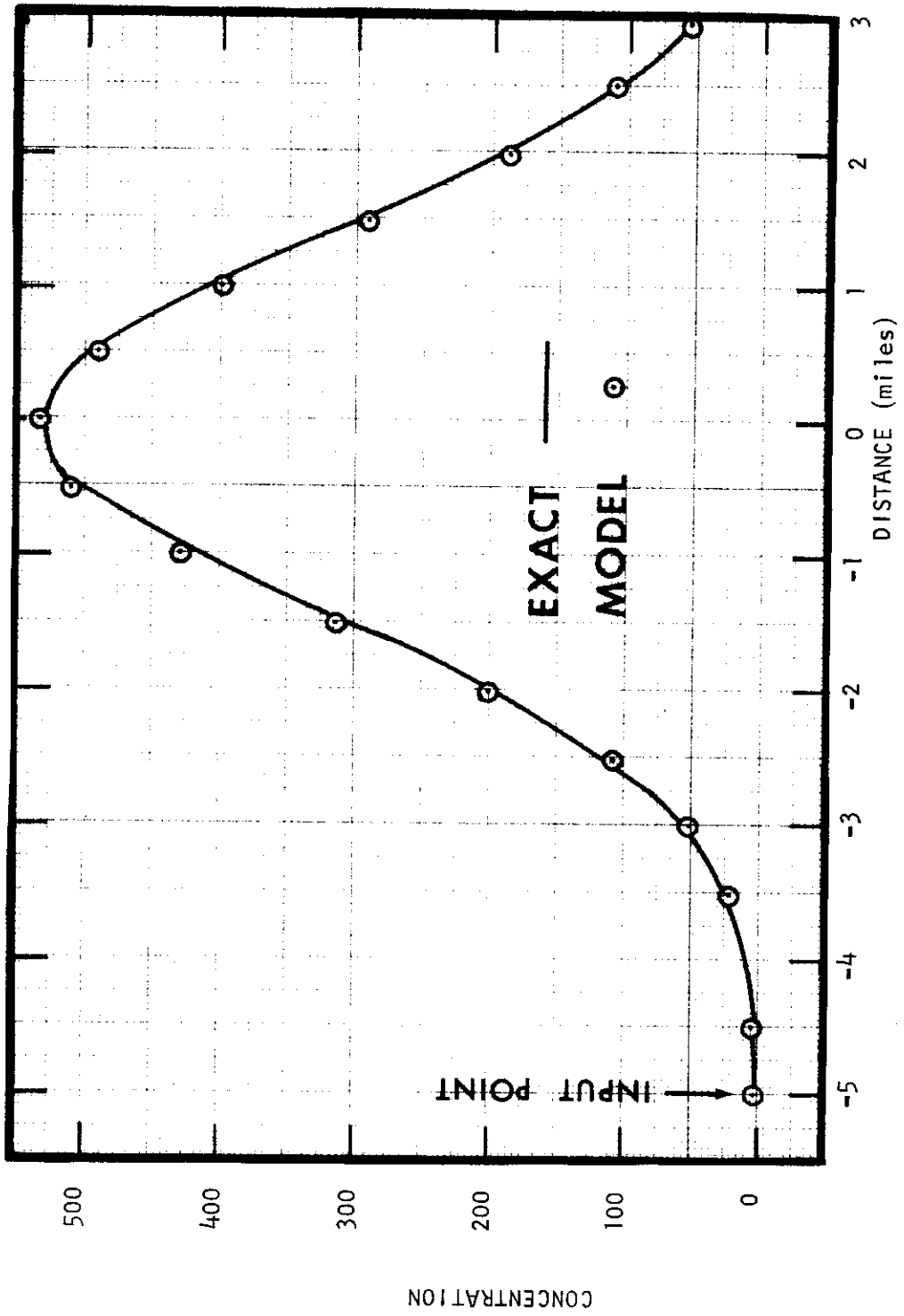


FIGURE 9-2. - PROFILE FROM A SLUG LOAD

developed in this study should be useful in predicting the concentration profiles resulting from slug loads; therefore, a more extensive analysis of stability and accuracy characteristics of the finite difference methods was believed to be worthwhile.

The explicit and implicit models were applied to a slug loading using several different time increments. The profiles were calculated until 0.6 days had elapsed and then the results were compared. The following parameters were the same for all runs: distance increment = 0.1 miles; dispersion coefficient = $1.0 \text{ mi}^2/\text{day}$ ($322.7 \text{ ft}^2/\text{sec}$); velocity = 5.0 miles/day ($.306 \text{ ft}/\text{sec}$); and decay rate = 0.0. Two initial starting conditions were applied. One starting condition placed a single concentration value of 18939.39 ppm at the source point at time = 0.0 days. The second starting condition applied a known concentration profile calculated by the analytical solution for an elapsed time of 0.1 days; this known initial distribution included 28 points and is shown in Table 9-8.

The accuracy of the computed profiles were compared for a series of different time increments. The maximum time increment for stability of the explicit method was calculated to be 0.005 days.

Table 9-9 shows the profiles for the Crank-Nicolson implicit method with a time increment of 0.002 days and the profile for the explicit method with a time increment of

TABLE 9-8.-INITIAL CONCENTRATION VALUES

<u>Mile</u>	<u>Concentration (ppm)</u>
-3.0	904.3
-2.9	1132.5
-2.8	1349.1
-2.7	1528.7
-2.6	1647.8
-2.5	1689.5
-2.4	1647.8
-2.3	1528.7
-2.2	1349.1
-2.1	1132.5
-2.0	904.3
-1.9	686.9
-1.8	496.3
-1.7	341.1
-1.6	223.0
-1.5	138.7
-1.4	82.04
-1.3	46.17
-1.2	24.71
-1.1	12.58
-1.0	6.09
-0.9	2.81
-0.8	1.23
-0.7	0.51
-0.6	0.20
-0.5	0.08
-0.4	0.03
-0.3	0.01

TABLE 9-9.-PROFILES CALCULATED BY FINITE DIFFERENCE METHODS FOR A SLUG LOAD; ELAPSED TIME = 0.6 DAYS

Distance from peak (miles)	Exact	Implicit $\Delta T = .002$ Days	% Error	Explicit $\Delta T = .0005$ Days	% Error
-2.5	51.0	50.1	-1.7	49.7	-2.5
-2.4	62.6	61.7		61.2	
-2.3	76.1	75.2		74.7	
-2.2	91.8	91.0		90.4	
-2.1	109.8	109.2		108.5	
-2.0	130.3	129.8	-.37	129.0	-.97
-1.9	153.3	153.0		152.2	
-1.8	178.8	178.9	~0.0	177.9	
-1.7	206.9	207.3		206.3	~0.0
-1.6	237.4	238.2		237.1	
-1.5	270.1	271.3		270.2	
-1.4	304.8	306.4		305.4	
-1.3	341.1	343.2		342.1	
-1.2	378.5	381.0		380.1	
-1.1	416.6	419.5		418.7	
-1.0	454.7	457.9	+ .70	457.2	+ .55
-0.9	492.2	495.6		495.1	
-0.8	528.3	531.9		531.6	
-0.7	562.4	566.0		566.0	
-0.6	593.7	597.2		597.5	
-0.5	621.5	624.8		625.4	
-0.4	645.3	648.2		649.1	
-0.3	664.4	666.8		668.0	
-0.2	678.3	680.2		681.7	
-0.1	686.9	688.1		689.9	
+0.0	689.7	690.2	+ .07	692.3	+ .37
+0.1	686.9	686.6	~0.0	688.8	
+0.2	678.3	677.4		679.7	
+0.3	664.4	662.7		665.1	
+0.4	645.3	642.9		645.4	~0.0
+0.5	621.5	618.6		621.0	
+0.6	593.7	590.4		592.7	
+0.7	562.4	558.7		560.9	
+0.8	528.3	524.5		526.5	
+0.9	492.2	488.4		490.3	
+1.0	454.7	451.2	-.77	453.0	-.39

0.0005 days. The percentage of error is shown at certain points. Similar analyses of profiles for other time increments are summarized in Tables 9-10, 9-11, 9-12, and 9-13. In these four tables, execution time is included to demonstrate the accuracy which can be obtained for a certain expenditure of computer time.

The percentage of error for various time increments in Table 9-10 are shown graphically in Figure 9-3. Error in terms of concentration is shown for the same data in Figure 9-4; the concentrations should be compared with the peak concentration of 689.7 ppm for 0.6 days lapsed time.

Discussion of Accuracy for Slug Loads. - The analyses in this section demonstrate that the Crank-Nicolson implicit method generally provides better accuracy and more freedom from oscillations than the explicit method. In addition, for one-dimensional applications the implicit method requires less computer time than the explicit method for an accurate solution. However, excellent accuracy also can be obtained by the explicit method when the time increment is chosen properly. The choice of a distance increment is also important in limiting the degree of accuracy which can be obtained by either method.

The accuracy of the explicit method improves noticeably for smaller and smaller time increments. This accuracy will have a lower limit because round-off errors become more significant as

TABLE 9-10.-ACCURACY ANALYSIS: EXPLICIT METHOD.
 PROFILE AFTER 0.6 DAYS FOR AN INITIAL SLUG LOAD OF 18939.39

Time increment in days ΔT	Execution time for 0.6 days (seconds)	% Error at peak	No error distance from peak (miles)	% Error at certain distances from peak +1.0 miles -1.0 miles -2.0 miles -2.5 miles	Oscillations and stability
0.0005	33.72	+0.37	+0.4	-0.39 +0.55 -0.97	Stable
0.001	17.90	+0.67	+1.0	0.00 +0.40 -1.6	Stable
0.00167	10.62	+1.06	-	+0.52 +0.19 -2.4	Stable
0.002	9.69	+1.25	-	+0.78 +0.07 -2.8	Stable
0.003	6.01	+1.85	-	+1.60 -0.26 -4.0	Stable
0.004	4.55	+2.45	-	+2.40 -0.62 -5.2	Early oscillations quickly damped out
0.0045	4.20	+2.80	-	+1.0 -1.9 -7.3	Early oscillations damped out
0.005	3.84	+205.70	-	- - -	Unstable oscillations

TABLE 9-11.-ACCURACY ANALYSIS: EXPLICIT METHOD.
 PROFILE AFTER 0.6 DAYS FOR AN INITIAL KNOWN DISTRIBUTION AT 28 POINTS

Time increment in days ΔT	Execution time for 0.5 days (seconds)	% Error at peak	No error distance from peak (miles)	% Error at certain distances from peak miles miles miles miles	Oscillations and stability
0.0005	29.37	+0.42	+0.5	-0.41 +0.82 +0.28	Stable
0.002	7.63	+1.16	+1.2	+0.32 +0.45 -1.20	Stable
0.004	3.59	+2.16	+1.3	+1.30 -0.08 -3.10	Stable
0.005	3.37	+2.67	+1.3	+1.80 -0.36 -4.10	Stable
0.006	3.15	-	-	- - -	Unstable oscillations after .2 days

TABLE 9-12.-ACCURACY ANALYSIS: CRANK-NICOLSON IMPLICIT METHOD.
PROFILE AFTER 0.6 DAYS FOR AN INITIAL SLUG LOAD OF 18939.39

Time increment in days ΔT	Execution time for 0.6 days (seconds)	% Error at peak	No error distance from peak (miles)	% Error at certain distance from peak miles miles miles miles	Oscillations and stability
.002	16.83	+ .07	+0.1	-1.8	-1.7
.005	6.81	+ .13	+0.1	-1.8	-1.7
.01	3.80	+ .17	+0.1	-1.8	-1.9
.015	2.67	+ 2.00	-	-2.4	-1.7
.02	1.92	+24.00	-	-	Early oscillations severely dis- tort profile

TABLE 9-13.-ACCURACY ANALYSIS: CRANK-NICOLSON IMPLICIT METHOD.
 PROFILE AFTER 0.6 DAYS FOR AN INITIAL KNOWN DISTRIBUTION AT 28 POINTS

Time increment in days ΔT	Execution time for 0.5 days (seconds)	% Error at peak	No error distance from peak (miles)	% Error at certain distance from peak +1.0 miles -1.0 miles -2.0 miles -2.5 miles	Oscillations and stability
.001	30.93	+ 0.22	+ .2	- 0.61 +0.98 +0.81 + 0.45	Stable
.002	14.54	+ 0.17	+ .2	- 0.66 +0.94 +0.76 + 0.39	Stable
.01	5.66	+ 0.26	+ .2	- 0.70 +0.99 +0.70 + 0.29	Stable
.02	3.40	+ 0.45	+ .3	- 0.94 +1.10 +0.43 - 0.12	Stable
.05	1.94	+ 1.80	+ .4	- 2.70 +1.80 -1.40 - 2.90	Stable
.10	1.18	+ 6.90	+ .4	-10.40 +4.00 -9.20 -11.80	Stable
.20	0.61	+19.50	-	-	Severe oscillations and distorted profile

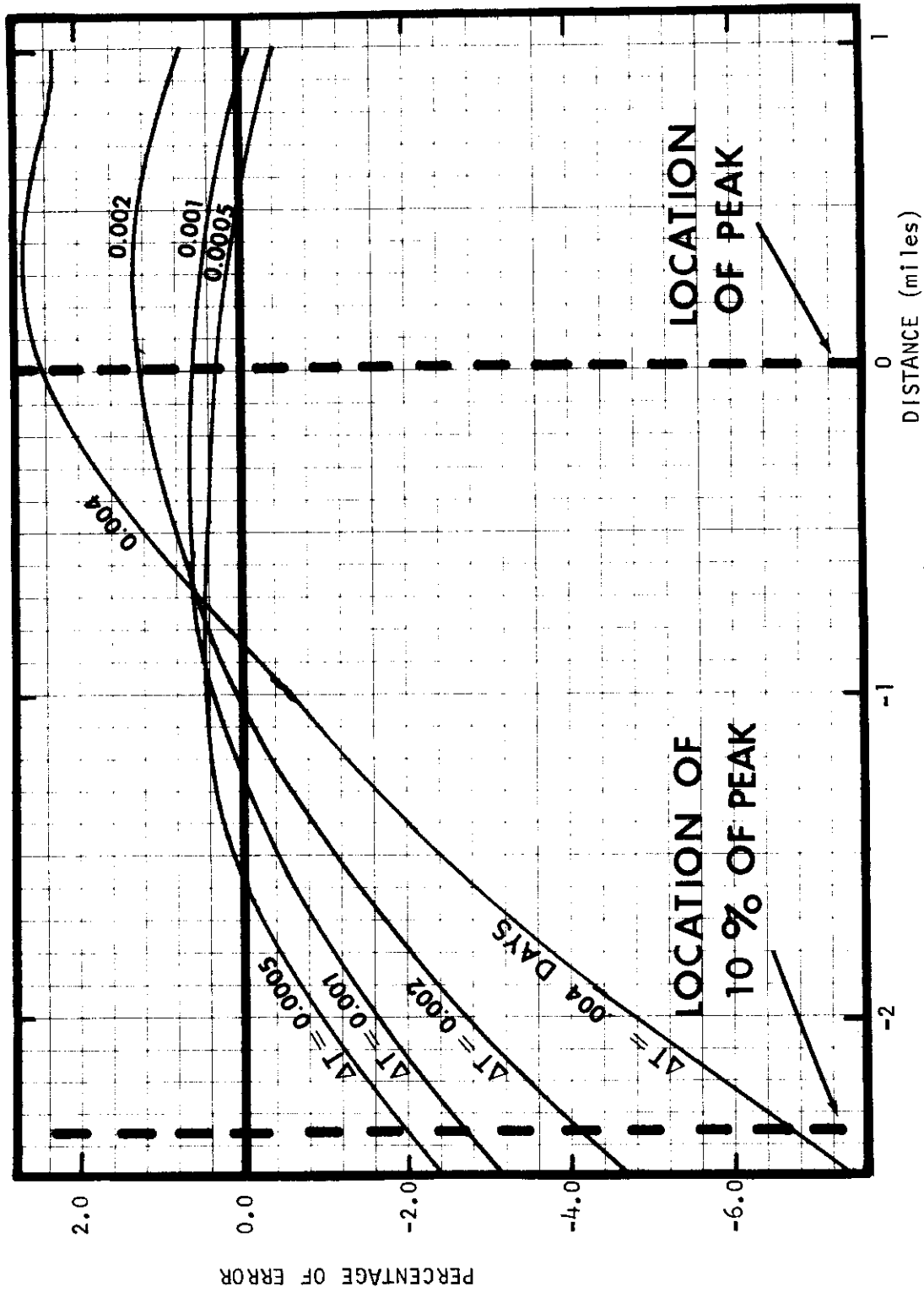


FIGURE 9-3. - PERCENTAGE OF ERROR (see TABLE 9-10)

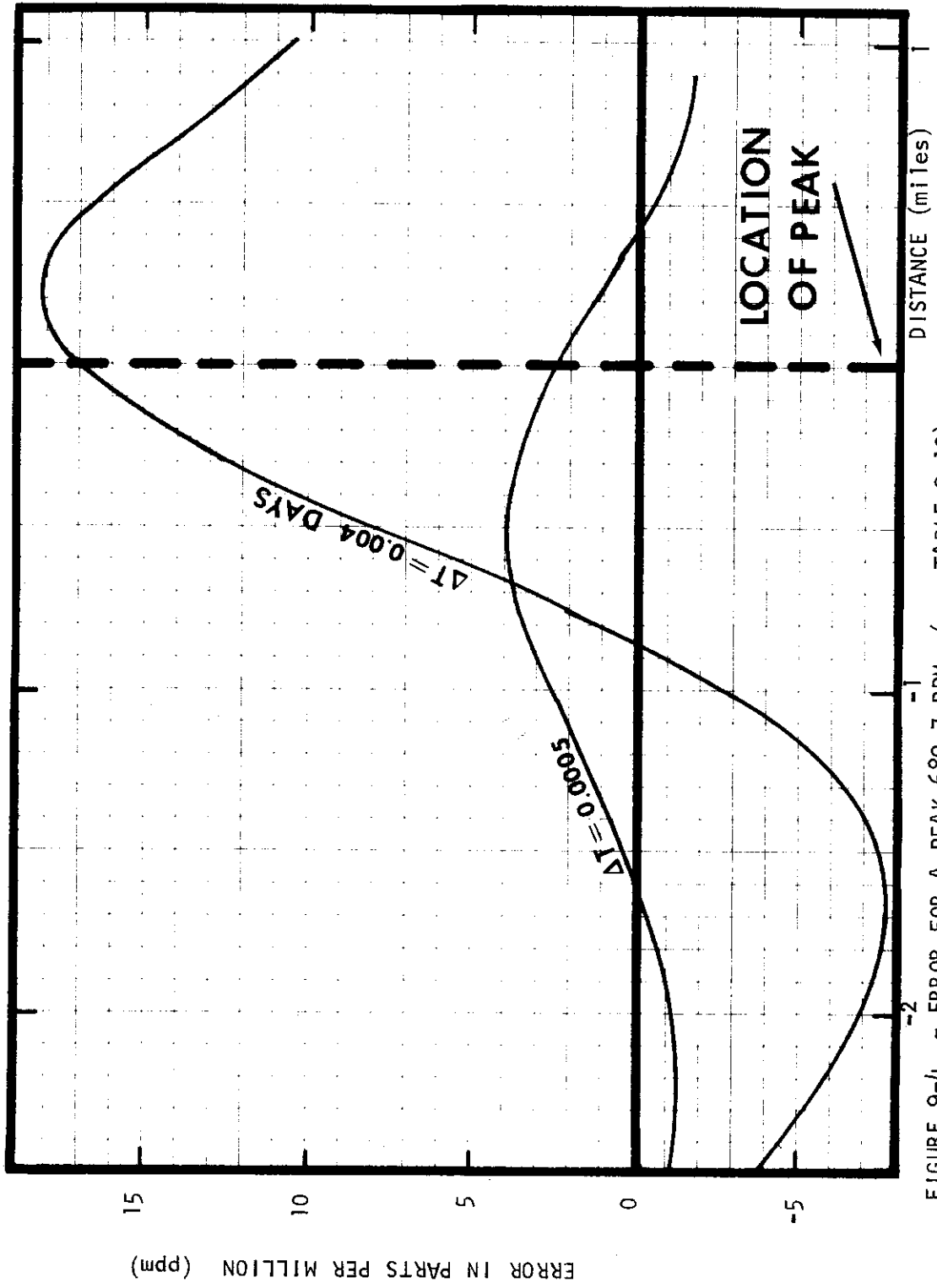


FIGURE 9-4. - ERROR FOR A PEAK 689.7 PPM (see TABLE 9-10)

the time increment becomes extremely small. Likewise, the distance increment limits the final accuracy which may be obtained, even when small time increments are used.

The implicit method has a more consistent accuracy throughout the useful range of time increments; this allows the use of larger time increments with the assurance that good accuracy can be obtained.

Starting the calculations with an initial distribution rather than a slug concentration at a single point has certain advantages. Although the accuracy near the peak is not significantly affected by the initial conditions, the accuracy of the leading and trailing edges is noticeably improved by using a distribution of values rather than a single concentration; this difference would become less significant as the dispersion coefficient is increased. Also, it should be noted that the leading and trailing edges of a profile for a slug load are generally not of much importance because they represent such a small percentage of the total mass of material.

Another advantage of using an initial distribution is that the implicit method remains accurate for much larger time increments. Table 9-12 shows that the implicit method is very accurate for a single starting concentration up to the time increment of 0.01 days, which is twice the value for stability for the explicit method. For an initial known distribution,

Table 9-13 shows that the implicit method is accurate up to a time increment of 0.05 days, which is ten times the stability criteria of the explicit method.

The distance increment of 0.1 miles provided good accuracy for any reasonable time increment applied in the error analysis. For example, with the explicit method a maximum error of 18.2 ppm is shown in Figure 9-4 near the peak concentration of 689.7 ppm and an error of about 4 ppm is shown for the trailing edge of the concentration. Thus, if a similar distribution were encountered from real data with a maximum BOD concentration of 6.89 mg/l, the maximum expected error from the explicit model would be 0.18 mg/l near the peak and about 0.04 mg/l near the edges of the profile; this is an acceptable error for real data. It should be noted that this magnitude of error resulted from a time increment of 0.004 days, which is larger than the time increment that would probably be used for these circumstances.

The following conclusions are clear from the calculations presented in this section: when a careful choice is made for the distance increment, the explicit method can provide good accuracy until the stability criteria is exceeded; the implicit method can provide good accuracy up to three times the stability criteria of the explicit method. When an initial distribution is known, the implicit method may remain accurate for up to ten times the stability criteria for time of the explicit method. Some authors recommend that the time increment for the explicit method be

restricted to one-half the stability criteria and that the Crank-Nicolson method be limited to twice this time increment in order to avoid inaccuracies caused by oscillations; these recommendations appear to be overly conservative for the examples presented in this section.

TWO-DIMENSIONAL ACCURACY ANALYSIS

The explicit and implicit finite difference models for two dimensions exhibited the same type of accuracy and stability as the one-dimensional models.

Steady-State Solutions. - Excellent accuracy can be obtained for steady-state solutions in two dimensions. However, iterating until the steady-state is reached can consume a large amount of computer time unless an initial distribution is used which is fairly close to the final values.

Instantaneous Releases of Slug Loads. - Excellent accuracy can be obtained in two dimensions when only dispersion is modeled. The introduction of velocity causes the same type of skewing and 'numerical dispersion' problems as in the one-dimensional cases; these inaccuracies can be minimized greatly by the proper choice of time and distance increments.

Table 9-14 shows the degree of accuracy which can be expected when both velocity and dispersion are included in the calculations. To obtain this distribution, an initial symmetrical

TABLE 9-14.--CONCENTRATIONS AT TIME = 0.11 DAYS

Miles	-.5	-.3	-.1	+ .1	+ .3	+ .5	+ .7
-.5	77	174	329	518	681	745	681
-.3	174	394	745	1174	1543	1689	1543
-.1	329	745	1408	2219	2915	3192	2915
+ .1	518	1174	2219	3496	4592	5029	4592
+ .3	681	1543	2915	4592	6032	6606	6032
+ .5	745	1689	3192	5029	6606	7234	6606
+ .7	681	1543	2915	4592	6032	6606	6032

(a) Exact Solution

Miles	-.5	-.3	-.1	+ .1	+ .3	+ .5	+ .7
-.5	75	171	327	519	680	739	669
-.3	171	392	750	1188	1558	1693	1532
-.1	327	750	1434	2273	2980	3238	2931
+ .1	519	1188	2273	3603	4723	5131	4645
+ .3	680	1558	2980	4723	6193	6727	6090
+ .5	739	1693	3238	5131	6727	7308	6616
+ .7	669	1532	2931	4645	6090	6616	5989

(b) Implicit Solution for $\Delta T = 0.0025$ Days

Miles	-.5	-.3	-.1	+ .1	+ .3	+ .5	+ .7
-.5	73	167	320	507	669	734	670
-.3	167	259	541	941	1529	1678	1532
-.1	320	730	1393	2212	2920	3203	2925
+ .1	507	1159	2212	3513	4635	5084	4641
+ .3	669	1529	2920	4635	6113	6700	6112
+ .5	734	1678	3203	5084	6700	7334	6684
+ .7	670	1532	2925	4641	6112	6684	6085

(c) Explicit Solution for $\Delta T = 0.0010$ Days

distribution was read into the models for a starting time of 0.01 days. The models then calculated the distribution until the elapsed time was 0.11 days. The explicit method used a time increment of 0.0025 days. The stability limits for the explicit method were 0.0025 days and 0.4 miles. The models used a distance increment of 0.1 miles; however, Table 9-14 presents the results for only every 0.2 miles. The exact solution was obtained by applying Equation 8-5 with $m = 10,000$; dispersion coefficient = $1 \text{ mi}^2/\text{day}$ ($322.7 \text{ ft}^2/\text{sec}$); and velocity = 5 miles/day ($.306 \text{ ft/sec}$). Almost equivalent accuracy was obtained by the models when a time increment of 0.002 days was used for the explicit method and a time increment of 0.005 days was used for the implicit method.

Good accuracy was also obtained when the dispersion coefficients and velocities varied in the two dimensions. The explicit model was applied to the following parameters: $E_X = 1 \text{ mi}^2/\text{day}$ ($322.7 \text{ ft}^2/\text{sec}$); $E_Z = 0.025 \text{ ft}^2/\text{sec}$ ($.775 \times 10^{-4} \text{ mi}^2/\text{day}$); $V_X = 5 \text{ miles/day}$ ($.306 \text{ ft/sec}$); $V_Z = 0.0 \text{ ft/sec}$. The initial profile at 0.01 days is shown in Table 9-15 for selected values, and the distribution calculated for 0.11 days is shown in Table 9-16. The vertical distance increment is 5 feet and the horizontal distance increment is 0.1 miles. The model used a time increment 0.002 days and the stability criteria for time was 0.0027 days. The exact solution is based on a value $m = 10$ in Equation 8-5.

TABLE 9-15.-INITIAL VALUES FOR NON-SYMMETRICAL DISTRIBUTION

Depth (Feet)	-0.5 Miles	-0.4 Miles	-0.3 Miles	-0.2 Miles	-0.1 Miles	L.P.*	0.1 Miles	0.2 Miles	0.3 Miles	0.4 Miles	0.5 Miles
0 ft.	17.	166.	953.	3325.	7040.	9039.	7040.	3325.	953.	166.	17.
5 ft.	13.					6769.					13.
10 ft.	5.					2842.					5.
15 ft.	1.					669.					1.
20 ft.	0.	2.	9.	32.	69.	88.	69.	32.	9.	2.	0.
25 ft.	0.					7.					0.
30 ft.	0.					0.					0.
35 ft.	0.	0.	0	0.	0.	0.	0.	0.	0.	0.	0.

*Loading Point

TABLE 9-16.--TWO DIMENSIONAL DISTRIBUTION AT TIME = 0.11 DAYS

Depth (Feet)	L.P.*	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
		Miles	Miles	Miles	Miles	Miles	Miles	Miles	Miles	Miles	Miles
0 ft.	$\frac{458.}{466.}$	$\frac{565.}{571.}$	$\frac{666.}{670.}$	$\frac{749.}{750.}$	$\frac{805.}{803.}$	$\frac{827.}{822.}$	$\frac{810.}{803.}$	$\frac{759.}{750.}$	$\frac{679.}{670.}$	$\frac{580.}{571.}$	$\frac{474.}{466.}$
5 ft.	$\frac{446.}{453.}$					$\frac{805.}{800.}$					$\frac{461.}{453.}$
10 ft.	$\frac{413.}{419.}$					$\frac{745.}{740.}$					$\frac{426.}{419.}$
15 ft.	$\frac{363.}{367.}$					$\frac{654.}{649.}$					$\frac{372.}{367.}$
20 ft.	$\frac{303.}{306.}$	$\frac{374.}{375.}$	$\frac{440.}{440.}$	$\frac{495.}{493.}$	$\frac{531.}{527.}$	$\frac{545.}{540.}$	$\frac{533.}{527.}$	$\frac{498.}{493.}$	$\frac{444.}{440.}$	$\frac{379.}{375.}$	$\frac{309.}{306.}$
25 ft.	$\frac{240.}{241.}$					$\frac{431.}{426.}$					$\frac{242.}{241.}$
30 ft.	$\frac{181.}{181.}$					$\frac{323.}{319.}$					$\frac{180.}{181.}$
35 ft.	$\frac{129.}{128}$	$\frac{159.}{157.}$	$\frac{187.}{185.}$	$\frac{210.}{207.}$	$\frac{225.}{221.}$	$\frac{230.}{227.}$	$\frac{225.}{221.}$	$\frac{209.}{207.}$	$\frac{186.}{185.}$	$\frac{157.}{157.}$	$\frac{127.}{128.}$

* Loading Point

Figure Key: $\frac{458.}{466.}$ = $\frac{\text{Exact}}{\text{Model}}$

For the examples shown in this chapter, the accuracy was poorest during early time steps and continued to improve as time progressed.

CHAPTER X

COMPUTATIONAL EXPERIENCE

Three aspects of the use of the computer models will be discussed briefly in this chapter: time requirements; use of boundary conditions; and choice of input parameters.

TIME REQUIREMENTS

The computer programs developed in this study are very economical in their use of computer time when compared to some other models for large estuaries (26). However, the programs are time-consuming with regard to the computer budget of a student; this is especially true for the two-dimensional programs when a large number of alternatives must be investigated to insure accuracy.

Table 10-1 presents data on the time requirements for some typical runs on an IBM 360/65 of the most recent versions of the models. The information includes the size of the grid, the number of iterations, the time to compile and initialize the data, the execution time for the computation phase, the number of seconds of execution time per iteration, and the number of seconds of execution time per iteration per grid point. This final parameter allows for the computation of an approximate run time of the models on an IBM 360/65. For instance, the approximate time requirement for MASSTRANS-I would be:

TABLE 10-1.-COMPUTATION TIMES

Grid	Iterations	Initial-ization (sec)	Execution Time (Seconds)	<u>Seconds</u> Iteration	<u>Seconds</u> Inter.-Pt.
(a) MASSTRANS-I (Explicit)					
15 x 15	102	22.	111.5	1.09	484×10^{-5}
15 x 15	52	22.	55.2	1.06	472×10^{-5}
36 x 11	42	22.	81.6	1.94	490×10^{-5}
42 x 1	122	20.	21.0	0.172	409×10^{-5}
36 x 1	182	20.	21.6	0.119	331×10^{-5}
(b) MASSTRANS-II (Implicit)					
15 x 15	42	25.	115.2	2.75	1225×10^{-5}
15 x 15	22	25.	58.2	2.65	1180×10^{-5}
(c) IDEAL-I (Explicit)					
51 x 1	1200	1.	33.72	.0281	55×10^{-5}
51 x 1	600	1.	17.90	.0298	58×10^{-5}
51 x 1	300	1.	9.69	.0323	63×10^{-5}
(d) IDEAL-II (Implicit)					
51 x 1	600	1.	30.93	.0516	101×10^{-5}
51 x 1	300	1.	16.83	.0562	110×10^{-5}
51 x 1	120	1.	6.81	.0567	111×10^{-5}
(e) OXTRANS-I (Explicit)					
42 x 1	204	21.	82.80	.406	965×10^{-5}
42 x 1	102	21.	34.20	.335	798×10^{-5}
31 x 1	402	21.	106.00	.265	853×10^{-5}
33 x 7	68	23.	147.00	2.160	935×10^{-5}

Total Computer Time in Seconds

$$= 25 + (500 \times 10^{-5} \times \text{iterations} \times \text{total number of grid points})$$

Since different options and boundary conditions were used for the various programs, the time requirements are not completely comparable. However, it is clear that the one-dimensional version of the Crank-Nicolson implicit method tends to take 1.75 times as long as the explicit method for the same time and distance increments. Likewise, the two-dimensional version of the implicit method takes 2.5 times as long as the explicit method. However, since the implicit method remains accurate for much larger time increments than the explicit method, the implicit method often proves to be the more economical choice.

OXTRANS-I, the largest program, has the following storage requirements:

Object Code = 66,000 bytes; Array Area = 59,000 bytes.

BOUNDARY CONDITIONS

For steady-state profiles and for leading and trailing edges of a concentration profile, inaccurate boundary conditions significantly affect only the closest three internal concentrations. However, as the concentrations near the boundary become a high percentage of the peak value, the entire profile can be distorted by inaccurate boundary extrapolations. The most straightforward

way of avoiding this "feedback" is to keep the peak far away from the boundaries where values must be extrapolated; this can be accomplished by using a large grid or by moving the grid when the peak gets too close to the boundaries.

CHOICE OF INPUT PARAMETERS

Two-dimensional, time-varying data is difficult and expensive to obtain. However, this type of data is required to completely calibrate and verify two-dimensional models.

In spite of the logistical problems involved, velocity is one of the more easily involved parameters because it can be measured directly with sensitive current meters. If horizontal velocities are carefully measured, vertical velocities can be calculated through the two-dimensional continuity equation. For estuaries whose width and velocities vary throughout the estuary, care must be taken to insure that input values for velocity satisfy continuity. If measured data is not available, approximate intra-tidal velocity values also can be obtained by coupling the finite difference models to computer models for tidal velocities; a one-dimensional tidal model is available for the Houston Ship Channel (110).

Dispersion coefficients are particularly troublesome to obtain in two dimensions. As shown by Ippen (51), the value of the horizontal dispersion coefficient for one-dimensional models is

much larger than the horizontal dispersion coefficients appropriate for two-dimensional models. Horizontal dispersion coefficients for one-dimensional models must represent the effects of the more localized eddy diffusion; in two-dimensional analysis, the two extra variables of vertical velocity and vertical dispersion are available to aid in representing these effects. More research needs to be done to determine representative values for vertical dispersion for individual estuaries. Work by Prichard (80) and Bowden (5) suggests that vertical dispersion coefficients can be expected to be in the range of 0.001 to 0.050 ft^2/sec , where the higher values indicate a higher degree of vertical mixing.

Adequate source and sink terms are sometimes difficult to obtain. Decay rates in highly polluted estuaries must be chosen with care because of possible inhibition effects (87, 85). Re-aeration rates are particularly elusive for slowly moving, highly contaminated estuaries.

As Callaway has said (20), this is truly "the age of the great coefficient hunt."

CHAPTER XI

APPLICATION TO THE HOUSTON SHIP CHANNEL

An important aspect in proving the accuracy and usefulness of a model comes through testing the ability of the model to reproduce actual data. On May 24-26, 1971, a dye study was carried out on the Houston Ship Channel by the Estuarine Systems Projects of Texas A&M University during a moderate rainfall on the watershed surrounding the channel. This study provided sufficient data (31) to determine the ability of the model to reproduce the two-dimensional behavior of the channel during dynamic conditions.

Two hundred pounds of Rhodamine WT dye were released at Mile 16.5 on May 24, 1971, at 12 noon (hereafter called H-hour). Two-dimensional measurements for dye were taken periodically on the channel during the 44 hours following the release. The gauged runoff into the channel peaked at H + 7 hours (7:00 p.m.) on May 24 at a discharge of approximately 4750 cfs (31). The channel was partially stratified during the study period; for example, on the morning of May 25, the salinity at Mile 12 ranged from 12.6 ppt at the surface to 15.6 ppt at the bottom. The tide readings for the study period are shown in Figure 11-1.

The first two-dimensional data collection for dye fluorescence was carried out at H + 4 hours. The fluorescence contours for that sampling run are shown in Figure 11-2; the channel

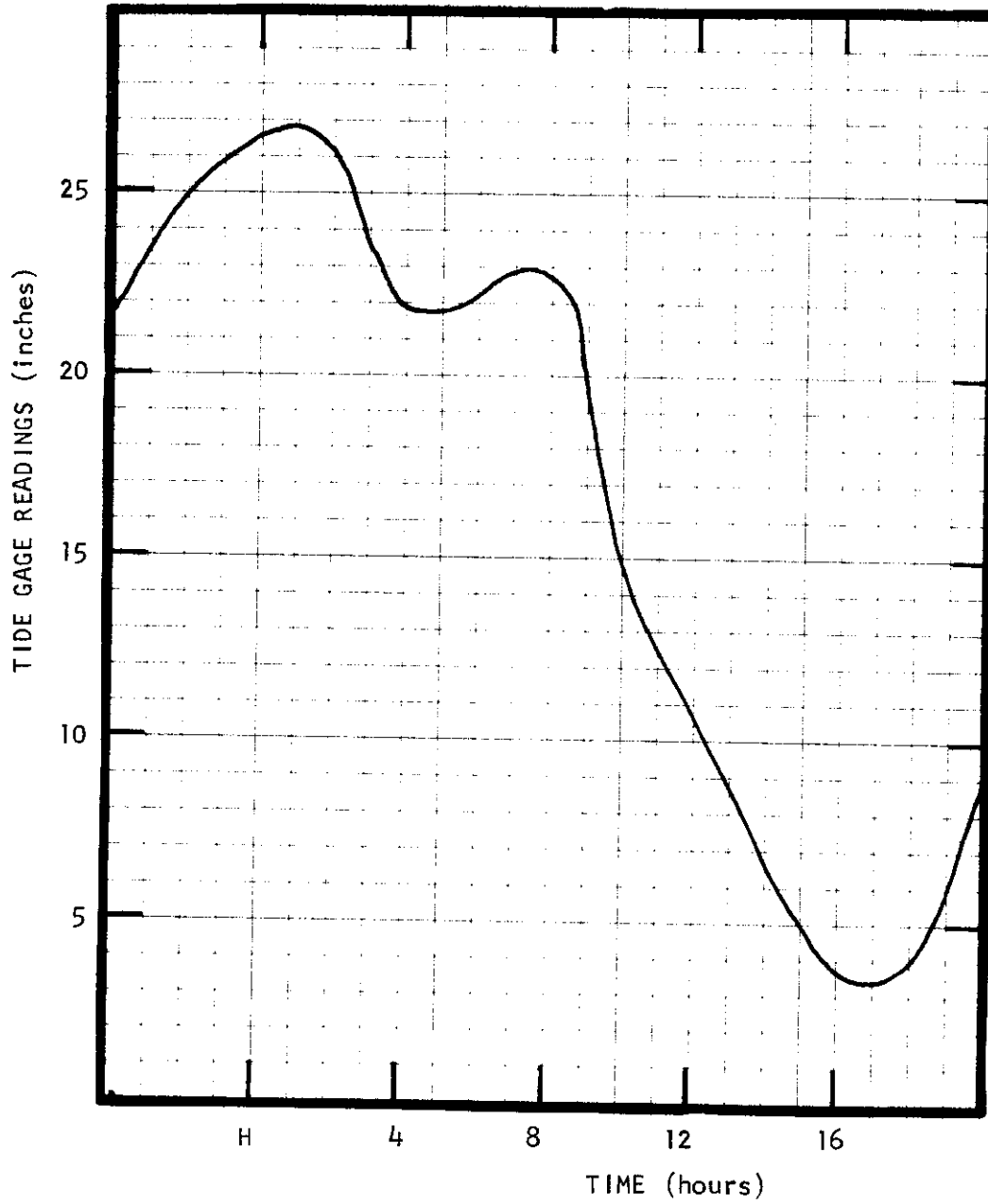


FIGURE 11-1. - TIDE READINGS

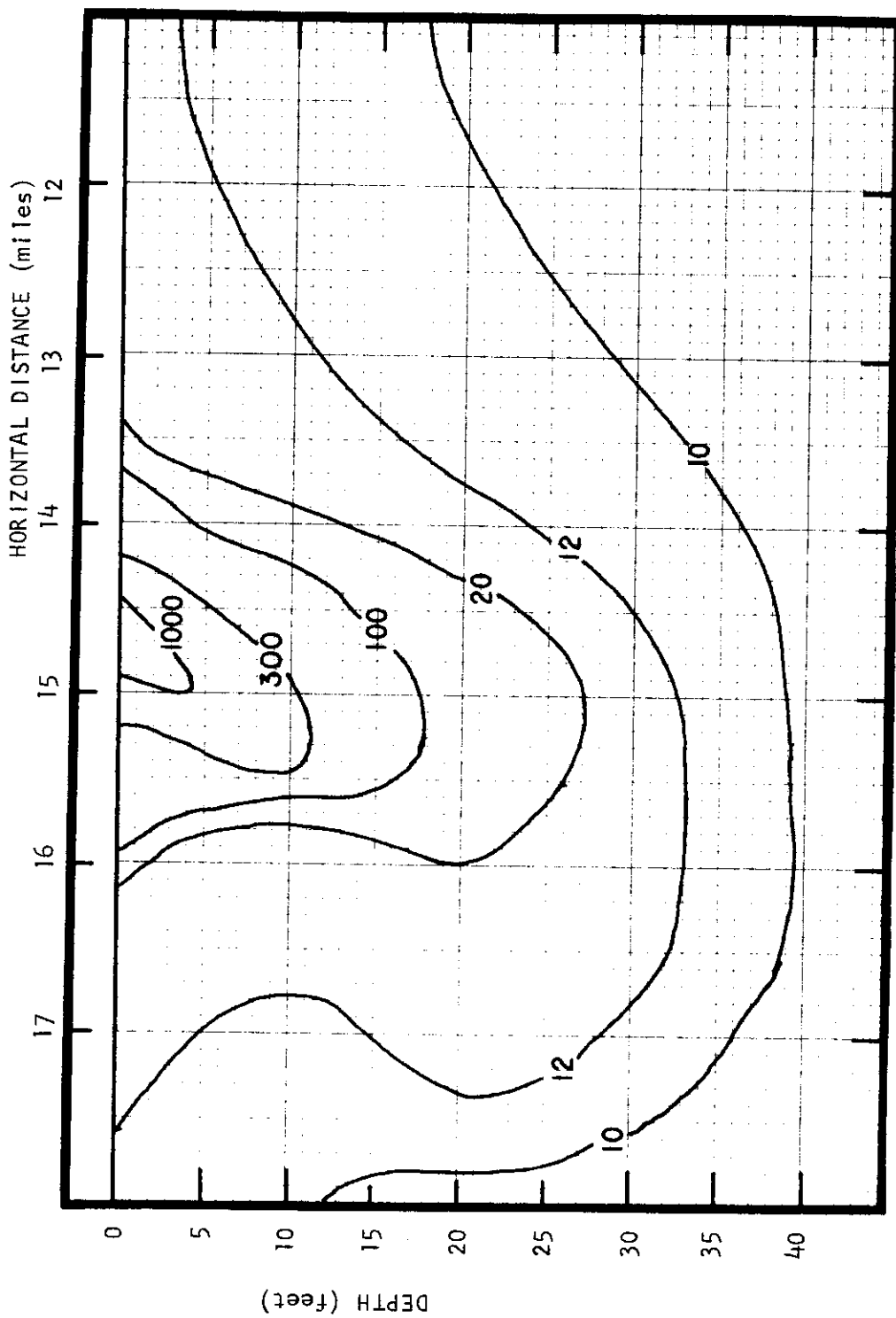


FIGURE 11-2. - MEASURED CONCENTRATIONS AT H + 4

had a background fluorescence of about 10 units.

The concentrations from this first sampling run were used as the input concentrations for the model. Concentrations were chosen at intervals of 0.25 miles in the longitudinal direction and 5.0 feet in the vertical. These initial concentration values are shown in Table 11-1.

Data was available also from a two-dimensional sampling run at $H + 15$ hours. The aim of the simulation was to compare the concentrations predicted by the model with the concentrations measured at $H + 15$ hours.

Average values were used in the model for velocity, dispersion, and decay during the 11-hour simulation period. Horizontal velocities were obtained by channel measurements and observation of fluorescence contours. Vertical velocity was considered to be insignificant and was set equal to zero. Horizontal dispersion was set at $450 \text{ ft}^2/\text{sec}$ ($1.39 \text{ miles}^2/\text{day}$) at all points. Vertical dispersion was considered to be proportional to horizontal velocity and had a maximum value of $0.004 \text{ ft}^2/\text{sec}$ ($1.24 \times 10^{-5} \text{ miles}^2/\text{day}$). Values for horizontal velocity and vertical dispersion coefficients are shown in Table 11-2.

Widths were approximated from Corps of Engineer cross-section data and from work by Hutton (45, 46). These values are shown in Table 11-3 and represent widths at a 20-foot depth. The sampling data indicated that a considerable

TABLE 11-1.--INITIAL CONCENTRATIONS

Mile	0'	5'	10'	15'	20'	25'	30'	35'	40'
17.00	13.2	12.0	11.7	12.0	13.0	13.0	11.8	10.2	10.0
16.75	14.0	13.5	12.0	13.0	14.0	14.0	12.1	10.3	10.0
16.50	16.2	15.0	14.0	14.0	15.0	15.0	12.3	10.4	10.0
16.25	19.5	17.0	16.0	16.0	17.0	16.0	12.5	10.5	10.0
16.00	100.0	19.0	19.0	19.0	20.0	17.0	12.7	10.6	10.0
15.75	125.0	80.0	50.0	40.0	35.0	19.0	13.0	10.8	10.0
15.50	175.0	220.0	300.0	130.0	65.0	22.0	13.5	11.0	10.0
15.25	300.0	500.0	450.0	175.0	80.0	30.0	14.0	10.8	10.0
15.00	750.0	900.0	330.0	150.0	65.0	24.0	13.3	10.6	10.0
14.75	2000.0	550.0	225.0	100.0	40.0	20.0	12.6	10.4	10.0
14.50	1000.0	300.0	130.0	60.0	25.0	17.0	12.0	10.3	10.0
14.25	300.0	200.0	85.0	30.0	18.0	14.0	11.6	10.2	10.0
14.00	175.0	85.0	35.0	19.0	15.0	12.0	11.2	10.1	10.0
13.75	100.0	20.0	19.0	15.0	12.0	11.5	10.8	10.0	10.0
13.50	30.0	18.0	17.0	12.0	11.7	11.0	10.4	10.0	10.0
13.25	19.0	17.0	15.0	11.8	11.5	10.7	10.0	10.0	10.0
13.00	17.0	16.0	14.0	11.6	11.2	10.4	10.0	10.0	10.0
12.75	15.0	15.0	13.0	11.4	11.0	10.2	10.0	10.0	10.0
12.50	14.0	14.0	12.0	11.3	10.7	10.0	10.0	10.0	10.0
12.25	13.0	13.0	11.8	11.2	10.5	10.0	10.0	10.0	10.0
*12.00	12.5	12.0	11.6	11.1	10.2	10.0	10.0	10.0	10.0

*All concentrations not shown were equal to 10.0. The background fluorescence of 10.0 was subtracted from the concentrations shown in the table and the resulting values were used as the input data.

TABLE 11-2.-HORIZONTAL VELOCITIES AND VERTICAL DISPERSION COEFFICIENTS

Depth (ft.)	Horizontal Velocity (ft/sec)	Vertical Dispersion (ft ² /sec)
0	0.3667	.3333
5'	0.4200	.3820
10'	0.4400	.4000
15'	0.4400	.4000
20'	0.4333	.3940
25'	0.3893	.3540
30'	0.2733	.2485
35'	0.2333	.2120
40'	0.2333	.2120

TABLE 11-3.-WIDTHS AT 20-FOOT DEPTH

Mile	Width (feet)	Mile	Width (feet)
17.00	497		
16.75	510	12.75	872
16.50	524	12.50	931
16.25	537	12.25	863
16.00	550	12.00	796
15.75	562	11.75	728
15.50	575	11.50	661
15.25	562	11.25	657
15.00	550	11.00	654
14.75	537	10.75	650
14.50	525	10.50	647
14.25	568	10.25	633
14.00	611	10.00	619
13.75	654	9.75	605
13.50	697	9.50	591
13.25	756	9.25	586
13.00	814	9.00	582

amount of dye was lost by sediment adsorption and other influences. An exponential decay rate of 2.5 / day was found to adequately represent this loss of dye for the period being simulated.

The simulation run used a time increment of 600 seconds; the maximum allowable time increment for stability would have been 1155 seconds. The simulation required 66 iterations; the compile time plus the execution time was 2.5 minutes on an IBM 360/65 for MASSTRANS-I.

The measured dye concentration profiles for hour H + 15 are shown in Figure 11-3. The concentration profiles predicted by MASSTRANS-I are shown in Figure 11-4. Agreement between the profiles is very good considering that only average values were used as input data in the model.

Comparison between the dye concentrations and model predictions are shown at 5-foot increments in Figure 11-5. Peak values are accurately matched and the mass of dye appears to be conserved by the model. The skewing of the measured profiles was caused by local velocity fluctuations which were not included in the model input data.

Figure 11-6 shows a comparison between peak concentrations in the vertical direction for the model and the sampled data. This comparison demonstrates that the model can be especially accurate in representing the peak concentrations at the appropriate depths.

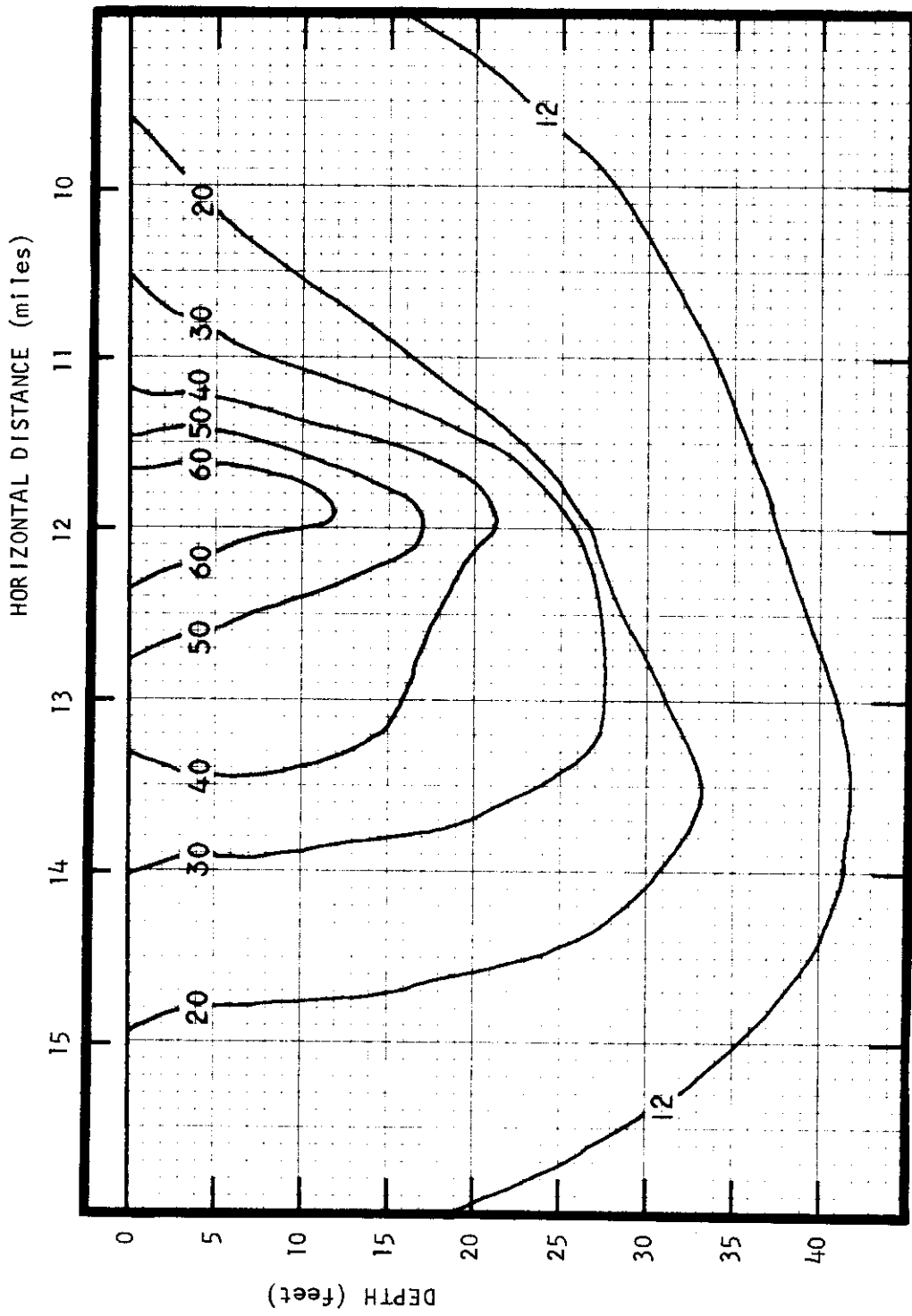


FIGURE 11-3. - MEASURED CONCENTRATIONS AT HOUR H + 15

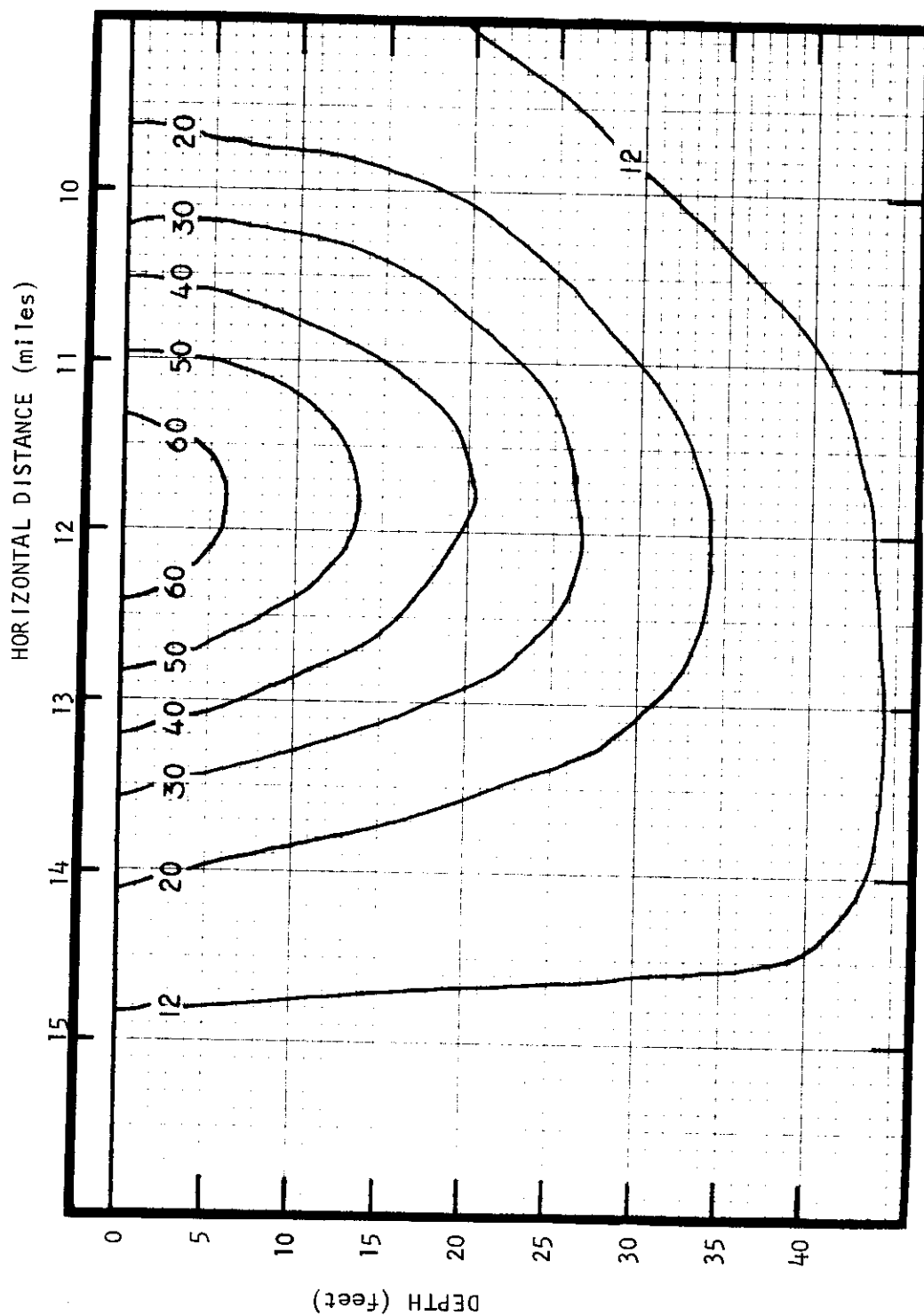


FIGURE 11-4. - CONCENTRATIONS PREDICTED BY MASSTRANS-1 FOR HOUR H + 15

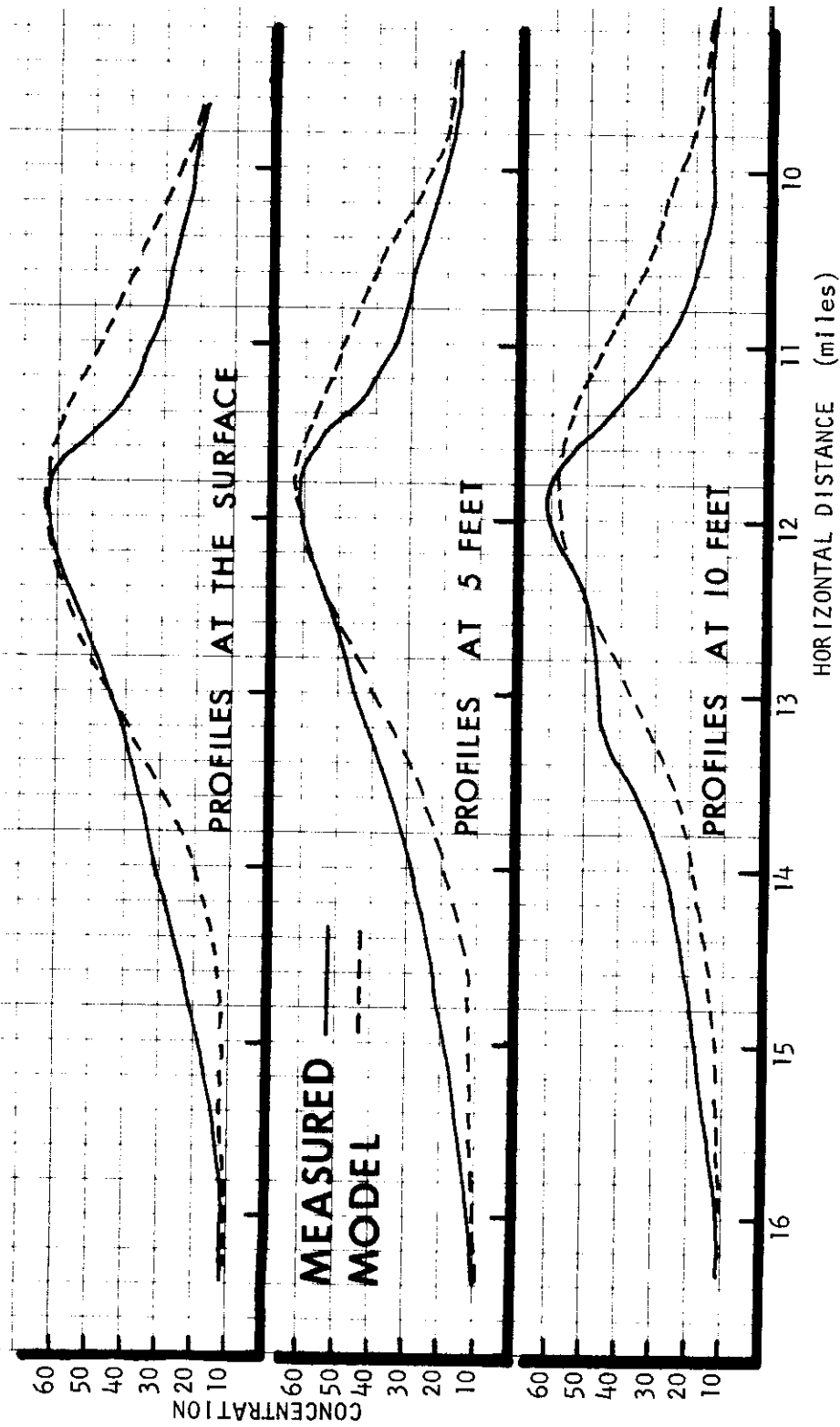
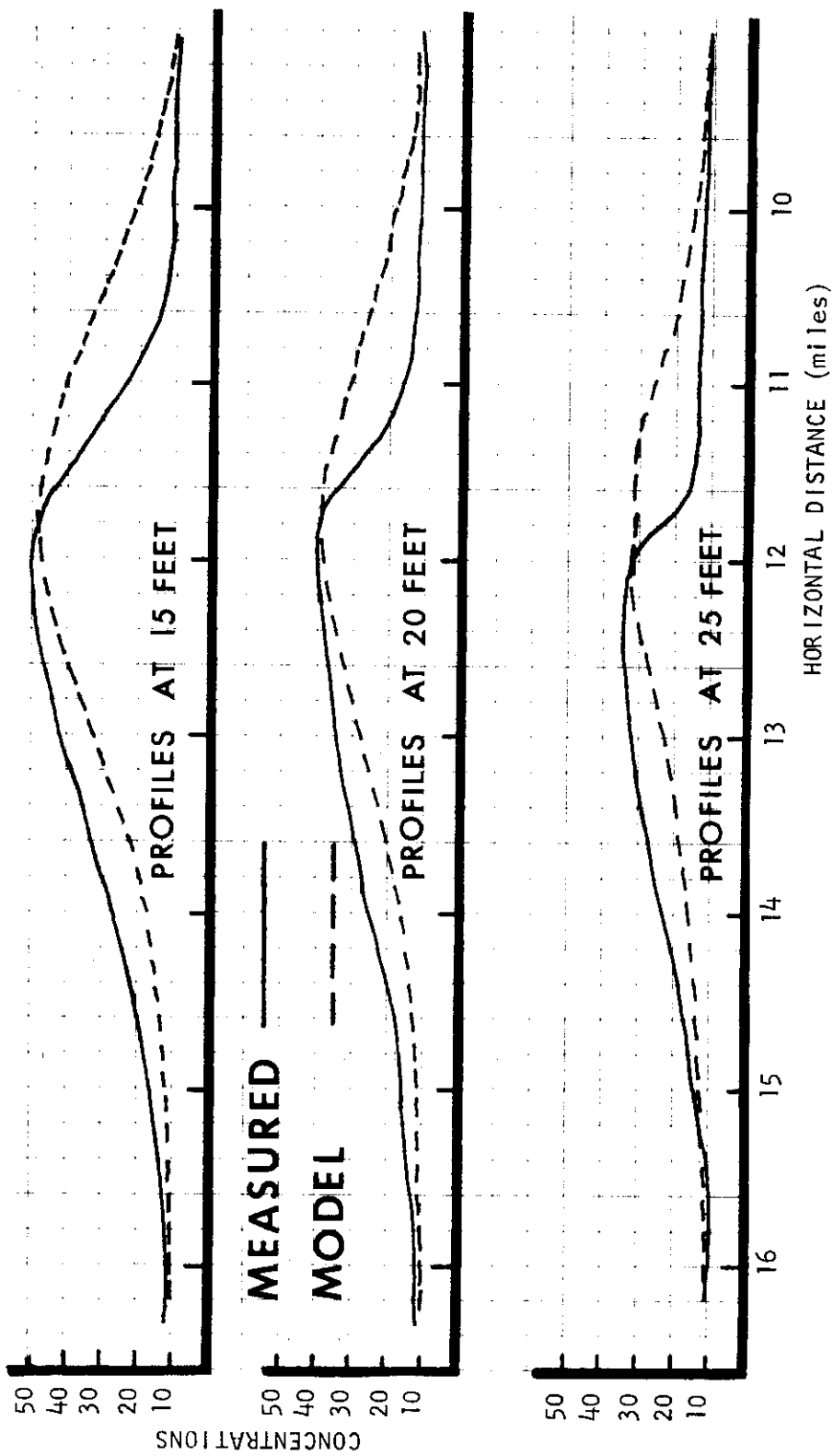


FIGURE 11-5. - COMPARISON OF MEASURED AND SIMULATED PROFILES

(Continued on next page)



(End of Figure 11-5)

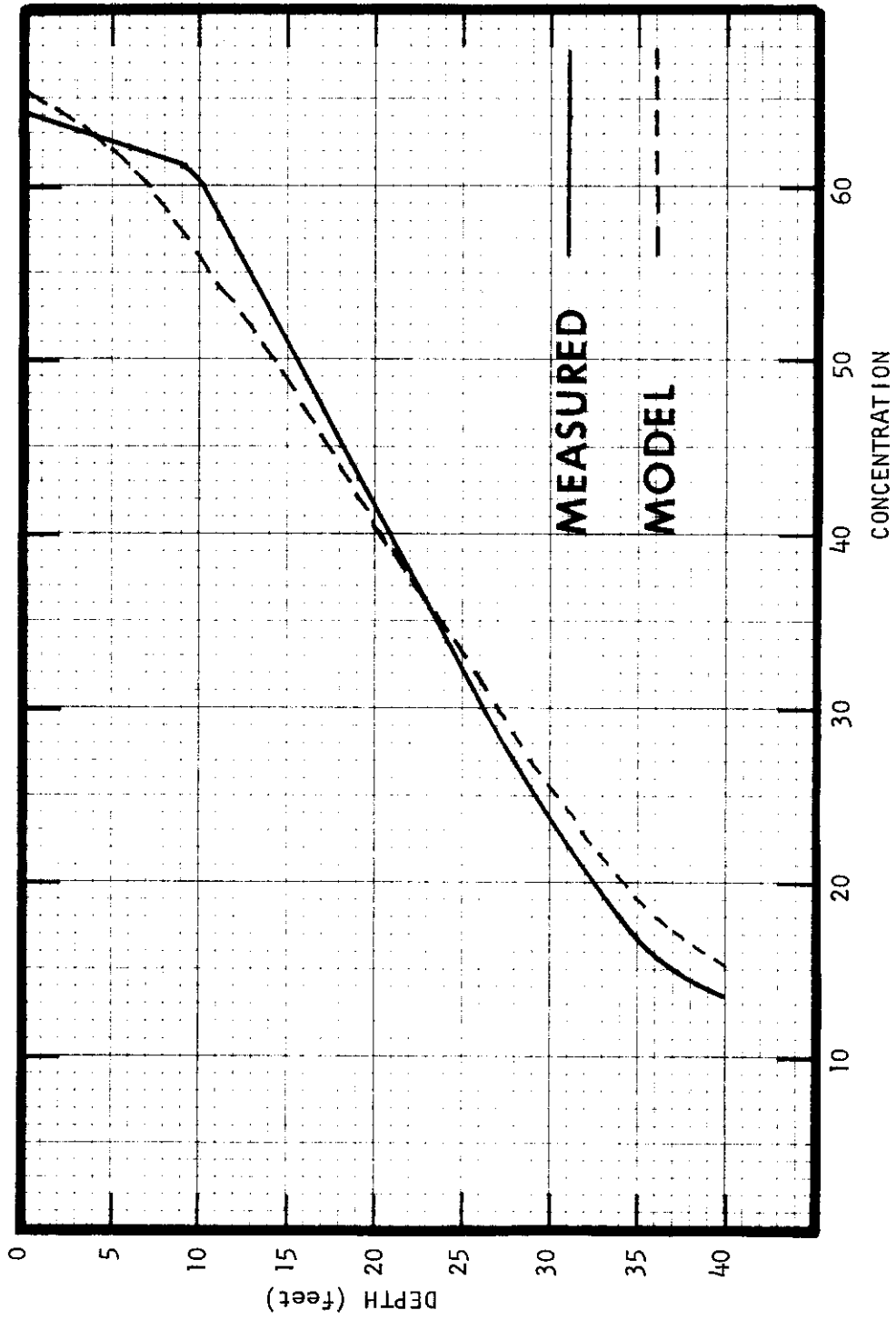


FIGURE 11-6. - COMPARISON OF PEAK CONCENTRATIONS

These comparisons demonstrate that the two-dimensional models can accurately represent dynamic conditions on the Houston Ship Channel, even when averaged input data is used. When more precise data for velocity, dispersion, and other parameters are fed into the model, more accurate simulations can be expected.

MODELING BOD AND DISSOLVED OXYGEN IN THE HOUSTON SHIP CHANNEL

A major use of OXTRANS-I will be to model the BOD and dissolved oxygen distributions in the Channel for rapidly changing conditions. The ability of the model to accomplish this task was demonstrated by applying the velocity and dispersion values which existed during the dye study to a hypothetical dissolved oxygen distribution. A simple initial distribution of dissolved oxygen and BOD was chosen in order that changes in the distribution would be more apparent. The hypothetical initial values for BOD_{20} and dissolved oxygen are shown in Figure 11-7; this figure shows a saturated value of 8 ppm of oxygen and no BOD below Mile 15. Above Mile 15, a BOD_{20} concentration of 13 ppm is shown along with a zero value for dissolved oxygen.

The velocity and dispersion values from the dye study simulation were applied to the initial DO-BOD values, and the simulation was run again until hour H + 15. The following parameters were used: aerobic decay rate (base e) = 0.25/day; anaerobic decay rate (base e) = 0.08/day; and reaeration rate (base e) =

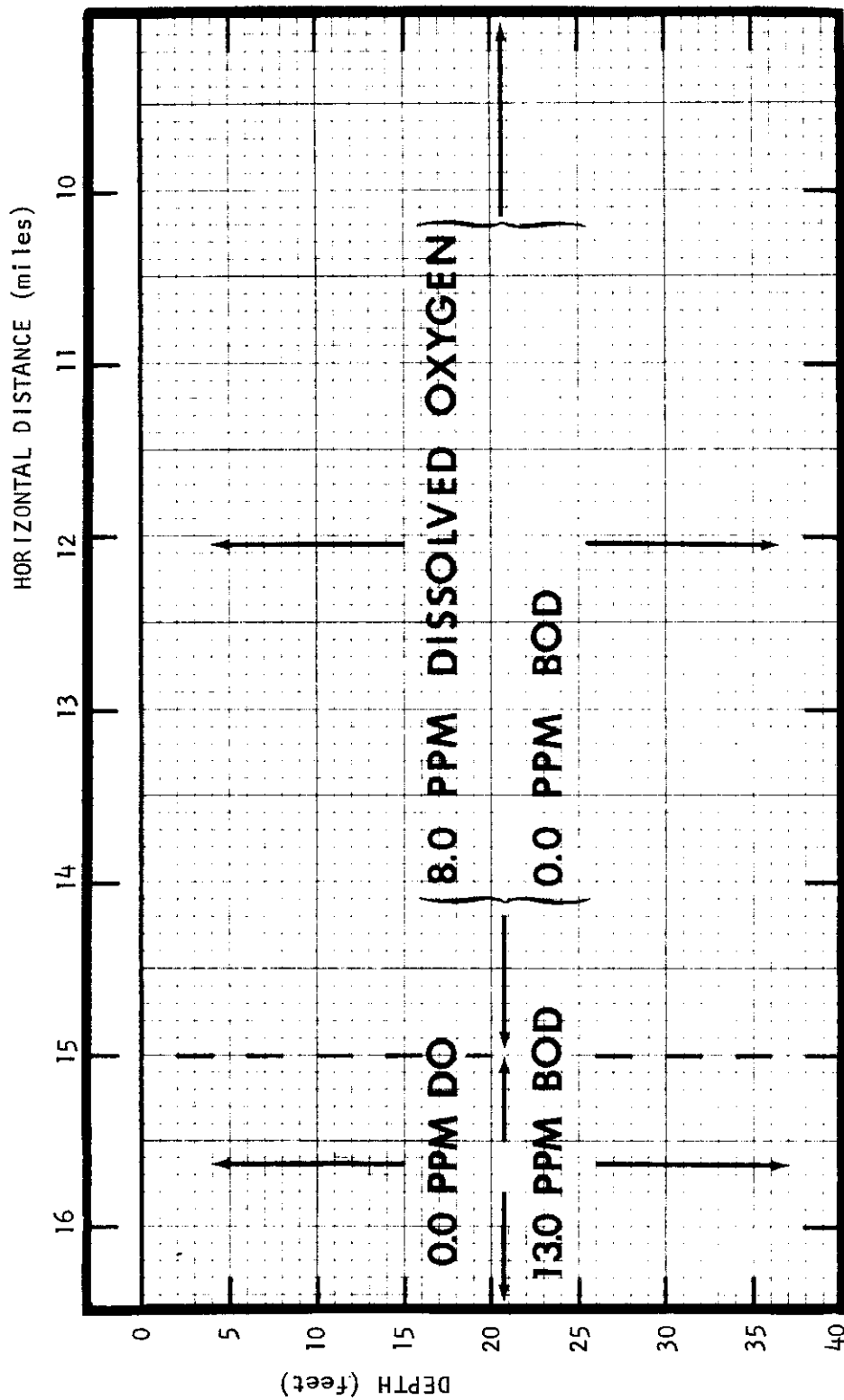


FIGURE 11-7. - INITIAL HYPOTHETICAL VALUES FOR BOD₂₀ AND DO AT H + 4 HOURS

0.10/day. The results of this simulation are shown in Figure 11-8, for BOD_{20} and Figure 11-9, for dissolved oxygen. During the 11 hours of simulation, the saturated value of 8 ppm is pushed from Mile 15 to Mile 9.

This example demonstrates that the most pronounced change during the early stages of increased flow are due mainly to the downstream movement of the low oxygen conditions by velocity and dispersion. Several different initial BOD concentrations were tried in the model and the dissolved oxygen pattern at the end of the simulation was essentially the same as the values already shown in Figure 11-9. However, over a longer period of time, the persistence of the low oxygen conditions would obviously depend upon the amount of organic material that remained in the part of the channel being studied.

The conditions modeled in this simulation represent only moderate runoff and moderate pollution conditions. Extensive studies by Reynolds and Eckenfelder (85) during 1968 and 1969 show that the BOD_{20} in the vicinity of Mile 16 can rise to at least 38 ppm and the decay rate (base e) can range between .07/day and .44/day. Likewise, much higher runoff conditions are common to the channel.

APPLICABILITY OF THE MODELS

The examples of this chapter and the previous chapter demonstrate the reliability of the models when applied to the Houston

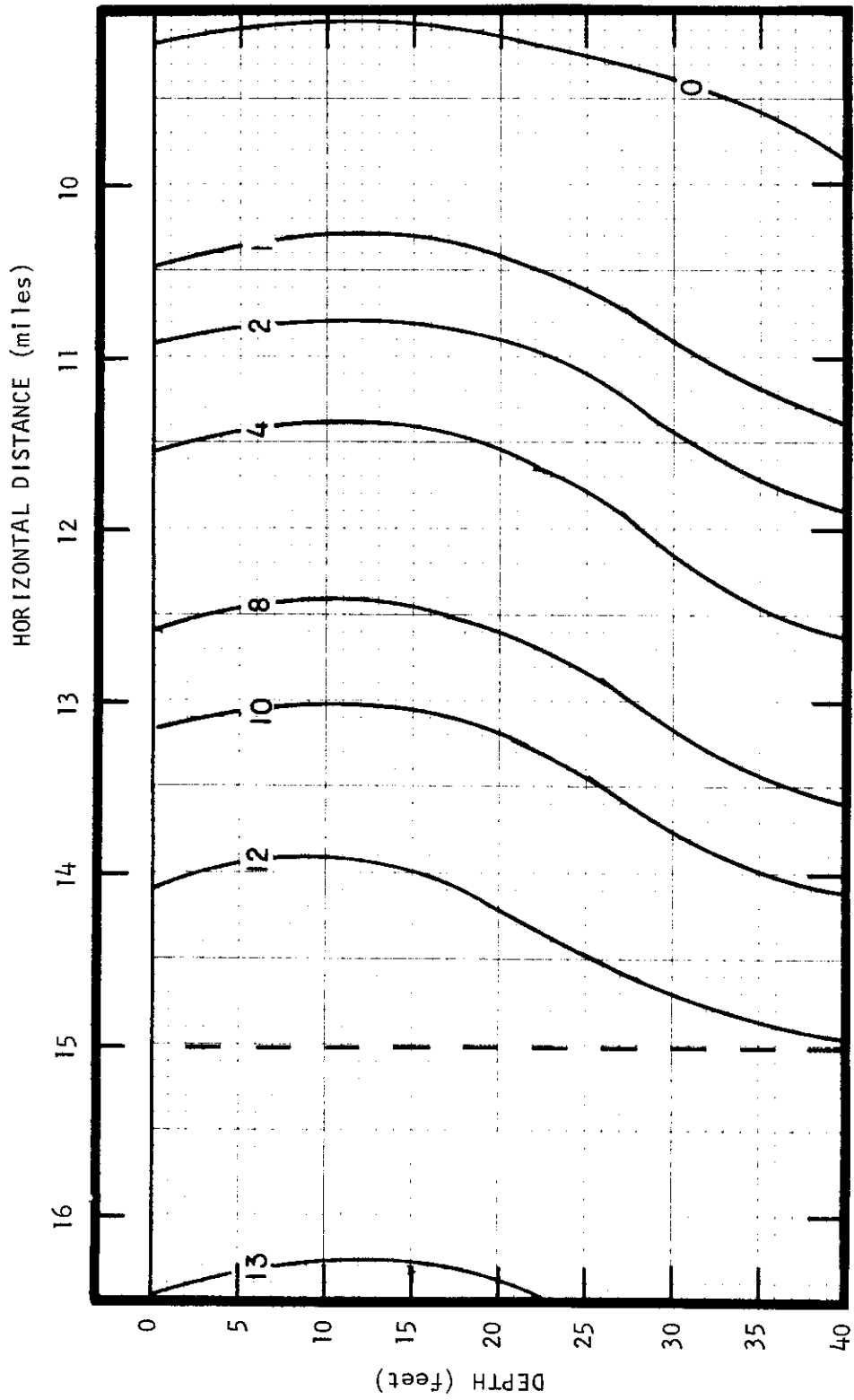


FIGURE 11-8. - BOD₂₀ (ppm) AT H + 15 HOURS

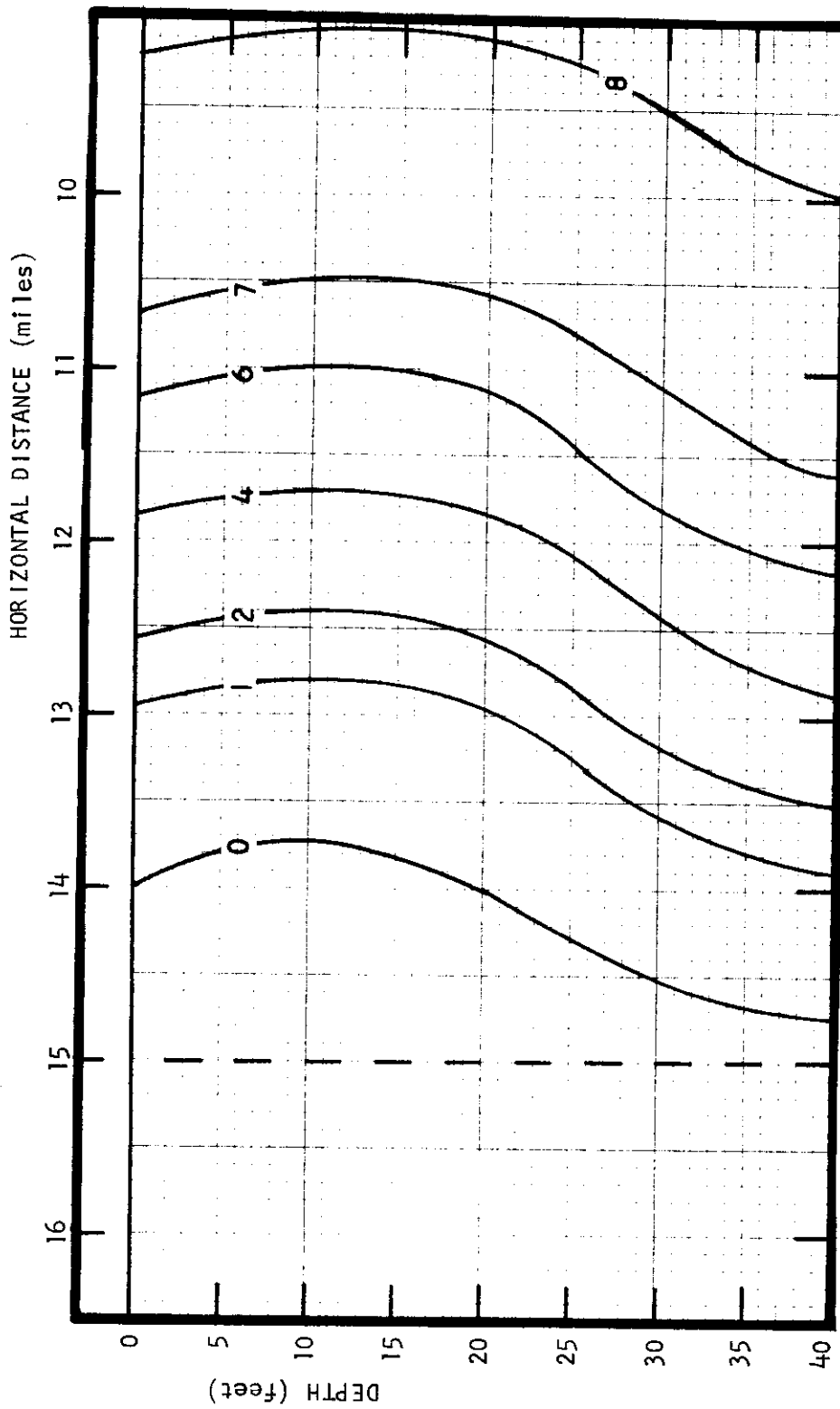


FIGURE 11-9. - DISSOLVED OXYGEN (ppm) AT H + 15 HOURS

Ship Channel and similar estuaries. These models can use approximate input parameters to provide estimates of concentration distributions in one or two dimensions; these approximations can be used for rough engineering estimates of conditions in the estuary for various hydrodynamic and loading conditions. The models can also accept input parameters which are much more exact; this type of approach is useful for simulation or predictive purposes.

CHAPTER XII

DISCUSSION

When modeling a partially stratified estuary, the following steps should be taken in utilizing the computer programs that were developed in this study. Representative parameter values for the estuary being studied should be used in the one-dimensional explicit and implicit models and the computed concentrations should be checked against exact solutions. Several time and distance steps should be tried until good accuracy is obtained for the range of conditions being studied. These initial computations should indicate the approximate increments to use in the two-dimensional models. In turn, the two-dimensional models should be run several times with different increments. The best approach is to use both the explicit and implicit models to cross-check each other. Special care should be taken when studying instantaneous releases; inaccurate initial conditions can invalidate the entire computation.

One of the real dangers in mathematical modeling is that almost any type of estuary model can generate profiles that appear to be reasonable. Some estuary models have gained acceptance through repeated use and promotion, not through repeated proof of their accuracy. For this reason, this study has taken great care to establish the accuracy of its finite difference models and has

provided means of checking their accuracy when applied to parameters other than those used in this report. The models developed in this study include no weighting factors and depend solely on the estuary data provided as input; all of this data can be obtained independently of the models.

In this study, good accuracy was obtained for horizontal distance increments up to 0.25 miles. For finite difference methods, increasing the distance increment generally decreases the accuracy of the model and increases interference by numerical dispersion; this is especially true when studying time-changing behavior. Thus, a model whose grid dimensions can be changed easily has a great advantage: the model can be run several times with different distance increments until results of sufficient accuracy are obtained.

The computer programs that were developed in this study have numerous applications. The two-dimensional models can be applied directly to partially stratified estuaries of constant center-line depth; dredged channels in the Gulf Coast region are the most obvious examples of this type of estuary. The one-dimensional models can be applied directly to well-mixed estuaries.

The accuracy requirements of the input data for the models is determined by the needs of the user. Rough estimates of concentration profiles can be obtained by inputting a representative range of values for the estuary being studied. If more accurate

results are required, data for the model should be obtained from dye releases, velocity measurements, reaeration rate measurements, decay rate studies, and benthic deposit studies. Examples of these types of investigations are available in several sources (2, 8, 40, 44, 46, 56, 71, 85, 103).

The usefulness of the present form of the computer programs is limited in several respects. The two-dimensional models require computers with large core storage; in addition, the computer programs require a compiler which accepts FORTRAN IV and unformatted READ and PRINT statements. These shortcomings can be overcome by straightforward programming techniques: discs and tapes can be used as alternate storage locations and format statement can be rewritten according to requirements of the computer being used.

The finite difference methods discussed in this study can be extended to estuaries with varying depths. Likewise, the finite difference equations can be expanded to three dimensions. However, in both these cases, the computational grids must be matched to the geometry of the particular estuary; this would require extensive additional programming and would result in models which were estuary-dependent. Additional work is needed on models of this type.

Additional verification of the present models is also needed. The ultimate usefulness of these methods can be determined only

after extensive computations are made for a wide variety of inter-tidal and intra-tidal conditions.

CHAPTER XIII

SUMMARY AND CONCLUSIONS

The following goals were accomplished in this study: (a) computer models were developed which can calculate time-varying vertical and horizontal mass transport in partially stratified estuaries; (b) the accuracy and usefulness of explicit finite difference models were compared with that of Crank-Nicolson implicit finite difference models; (c) the applicability of finite difference models to the mass transport characteristics of the Houston Ship Channel was demonstrated; and (d) a summary was made of existing information on one- and two-dimensional mathematical models that have been applied to significant estuary problems.

Explicit and Crank-Nicolson finite difference models were developed for the one- and two-dimensional estuary equations with varying coefficients. The models were constructed to allow for the varying of parameters at any time at any grid point and were programmed in FORTRAN-IV computer language. Good accuracy was obtained by both types of models when proper time and distance increments were used. The Crank-Nicolson approach was found to be more accurate for a wider range of these increments. The concentration profiles for instantaneous releases and for steady-state conditions were analyzed. Accuracy was determined by comparison with analytical closed-form solutions.

Models also were developed to analyze the profiles for biochemical oxygen demand and dissolved oxygen under time-changing conditions. These models can analyze both aerobic and anaerobic conditions and included provisions for analyzing the effects of mechanical reaeration.

Applicability of these models to partially stratified estuaries was established by comparisons with dye study data from the Houston Ship Channel.

The Crank-Nicolson implicit method programmed in this study tends to take 1.75 times as much computer time as the explicit method in one-dimensional applications and 2.5 times as much time for two-dimensional applications when equal time and distance increments are used; however, the implicit method is sometimes the more economical choice since it remains stable and accurate for larger time increments than the explicit method.

The accuracy of the finite difference models is particularly sensitive to the distance increments. Horizontal grid increments of 0.25 miles or less can provide excellent accuracy when finite difference models are applied to instantaneous releases. Grid increments of 2.0 miles or less can provide excellent accuracy when finite difference models are applied to the downstream portions of steady-state profiles. Finite difference models should have grid sizes which can be varied easily to enable the choice of a distance increment which provides good accuracy for

the conditions being studied.

The finite difference methods outlined in this study can be extended to partially stratified estuaries with varying depth and to three-dimensional estuary calculations; these applications would require extensive modifications to the existing programs. Further verification of the present two-dimensional models also is needed to determine their general applicability.

This study and similar studies demonstrate that techniques are presently available to model a wide range of hydrodynamic and mass transport conditions in estuaries. A major future task is to determine for two and three dimensions the appropriate values for parameters such as velocity, dispersion, decay, re-aeration, photosynthesis, and benthic demands.

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 APPENDIX II. - NOTATION

Dimensions are shown in brackets and are represented as follows: M = mass; L = length; and T = time.

- a = abbreviated notation for $a(x,z)$
- $a(x,z)$ = left-hand boundary of estuary; lower limit of integration across the width, [L]
- A = average top salinity (equation 4-8 only)
- A_i = constant coefficient for Crank-Nicolson method
- A_x = cross-sectional area at location x, [L²]
- A(X) = coefficient array for Crank-Nicolson method
- b = abbreviated notation for $b(x,z)$
- $b(x,z)$ = right-hand boundary of estuary; upper limit of integration across the width, [L]
- B = seaward distance, [L]
- B = average bottom salinity (equation 4-8 only)
- B_i = constant coefficient for Crank-Nicolson method
- $B'_i = B_i - 2$
- BOD₂₀ = ultimate or 20-day BOD
- B(X) = coefficient array for Crank-Nicolson method
- BOD = concentration of biochemical oxygen demand
- C = salinity of the ocean or bay outside the estuary being studied (equation 4-8 only)

- C = concentration
 C_6 = concentration in segment 6
 C_A = concentration of constituent A
 \bar{C}_A = time averaged concentration of constituent A
 C'_A = perturbation component of the concentration of constituent A
 $C_{i,j}^{(k)}$ = concentration at location i,j , at time t (equation 4-17 only)
 C_0 = concentration at an outfall
 $C_{x,t}$ = concentration at location x , at time t
 $C_{x,z,t}$ = concentration at location x,z , at time t
 $CSTAR_{x,z}$ = intermediate concentration value at location x,z , at completion of the first phase of the implicit alternating-direction method
 C_{SAT} = saturation value for dissolved oxygen
 D = molecular diffusion coefficient, $[L^2T^{-1}]$
 D_A = molecular diffusion coefficient for constituent A, $[L^2T^{-1}]$
 D_i = constant coefficient for Crank-Nicolson method
 D'_0 = apparent diffusion coefficient at $x = 0$, the mouth of the estuary, $[L^2T^{-1}]$
 D_t = longitudinal coefficient of turbulent diffusion, $[L^2T^{-1}]$
 D'_x = apparent diffusion coefficient, $[L^2T^{-1}]$
 DO = dissolved oxygen concentration
 DS = degree of stratification
 $D(X)$ = array for Crank-Nicolson method
 e, \exp = base of natural logarithms, equals 2.718..., usually raised to a power

erfc = coerror function

E = dispersion coefficient, generally for the x-direction, $[L^2T^{-1}]$

E_i = turbulent diffusion component $[L^2T^{-1}]$

$E'_{k,k+1}$ = eddy exchange coefficient at interface between segment k and segment $k+1$, $[L^3T^{-1}]$

E_x, E_y, E_z = directional components of turbulent diffusion coefficient, $[L^2T^{-1}]$

$E_{x_{i,j}}$ = dispersion component in the x-direction at location i,j , $[L^2T^{-1}]$

$E_{y_{i,j}}$ = dispersion component in the y-direction at location i,j , $[L^2T^{-1}]$

EX = dispersion coefficient for the x-direction, $[L^2T^{-1}]$

$EX_{x,z}$ = dispersion coefficient for the x-direction at location x,z , at time t , $[L^2T^{-1}]$

$EXNEXT_{x,z}$ = dispersion coefficient for the x-direction at location x,z at time $t + \Delta t$, $[L^2T^{-1}]$

$EXPON1$ = exponent expression occurring in an extrapolation technique

$EXPON2$ = exponent expression occurring in an extrapolation technique

EZ = dispersion coefficient for the z-direction, $[L^2T^{-1}]$

$EZ_{x,z}$ = dispersion coefficient for the z-direction at location x,z , at time t , $[L^2T^{-1}]$

$EZNEXT_{x,z}$ = dispersion coefficient for the z-direction at location x,z , at time $t + \Delta t$, $[L^2T^{-1}]$

f = bed resistance coefficient, [dimensionless]

Fo = FROUDE number, [dimensionless]

g = acceleration of gravity, $[LT^{-2}]$

G = rate of energy dissipation per unit mass of fluid, $[L^2T^{-3}]$

- $G(X)$ = coefficient array for Crank-Nicolson method
- h = mean depth, [L]
- H = tidal height, [L]
- H = time zero, point in time at which tracer is released
- H = water level elevation relative to the local mean sea level datum (equations 4-13 and 4-15 only), [L]
- J = rate of gain of potential energy per unit mass of fluid, [L²T⁻³]
- J_k = direct sources of biochemical oxygen demand in segment k ; added concentration per unit time
- K = frictional resistance term, [L⁻¹]
- K = Van Dorn coefficient for wind stress (equations 4-13 and 4-14 only), [dimensionless]
- K_d = decay rate; removal coefficient to the base e , [T⁻¹]
- KD = decay rate to the base e , [T⁻¹]
- KD_x = decay rate, to the base e , at location x , [T⁻¹]
- L_k = concentration of ultimate carbonaceous biochemical oxygen demand in estuary segment k
- m = amount per unit cross-sectional area for an instantaneous release into a one-dimensional estuary
- m = amount per unit cross-sectional area per unit time for a continuous release into a one-dimensional estuary
- m = amount per unit depth for an instantaneous release into a two-dimensional estuary
- N = number of points
- \vec{N} = mass flux, [ML⁻²T⁻¹]
- \vec{N}_A = mass flux of constituent A across a boundary, [ML⁻²T⁻¹]
- N_x, N_y, N_z = directional components of the mass flux, [ML⁻²T⁻¹]
- NX = number of points in the x -direction

- NZ = number of points in the z-direction
- ppm = parts per million parts
- P_t = tidal prism, $[L^3]$
- Q = flowrate, $[L^3T^{-1}]$
- Q = flowrate of water in pounds per day (equations 8-6 and 8-11 only)
- Q = vector average of transport per unit width (equations 4-13 and 4-14 only), $[L^2T^{-1}]$
- Q_f = fresh water discharge, $[L^3T^{-1}]$
- $Q_{k,k+1}$ = net flow across the interface between segment k and segment k+1, $[L^3T^{-1}]$
- r = time rate of production, $[ML^{-3}T^{-1}]$
- r_A = time rate of production of constituent A, $[ML^{-3}T^{-1}]$
- R = rainfall rate, $[LT^{-1}]$
- \bar{s}_{lws} = local salinity at low water slack
- s_o = ocean salinity
- S_i = term defined for Gaussian elimination method
- SLUG = point concentration used for an instantaneous release
- t = time, $[T]$
- T = tidal period, $[T]$
- u = net velocity, $[LT^{-1}]$
- u_o = maximum flood tide velocity at $x = 0$, $[LT^{-1}]$
- U = advective velocity, generally in the x-direction, $[LT^{-1}]$
- U = vertically integrated x-component of transport per unit width (equations 4-13 to 4-15 only), $[L^2T^{-1}]$
- $U_{i,j}$ = velocity component in the x-direction at location i,j, $[LT^{-1}]$

- V = advective velocity, generally in the y-direction, $[LT^{-1}]$
 V = vertically integrated y-component of transport per unit width (equations 4-13 to 4-15 only), $[L^2T^{-1}]$
 \vec{V} = velocity vector, $[LT^{-1}]$
 V_i = a time-averaged component of velocity, $[LT^{-1}]$
 V'_i = perturbation component for turbulent velocity, $[LT^{-1}]$
 $V_{i,j}$ = velocity component in the y-direction at location i,j, $[LT^{-1}]$
 V'_x = perturbation component in the x-direction for turbulent velocity, $[LT^{-1}]$
 V_x, V_y, V_z = directional velocity components, $[LT^{-1}]$
 VX = velocity component in the x-direction, $[LT^{-1}]$
 $VX_{x,z}$ = velocity component in the x-direction at location x,z, at time t, $[LT^{-1}]$
 $VXNEXT_{x,z}$ = velocity component in the x-direction at location x,z, at time t + Δt , $[LT^{-1}]$
 VZ = velocity component in the z-direction, $[LT^{-1}]$
 $VZ_{x,z}$ = velocity component in the z-direction at location x,z, at time t, $[LT^{-1}]$
 $VZNEXT_{x,z}$ = velocity component in the z-direction at location x,z, at time t + Δt , $[LT^{-1}]$
 VOL_k = volume of segment k
 W = $b(x,z) - a(x,z)$; width of estuary at location x,y, $[L]$
 W = wind speed 10 meters above the water (equations 4-13 and 4-14 only), $[LT^{-1}]$
 W = loading rate in pounds per day (equations 8-6 to 8-11 only)
 W_i = collection of terms for Crank-Nicolson method
 W_x = width at location x, $[L]$

x = longitudinal direction or distance, [L]

y = lateral direction or distance, [L]

z = vertical direction or distance, [L]

Z = depth of water, [L]

Symbols

α_i = term defined for Gaussian elimination method

Δt = time increment, [T]

Δx = x-increment, [L]

Δy = y-increment, [L]

Δz = z-increment, [L]

θ = weighting factor for finite difference equation

$\varepsilon_{k,k+1}$ = advective coefficient dependent upon the ratio of dispersion to advective forces at interface between segment k and segment $k+1$, [dimensionless]

Π = 3.14159.....

ρ = mass density, [ML⁻³]

ρ_A = mass density of a mixture with constituent A, [ML⁻³]

ψ = angle between the wind velocity vector and the x-axis

Subscripts

1,2, etc. = location

A = constituent A

d = decay

f = fresh water

i = a general index

i = location i
 j = location j
 k = segment designation in a segmented estuary
 NX = value at location NX , the final grid point in the x -direction
 NZ = value at location NZ , the final grid point in the z -direction
 o = origin; $x = 0$
 o = outfall
 o = ocean (equation 4-12 only)
 t = tide (equation 4-10 only)
 t = time
 t = turbulence
 x = the x -component; in the longitudinal direction
 y = the y -component; in the lateral direction
 z = the z -component; in the vertical direction

Superscripts

(k) = at time t
 $(k+1)$ = at time $t + \Delta t$
 $'$ = a perturbation component resulting from turbulent fluctuations
 $'$ = apparent value
 $'$ = dispersion term for Thomann method

APPENDIX III. - COMPUTER PROGRAMS AND DATA

IDEAL-I

IDEAL-II

MASSTRANS-I

MASSTRANS-II

OXTRANS-I

STABLE-I

STABLE-II

EXACT-I

EXACT-II

PROFILE-I

PROFILE-II

COMPUTER PROGRAM FOR
IDEAL-I

Object Code = 7912 bytes

Array Area = 1616 bytes

Total = 9528 bytes


```

814 REAL LOAD,LOWER,LMAX,MASS
815 INTEGER COUNT
816 DIMENSION CT(101),CTI(101),LOAD(101),X(101)
C
C-----
C ***INITIALIZE TERMS AND READ INPUT DATA***
C-----
817 DATA CT,CTI,LOAD,X/404*0.0/
818 READ ,N,DELT,DELX,E,XKD,U,NC,NS,LTYPE,NDTPR,NSKIP,TBEGIN,TSTOP
819 PRINT,N,DELT,DELX,E,XKD,U,NC,NS,LTYPE,NDTPR,NSKIP,TBEGIN,TSTOP
820 DO 3 J=1,NC
821 READ, JJ, LOAD(JJ)
822 CT(JJ)=LOAD(JJ)
823 3 CONTINUE
824 IF(LTYPE.NE.3) GO TO 4
825 READ, MASS
826 4 CONTINUE
827 NSEG=N-1
828 MSU=1
829 MSL=N
830 UPPER=0.0
831 LOWER=(N-1)*DELX
832 UMAX=LOAD(1)
833 LMAX=LOAD(N)
C
C-----
C ***ESTABLISH THE LOCATIONS OF THE UPPERMOST
C AND LCWERMOST WASTE SOURCES***
C-----

```

```

834      DO 6 I=1,NSEG
835      IF (LOAD(I+1).LT.LOAD(I)) GO TO 7
836      MSU=I+1
837      UPPER=(I)*DELX
838      UMAX=LOAD(I+1)
839      CONTINUE
840      6 DO 8 I=1,NSEG
841      IF (LOAD(N-I).LT.LOAD(N+1-I)) GO TO 9
842      MSL=N-I
843      LOWER=(MSL-1)*DELX
844      LMAX=LOAD(N-I)
845      CONTINUE
846      8 CONTINUE
      9 CONTINUE
C-----
C      **DETERMINE THE INITIAL VALUE OF COUNT**
C-----
847      COUNT=(TBEGIN/DELT + 0.01)
848      COUNT=COUNT-(COUNT/NDTPR)*NDTPR
C-----
C      **DETERMINE STABILITY CRITERIA**
C-----
849      STABLX=DELX
850      STABLT=DELT
851      IF (E.GT.0.0.AND.U.GT.0.0) STABLX=2.*E/U
852      TERM=2.*E+DELX*DELX*XKD
853      IF (TERM.NE.0.0) STABLT=(DELX*DELX)/TERM

```

```

C
C-----
C  **PRINT DATA VALUES**
C-----
854 PRINT 10
855 10 FORMAT(IH1)
856 PRINT 11
857 11 FORMAT(IH1,///, 15X, *****DISPERSION CALCULATIONS BY THE EXPLI
      $T METHOD*****, ///)
858 PRINT 12, N
859 12 FORMAT(22X, ' NUMBER OF POINTS = ', I3,/)
860 SECS=DELT*86400.
861 PRINT 13, DELT,SECS
862 13 FORMAT(25X, 'TIME INCREMENT = ', F7.4, ' DAYS ( ', F8.2, ' SECONDS )
      $',/)
863 FEET=DELX*5280.
864 PRINT 14, DELX, FEET
865 14 FORMAT(21X, 'DISTANCE INCREMENT = ', F7.3, ' MILES ( ', F8.2, ' FE
      $ET )',/)
866 EFEET=E*5280.*5280./86400.
867 PRINT 15, E,EFEET
868 15 FORMAT(17X, 'DISPERSION COEFFICIENT = ', F7.2, ' MILES SQUARED/DAY
      $ ( ', F7.2, ' FEET SQUARED/SEC )',/)
869 PRINT 16, XKD
870 16 FORMAT(22X, 'DECAY COEFFICIENT = ', F7.2, ' PER DAY',/)
871 UF=U*5280./86400.
872 PRINT 17, U,UF
873 17 FORMAT(31X, 'VELOCITY = ', F7.2, ' MILES/DAY ( ', F6.3, ' FEET/SEC
      $ )',/)
874 PRINT 18, NC
875 18 FORMAT(1X, 'NUMBER OF KNOWN INITIAL CONCENTRATIONS = ', I3,/)

```

```

876 PRINT 185, NS
877 FORMAT(22X, 'NUMBER OF SOURCES = ', I3,/)
878 PRINT 19, LTYPE
879 FORMAT( 16X, 'TYPE OF LOADING (LTYPE) = ', I2,/)
880 DTPR=NDTPR*DELT
881 WRITE(6,20)DTPR
882 FORMAT(16X, 'PRINTOUT TIME INCREMENT = ', F7.4, ' DAYS',/)
883 DXPR=NSKIP*DELX
884 WRITE(6,22) DXPR
885 FORMAT(12X, 'PRINTOUT DISTANCE INCREMENT = ', F7.4, ' MILES',/)
886 PRINT 221, STABLX
887 FORMAT( 13X, 'MAXIMUM STABLE X INCREMENT = ', F7.3, ' MILES',/)
888 PRINT 222, STABLT
889 FORMAT( 13X, 'MAXIMUM STABLE T INCREMENT = ', F7.4, ' DAYS',/)
890 PRINT 23, TBEGIN, TSTOP
891 FORMAT(26X, 'RANGE OF TIME = ', F5.3, ' TO ', F8.3, ' DAYS',/,IHI)
892 T=TBEGIN
893 WRITE(6,24)
894 FORMAT(1X, 119(' '),/)

C-----
C ***CALCULATE DISTANCES TO BE PRINTED***
C-----
895 DO 25 IJ=1,N,NSKIP
896 X(IJ)=(IJ-1)*DELX
897 25 CONTINUE
898 NNI=1+NSKIP

C-----
C ***PRINT DISTANCES***
C-----

```

```

899      WRITE(6,27) (X(IJ), IJ=NN1,N,NSKIP)
900      27 FORMAT(' X VALUES IN MILES', 10F10.2,/,1(18X,10F10.2))
901      WRITE(6,24)
C
C-----
C      ***PRINT INITIAL CONCENTRATIONS***
C-----
902      WRITE(6,32)
903      32 FORMAT(4X, 'TIME',45X, 'CONCENTRATIONS (PPM)',/)
904      WRITE(6,35)T,(CT(J),J=1,N,NSKIP)
905      35 FORMAT(F8.2,F10.4,10F10.4,/,1(18X,10F10.4))
C
C-----
C      ***CALCULATE CONSTANT COEFFICIENTS FOR
C      THE FINITE DIFFERENCE EQUATION***
C-----
906      COEF1=E*DELX/(DELX**2.)
907      COEF2=U*DELX/(2.*DELX)
908      COEF3=XKD*DELX
909      QUIT=(TSTOP-TBEGIN)/DELX
910      NOMORE=QUIT+1
C
C-----
C      ***BEGIN LCOP WHICH IS REPEATED FOR EACH TIME INCREMENT***
C-----
911      DO 200 JJ=1,NOMORE
912      T=T+DELX
913      COUNT=COUNT+1

```

```

C-----
C ***APPLY EXPLICIT FINITE DIFFERENCE EQUATION
C TO EACH INTERNAL POINT***
C-----
914 DO 50 K=2,NSEG
915 CT1(K)=COEF1*(CT(K+1))-2*CT(K)+CT(K-1))-COEF2*(CT(K+1))-CT(K-1))-COE
2F3*CT(K)+CT(K)
916 50 CONTINUE
C-----
C ***DETERMINE IF CURVE IS TO BE EXTENDED UPSTREAM AND
C APPLY THE APPROPRIATE BOUNDARY CONDITIONS***
C-----
917 IF(MSU.EQ.1.AND.LTYPE.EQ.1) GO TO 55
918 IF(LTYPE.NE.2) GO TO 53
919 XUP=UPPER-U*TBEGIN
920 EXPON= (+2.*(-XUP +DELX) * DELX-(DELX**2.) - 2.*DELX*U*T) /
$ (4.*E*T)
921 CT1(1)=CT1(2)*EXP(EXPON)
922 GO TO 54
923 53 CONTINUE
924 CT1(1)=2.*CT1(2)-CT1(3)
925 54 CONTINUE
926 IF(CT1(3).LE.0.001) GO TO 55
927 IF(CT1(1).LT.0.0) CT1(1)=CT1(2)*CT1(2)/CT1(3)
928 55 CONTINUE
929 IF(CT1(1).LT.0.0) CT1(1)=0.0
C-----
C ***DETERMINE IF CURVE IS TO BE EXTENDED DOUNSTREAM AND
C APPLY THE APPROPRIATE BOUNDARY CONDITIONS***
C-----

```

```

930 IF(MSL.EQ.N.AND.LTYPE.EQ.1) GO TO 59
931 IF(LTYPE.NE.2) GC TO 57
932 XLOW=LOWER-U*TBEGIN
933 EXPON= (-2.*((N-2)*DELX-XLOW) * DELX- (DELX**2.) + 2.*DELX*U*T)/
      $ (4.*E*T)
934 CT1(N)=CT1(N-1)*EXP(EXPON)
935 GO TO 58
936 57 CONTINUE
937 CT1(N)=2.*CT1(N-1)-CT1(N-2)
938 58 CONTINUE
939 IF(CT1(N-2).LE.0.001) GO TO 59
940 IF(CT1(N).LT.0.0) CT1(N)=CT1(N-1)*CT1(N-1)/CT1(N-2)
941 59 CONTINUE
942 IF(CT1(N).LT.0.0) CT1(N)=0.0
C-----
C **APPLY APPROPRIATE LOADING CONDITIONS**
C-----
943 IF(LTYPE.EQ.1) GC TO 60
944 IF(LTYPE.EQ.3) CALL CNTINU(E,T,MASS,CT1(MSU))
945 GO TO 70
946 60 CONTINUE
947 CT1(MSU)=UMAX
948 CT1(MSL)=LMAX
949 70 CONTINUE
950 IF(COUNT.GE.NDTPR) GO TO 75
951 GO TO 100
C-----
C **PRINT CONCENTRATIONS FOR EACH DESIRED TIME INCREMENT**
C-----
952 75 WRITE(6,80) T,(CT1(M),M=1,N,NSKIP)
953 80 FORMAT(/, F8.3, F10.4, 10F10.4, /, 1(18X, 10F10.4))

```



```

954      COUNT=0
955      DO 150 L=1,N
956        IF(CT1(L).LT.0.001) CT1(L)=0.0
957        CT(L)=CT1(L)
958      200 CONTINUE
959      1000 CONTINUE
C
C-----
C      **DETERMINE THE NUMBER OF CALCULATIONS MADE BY THE PROGRAM
C-----
960      NCALC=3*NS + 10*NSEG + 2*N/NSKIP + NOMORE*(5*NSEG + 32)
961      INDEX=N*NOMORE
962      PRINT 205, N,NOMORE,NCALC, INDEX
963      205 FORMAT(///, I3, 'N=', I3, 5X, 'NOMORE=', I5, 5X, 'NCALC=', I7,
          $ 5X, 'INDEX=', I6,///)
          STOP
          END
C
C-----
966      SUBROUTINE CNTINU(EX,TIME,XMASS,CONCIN)
C
C      THIS SUBROUTINE SHOULD BE WRITTEN ACCORDING TO THE
C      PARTICULAR LOADING RATE OF THE OUTFALL BEING STUDIED.
C      THE SUBROUTINE WRITTEN BELOW IS VALID ONLY IN THE CASE OF
C      ONE OUTFALL AND WHERE NO VELOCITY IS PRESENT.
C
C      THIS SUBROUTINE IS USED ONLY IF LTYPE EQUALS 3.
C
C-----
967      RAD=3.14159*EX
968      CONCIN=(XMASS*TIME**.5)/SQRT(RAD)
969      RETURN
970      END

```

Input Data for IDEAL-I

Each line represents a new card unless single spaced.

51, 0.002, 0.10, 1., 0., 5., 1, 1, 2, 25, 1, 0., 0.6

11

18939.4

COMPUTER PROGRAM FOR
IDEAL-II

Object Code = 10,968 bytes

Array Area = 2,828 bytes

Total = 13,796 bytes

C THIS PROGRAM WAS WRITTEN PRIMARILY TO TEST THE ACCURACY
 C OF A FINITE DIFFERENCE METHOD AS COMPARED WITH ANALYTICAL,
 C CLOSED FORM SOLUTIONS FOR THE ESTUARY MASS TRANSPORT EQUATION.
 C REPRESENTATIVE VALUES FOR DISPERSION, VELOCITY, AND DECAY RATES
 C SHOULD BE CHOSEN FOR THE ESTUARY BEING STUDIED. THESE VALUES
 C CAN BE USED SEVERAL TIMES IN THIS MODEL TO DETERMINE WHICH
 C TIME AND DISTANCE INCREMENTS GIVE SATISFACTORY ACCURACY.
 C STABILITY CRITERIA IS PRINTED BY THE PROGRAM AND SEVERE
 C INACCURACIES MAY RESULT IF THESE LIMITS ARE EXCEEDED.
 C

C THE PROGRAM CAN ANALYZE INSTANTANEOUS RELEASES, CONTINUOUS
 C RELEASES, OR STEADY-STATE PROFILES. EXPONENTIAL EXTRAPOLATION
 C AT THE BOUNDARIES IS USED FOR INSTANTANEOUS RELEASES. FOR
 C OTHER TYPES OF LOADING, A CONSTANT SLOPE EXTRAPOLATION IS USED.
 C

C INFORMATION REGARDING THIS COMPUTER PROGRAM CAN BE
 C OBTAINED FROM JONATHAN YOUNG AT HYDROSCIENCE, INC., WESTWOOD,
 C NEW JERSEY.
 C

C *****


```

2997 REAL LOAD,LOWER,LMAX,MASS
2998 INTEGER COUNT
2999 DIMENSION CT(101),CTI(101),LOAD(101),X(101),W(101),ALPHA(101),
      $ S(101)
C-----
C **INITIALIZE TERMS AND READ INPUT DATA**
C-----
3000 DATA CT,CTI,LOAD,X,W,ALPHA,S/707*0.0/
3001 READ ,N,DELT,DELX,E,XKD,U,NC,NS,LTYPE,NDTPR,NSKIP,TBEGIN,TSTOP
3002 PRINT,N,DELT,DELX,E,XKD,U,NC,NS,LTYPE,NDTPR,NSKIP,TBEGIN,TSTOP
3003 DO 3 J=1,NC
3004 READ, JJ, LGAD(JJ)
3005 CT(JJ)=LOAD(JJ)
3006 3 CONTINUE
3007 IF(LTYPE.NE.3) GO TO 4
3008 READ, MASS
3009 4 CONTINUE
3010 NSEG=N-1
3011 MSU=1
3012 MSL=N
3013 UPPER=0.0
3014 LOWER=(N-1)*DELX
3015 UMAX=LOAD(1)
3016 LMAX=LOAD(N)
C-----
C **ESTABLISH THE LOCATIONS OF THE UPPERMOST
C AND LOWERMOST WASTE SOURCES**
C-----

```



```

3017 DO 6 I=1,NSEG
3018 IF (LOAD(I+1).LT.LOAD(I)) GO TO 7
3019 MSU=I+1
3020 UPPER=(I)*DELX
3021 UMAX=LOAD(I+1)
3022 6 CONTINUE
3023 7 DO 8 I=1,NSEG
3024 IF (LOAD(N-I).LT.LOAD(N+1-I)) GO TO 9
3025 MSL=N-I
3026 LOWER=(MSL-1)*DELX
3027 LMAX=LOAD(N-I)
3028 8 CONTINUE
3029 9 CONTINUE
C
C-----
C
C ***DETERMINE THE INITIAL VALUE OF COUNT***
C-----
C
3030 COUNT=(TBEGIN/DELT + 0.01)
3031 COUNT=COUNT-(COUNT/NDTPR)*NDTPR
C
C-----
C
C ***DETERMINE STABILITY CRITERIA***
C-----
C
3032 STABLX=DELX
3033 STABLT=DELT
3034 IF (E.GT.0.0.AND.U.GT.0.0) STABLX=2.*E/U
3035 TERM=2.*E+DELX*DELX*XKD
3036 IF (TERM.NE.0.0) STABLT=2.*(DELX*DELX)/TERM
C

```

```

C-----
C  **PRINT DATA VALUES**
C-----
3037 PRINT 10
3038 10 FORMAT(1H1)
3039 PRINT 11
3040 11 FORMAT(1H1,///, 15X, *****DISPERSION CALCULATIONS BY THE CRANK-N
      $ICOLSON IMPLICIT METHOD*****,///)
3041 PRINT 12, N
3042 12 FORMAT(22X, ' NUMBER OF PCINTS = ', I3,/)
3043 SECS=DELT*86400.
3044 PRINT 13, DELT,SECS
3045 13 FORMAT(25X, 'TIME INCREMENT = ', F7.4, ' DAYS ( ', F8.2, ' SECONDS )
      $',/)
3046 FEET=DELT*5280.
3047 PRINT 14, DELX, FEET
3048 14 FORMAT(21X, 'DISTANCE INCREMENT = ', F7.3, ' MILES ( ', F8.2, ' FE
      $ET )',/)
3049 EFEE=E*5280.*5280./86400.
3050 PRINT 15, E,EFEET
3051 15 FORMAT(17X, 'DISPERSION COEFFICIENT = ', F7.2, ' MILES SQUARED/DAY
      $( ', F7.2, ' FEET SQUARED/SEC )',/)
3052 PRINT 16, XKD
3053 16 FORMAT(22X, 'DECAY COEFFICIENT = ', F7.2, ' PER DAY',/)
3054 UF=U*5280./86400.
3055 PRINT 17, U,UF
3056 17 FORMAT(31X, 'VELOCITY = ', F7.2, ' MILES/DAY ( ', F6.3, ' FEET/SEC
      $ )',/)
3057 PRINT 18, NC
3058 18 FORMAT(1X, 'NUMBER OF KNOWN INITIAL CONCENTRATIONS = ', I3,/)

```

```

3059 PRINT 185, NS
3060 185 FORMAT(22X, 'NUMBER OF SOURCES = ', I3,/)
3061 PRINT 19, LTYPE
3062 19 FORMAT( 16X, 'TYPE OF LOADING (LTYPE) = ', I2,/)
3063 DTPR=NDTPR*DELT
3064 WRITE(6,20)DTPR
3065 20 FORMAT(16X, 'PRINTOUT TIME INCREMENT = ', F7.4, ' DAYS',/)
3066 DXPR=NSKIP*DELTX
3067 WRITE(6,22)DXPR
3068 22 FORMAT(12X, 'PRINTOUT DISTANCE INCREMENT = ', F7.4, ' MILES',/)
3069 PRINT 221, STABLX
3070 221 FORMAT( 13X, 'MAXIMUM STABLE X INCREMENT = ', F7.3, ' MILES',/)
3071 PRINT 222, STABLT
3072 222 FORMAT( 13X, 'MAXIMUM STABLE T INCREMENT = ', F7.4, ' DAYS',/)
3073 PRINT 23, TBEGIN, TSTOP
3074 23 FORMAT(26X, 'RANGE OF TIME = ', F5.3, ' TO ', F8.3, ' DAYS',/,1HL)
3075 T=TBEGIN
3076 WRITE(6,24)
3077 24 FORMAT(1X, 119('**'),/)
C
C-----
C **CALCULATE DISTANCES TO BE PRINTED***
C-----
3078 DO 25 IJ=1,N,NSKIP
3079 X(IJ)=(IJ-1)*DELTX
3080 25 CONTINUE
3081 NN1=1+NSKIP
C
C-----
C **PRINT DISTANCES***
C-----

```

```

3082 WRITE(6,27) (X(IJ), IJ=MN1,N,NSKIP)
3083 27 FORMAT(' X VALUES IN MILES', 10F10.2,/,1(18X,10F10.2))
3084 WRITE(6,24)
C-----
C
C ***PRINT INITIAL CONCENTRATIONS***
C-----
3085 WRITE(6,32)
3086 32 FORMAT(4X, 'TIME',45X, 'CONCENTRATIONS (PPM)',/)
3087 WRITE(6,35)T,(CT(J),J=1,N,NSKIP)
3088 35 FORMAT(F8.2,F10.4,10F10.4,/,1(18X,10F10.4))
C-----
C
C ***CALCULATE CONSTANT COEFFICIENTS FOR
C THE FINITE DIFFERENCE EQUATION***
C-----
3089 A=E*DEL/(2.*DELX**2.)+U*DEL/(4.*DELX)
3090 B1=E*DEL/(DELX**2.)+XK0*DEL/2.+1.
3091 B2=B1-2.
3092 D=E*DEL/(2.*DELX**2.)-U*DEL/(4.*DELX)
3093 QUIT=(TSTOP-TBEGIN)/DEL
3094 NOMORE=QUIT+1
C-----
C
C ***BEGIN LCOP WHICH IS REPEATED FOR EACH TIME INCREMENT***
C-----
3095 DO 200 JK=1,NOMORE
3096 T=T+DEL
3097 COUNT=COUNT+1
C

```

```

3098
3099
3100
3101
3102
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3117

C-----
C ***SET THE UPPER BOUNDARY CONDITIONS***
C
C STATEMENT NUMBER 36 FOR EXPONENTIAL EXTRAPOLATION
C STATEMENT NUMBER 37 FOR CONSTANT SLOPE EXTRAPOLATION
C STATEMENT NUMBER 39 FOR CCNSTANT CONCENTRATION AT THE BOUNDARY
C-----
C IF(MSU.EQ.1.AND.LTYPE.EQ.1) GO TO 39
C IF(LTYPE.NE.2) GO TO 37
36 CONTINUE
C XUP=UPPER-U*TBEGIN
C EXPON= (+2.*(-XUP +DELX) * DELX-(DELX**2.) - 2.*DELX*U*T) /
C $ (4.*E*T)
C IF(EXPON.LT.-170.) EXPCN=-170.
C ALPHA(1)=-A*EXP(EXPCN)+B1
C W(1)=A*CT(1) -B2*CT(2)+D*CT(3)
C ALPHA(2)=B1-A*D/ALPHA(1)
C GO TO 41
37 CONTINUE
C ALPHA(1)=-2.*A+B1
C W(1)=A*CT(1)-B2*CT(2)+D*CT(3)
C ALPHA(2)=B1-A*(D-A)/ALPHA(1)
C GO TO 41
39 CONTINUE
C ALPHA(1)=B1
C W(1)=A*2.*CT(1)-B2*CT(2)+D*CT(3)
C ALPHA(2)=B1-A*D/ALPHA(1)
C 41 CONTINUE
C

```

```

C-----
C   **BUILD ARRAYS FOR MATRIX SOLUTION***
C-----
3118   MOST=NSEG-2
3119   DO 40 M=2,MOST
3120     W(M)=A*CT(M)-B2*CT(M+1)+D*CT(M+2)
3121     40 CONTINUE
C
C-----
C   **SOLVE TRIDIAGONAL MATRIX***
C-----
3122   S(1)=W(1)
3123   S(2)=W(2)+A*S(1)/ALPHA(1)
3124   DO 42 MM=3,MOST
3125     ALPHA(MM)=B1-A*D/ALPHA(MM-1)
3126     S(MM)=W(MM)+A*S(MM-1)/ALPHA(MM-1)
3127     42 CONTINUE
C
C-----
C   **SET LOWER BOUNDARY CONDITIONS***
C-----
3128   IF(LTYPE.EQ.1.AND.MSL.EQ.N) GO TO 43
3129   IF(LTYPE.NE.2) GO TO 44
3130   421 CONTINUE
3131   XLOW=LOWER-U*TBEGIN
C-----
C   STATEMENT NUMBER 421 FOR EXPONENTIAL EXTRAPOLATION
C   STATEMENT NUMBER 43 FOR CONSTANT CONCENTRATION AT THE BOUNDARY
C   STATEMENT NUMBER 44 FOR CONSTANT SLOPE EXTRAPOLATION
C-----

```

```

3132 EXPON= (-2.*((N-2)*DELX-XLOW) * DELX- (DELX**2.) + 2.*DELX*U*T)/
      $ (4.*E*T)
3133 IF (EXPON.LT.-170.) EXPCN=-170.
3134 B1F=B1-D*EXP(EXPON)
3135 ALPHA(NSEG-1)=B1F-A*D/ALPHA(NSEG-2)
3136 W(NSEG-1)=A*CT(NSEG-1)-B2*CT(NSEG)+D*CT(NSEG+1)
3137 S(NSEG-1)=W(NSEG-1)+A*S(NSEG-2)/ALPHA(NSEG-2)
3138 GO TO 45
3139 43 CONTINUE
3140 ALPHA(NSEG-1)=B1-A*D/ALPHA(NSEG-2)
3141 W(NSEG-1)=A*CT(NSEG-1)-B2*CT(NSEG)+2.*D*CT(NSEG+1)
3142 S(NSEG-1)=W(NSEG-1)+A*S(NSEG-2)/ALPHA(NSEG-2)
3143 GO TO 45
3144 44 CONTINUE
3145 ALPHA(NSEG-1)=(B1-2.*D)+(D-A)*D/ALPHA(NSEG-2)
3146 W(NSEG-1)=A*CT(NSEG-1)-B2*CT(NSEG)+D*CT(NSEG+1)
3147 S(NSEG-1)=W(NSEG-1)-(D-A)*S(NSEG-2)/ALPHA(NSEG-2)
3148 45 CONTINUE
3149 CT1(NSEG)=S(NSEG-1)/ALPHA(NSEG-1)
3150 NFINAL=NSEG-2
3151 DO 46 II=1,NFINAL
3152 CT1(NSEG-II)=(S(NSEG-II-1)+D*CT1(NSEG-II+1))/ALPHA(NSEG-II-1)
3153 46 CONTINUE
C-----
C
C ***DETERMINE IF CURVE IS TO BE EXTENDED UPSTREAM AND
C APPLY THE APPROPRIATE BOUNDARY CONDITIONS***
C-----
3154 IF (MSU.EQ.1.AND.LTYPE.EQ.1) GO TO 55
3155 IF (LTYPE.NE.2) GO TO 53
3156 XUP=UPPER-U*TBEGIN

```

```

3157     EXPON= (+2.*(-XUP +DELX) * DELX-(DELX**2.) - 2.*DELX*U*T) /
          $ (4.*E*T)
3158     CT1(1)=CT1(2)*EXP(EXPON)
3159     GO TO 55
3160     53 CONTINUE
3161     CT1(1)=2.*CT1(2)-CT1(3)
3162     55 CONTINUE
C
C-----
C     **DETERMINE IF CURVE IS TO BE EXTENDED DOWNSTREAM AND
C     APPLY THE APPROPRIATE BOUNDARY CONDITIONS**
C-----
3163     IF(MSL.FQ.N.AND.LTYPE.EQ.1) GO TO 59
3164     IF(LTYPE.NE.2) GO TO 57
3165     XLOW=LOWER-U*TBEGIN
3166     EXPON= (-2.*((N-2)*DELX-XLOW) * DELX- (DELX**2.) + 2.*DELX*U*T) /
          $ (4.*E*T)
3167     CT1(N)=CT1(N-1)*EXP(EXPCN)
3168     GO TO 59
3169     57 CONTINUE
3170     CT1(N)=2.*CT1(N-1)-CT1(N-2)
3171     59 CONTINUE
C
C-----
C     ***APPLY APPROPRIATE LOADING CONDITIONS***
C-----
3172     IF(LTYPE.EQ.1) GO TO 60
3173     IF(LTYPE.EQ.3) CALL CNTINU(E,T,MASS,CT1(MSU))
3174     GO TO 70
3175     60 CONTINUE

```



```

3176      CT1(MSU)=UMAX
3177      CT1(MSL)=LMAX
3178      7C CONTINUE
3179      DO 73 L=1,N
3180      IF(CT1(L).LT.0.001) CT1(L)=0.0
3181      CT(L)=CT1(L)
3182      73 CONTINUE
3183      IF(COUNT.GE.NDTPR) GO TO 75
3184      GO TO 200
C
C-----
C      ***PRINT CONCENTRATIONS FOR EACH DESIRED TIME INCREMENT***
C-----
3185      75 WRITE(6,80) T,(CT1(M),M=1,N,NSKIP)
3186      80 FORMAT(/, F8.3, F10.4, 10F10.4, /, 1(18X, 10F10.4))
3187      COUNT=0
3188      200 CONTINUE
3189      1000 CONTINUE
C
C-----
C      ***DETERMINE THE NUMBER OF CALCULATIONS MADE BY THE PROGRAM
C-----
3190      NCALC=3*NS + 10*NSEG + 2*N/NSKIP + NOMORE*(10*NSEG+46)
3191      INDEX=N*NOMORE
3192      PRINT 205, N,NOMORE,NCALC, INDEX
3193      205 FORMAT(///, 1X, 'N=', 13, 5X, 'NCMORE=', 15, 5X, 'NCALC=', 17,
          $ 5X, 'INDEX=', 16,///)
3194      STOP
3195      END

```

```
3196 C-----  
      C SUBROUTINE CNTINU(EX, TIME, XMASS, CONCIN)  
      C  
      C THIS SUBROUTINE SHOULD BE WRITTEN ACCORDING TO THE  
      C PARTICULAR LOADING RATE OF THE OUTFALL BEING STUDIED.  
      C THE SUBROUTINE WRITTEN BELOW IS VALID ONLY IN THE CASE OF  
      C ONE OUTFALL AND WHERE NO VELOCITY IS PRESENT.  
      C  
      C THIS SUBROUTINE IS USED ONLY IF LTYPE EQUALS 3.  
      C  
      C-----  
3197 RAD=3.14159*FX  
3198 CONCIN=(XMASS*TIME**.5)/SQRT(RAD)  
3199 RETURN  
3200 END
```

Input Data for IDEAL-II

Each line represents a new card unless single spaced.

15, 0.002, 0.1, 1., .23, 5.0, 2, 2, 1, 1, 1, 0.0, 0.022

4

1000.

15

1000.

COMPUTER PROGRAM FOR
MASSTRANS-I

Object Code = 54,728 bytes

Array Area = 40,196 bytes

Total = 94,924 bytes

C THIS COMPUTER PROGRAM WAS DESIGNED PRIMARILY TO ANALYZE THE
 C MASS TRANSPORT OF DISSOLVED MATERIALS IN A PARTIALLY STRATIFIED
 C ESTUARY. THESE MATERIALS MAY BE CONSERVATIVE OR NON-CONSERVATIVE.
 C AN EXPLICIT FINITE DIFFERENCE APPROXIMATION TO THE ESTUARY MASS
 C TRANSPORT EQUATION FORMS THE BASIS OF THE COMPUTATIONS.
 C

C THIS PROGRAM WAS DEVELOPED TO ANALYZE ESTUARIES WHOSE
 C CHARACTERISTICS DO NOT VARY SIGNIFICANTLY WITH WIDTH. THE WIDTH
 C OF THE ESTUARY MAY BE VARIED THROUGHOUT BUT THE CONCENTRA-
 C TIONS OF DISSOLVED MATERIALS AT EACH CROSS SECTION ARE CONSIDERED
 C TO BE UNCHANGING IN THE LATERAL DIRECTION. ALL PHYSICAL AND
 C HYDRODYNAMIC CHARACTERISTICS MAY VARY WITH TIME IN THE LONGITU-
 C DINAL AND VERTICAL DIRECTIONS. THE PROGRAM CAN BE APPLIED WITH
 C EQUAL EASE TO THE TWO HORIZONTAL DIRECTIONS, ALLOWING DEPTH TO
 C VARY RATHER THAN WIDTH.
 C

C CONCENTRATION PROFILES CAN BE CALCULATED BY THIS PROGRAM FOR
 C CONTINUOUS OR INSTANTANEOUS RELEASES. INPUT DATA MAY INCLUDE GRID
 C DIMENSIONS, DISTANCE INCREMENTS, TIME INCREMENTS, WIDTHS, LOADING
 C PARAMETERS, VELOCITIES, DISPERSION COEFFICIENTS, DECAY RATES,
 C BENTHAL DEMANDS, AND OTHER SOURCE OR SINK TERMS.
 C

C THE TIME INCREMENTS CAN BE INCREASED OR DECREASED AT ANY TIME
 C DURING THE CALCULATION OF THE CONCENTRATION PROFILE. THE DIS-
 C TANCES BETWEEN GRID POINTS CAN BE INCREASED AT ANY TIME, AND A
 C ROUTINE WITHIN THE PROGRAM WILL CHOOSE THE APPROPRIATE VALUES FROM
 C THE PREVIOUSLY CALCULATED PROFILE AND WILL PLACE THESE VALUES IN
 C THE DESIRED LOCATIONS IN THE NEW GRID SYSTEM. THE NUMBER OF GRID
 C POINTS MAY BE INCREASED OR DECREASED AT ANY TIME. LIKEWISE, AT
 C ANY TIME DURING THE CALCULATION, A NEW SET OF DATA FOR PHYSICAL
 C AND HYDRODYNAMIC CONDITIONS MAY BE READ INTO THE PROGRAM.
 C

C A USER OF THIS PROGRAM MUST BE FAMILIAR WITH THE LIMITATIONS
 C ON ACCURACY AND STABILITY INHERENT IN THE TYPE OF NUMERICAL PROCEDURE
 C USED IN THESE CALCULATIONS. A SUBROUTINE WITHIN THE PROGRAM
 C PRINTS OUT THE PROPER INCREMENTS TO INSURE STABILITY AND TERMINATES
 C THE PROGRAM IF THIS CRITERIA IS VIOLATED BY THE INPUT PARAMETERS.
 C ANOTHER SUBROUTINE EXTRAPOLATES CONCENTRATIONS AT THE BOUNDARIES--
 C SEVERAL METHODS CAN BE USED FOR THESE EXTRAPOLATIONS DEPENDING
 C ON THE TYPE OF PROFILE BEING ANALYZED AND THE CHOICE OF THE USER.
 C A SUBROUTINE IS ALSO INCLUDED WHICH PRINTS OUT ERROR MESSAGES
 C AND TERMINATES THE PROGRAM IF CERTAIN INCONSISTENCIES OCCUR
 C IN THE INPUT DATA.

C THIS PROGRAM WAS DEVELOPED PRIMARILY TO ANALYZE PARTIALLY
 C STRATIFIED ESTUARIES WHICH HAVE BEEN DREDGED OUT TO A FAIRLY
 C CONSTANT DEPTH AT THE CENTERLINE OF THE CHANNEL--THESE ESTUARIES
 C ARE COMMON IN THE GULF COAST REGION. APPLICATION OF THIS PROGRAM
 C TO PARTIALLY STRATIFIED ESTUARIES WITH VARIABLE DEPTHS WOULD
 C REQUIRE MODERATE REVISIONS TO THE PROGRAM AND WOULD MAKE THE
 C PROGRAM ESTUARY-DEPENDENT.

C THIS COMPUTER PROGRAM CAN ALSO BE APPLIED TO ESTUARIES WHICH
 C ARE WELL-MIXED IN THE VERTICAL DIRECTION. THIS OPTION ALLOWS FOR
 C VARYING WIDTH OR VARYING DEPTH AND USES MOST OF THE ROUTINES
 C AVAILABLE TO THE TWO-DIMENSIONAL ANALYSIS.

C QUESTIONS REGARDING THIS PROGRAM MAY BE REFERRED TO
 C JONATHAN YOUNG AT HYDROSCIENCE, INC, 363 OLD HOOK ROAD,
 C WESTWOOD, NEW JERSEY 07675 PHONE 201 / 666-2600



C *****NOTE---VALUES FOR VZ AND EZ ARE TO BE INCLUDED
 C WITH THE DATA ONLY IF NZ IS GREATER THAN 1
 C VZ(X,Z) = VERTICAL VELOCITIES (FT/SEC)
 C EZ(X,Z) = VERTICAL DISPERSION (FT**2/SEC)
 C
 C FMT1(I) = FORMAT FOR HEADING FOR COMPUTED PROFILES
 C FMT2(I) = FORMAT FOR DEPTH NOTATION FOR PROFILES
 C FMT3(I) = FORMAT FOR OUTPUT OF COMPUTED PROFILES
 C
 C
 C
 C

 C
 C *** GRID ADJUSTMENT PARAMETERS ***
 C
 C

C NXSKIP = NUMBER OF X INTERVALS SKIPPED IN OLD GRID
 C NZSKIP = NUMBER OF Z INTERVALS SKIPPED IN OLD GRID
 C NXTAKE = LOWEST GRID NUMBER IN X DIRECTION IN THE OLD GRID
 C FROM WHICH A CONCENTRATION IS TAKEN
 C NZTAKE = LOWEST GRID NUMBER IN Z DIRECTION IN THE OLD GRID
 C FROM WHICH A CONCENTRATION IS TAKEN
 C NXPUT = LOWEST GRID NUMBER IN THE X DIRECTION IN THE NEW
 C GRID INTO WHICH A CONCENTRATION IS PLACED
 C NZPUT = LOWEST GRID NUMBER IN THE Z DIRECTION IN THE NEW
 C GRID INTO WHICH A CONCENTRATION IS PLACED
 C

 ***** MAIN PROGRAM *****

THE MAIN PROGRAM CALLS FOR THE DATA TO BE READ IN, THEN CALLS FOR THE STABILITY CONDITIONS TO BE EVALUATED, AND NEXT CALLS FOR THE PRINTING OF A SUMMARY OF INPUT DATA. THE NUMERICAL ANALYSIS SUBROUTINE IS THEN CALLED FOR EITHER THE ONE-DIMENSIONAL OR TWO-DIMENSIONAL CASE. IF THE TIME INCREMENT IS TO BE CHANGED DURING THIS PART OF THE PROGRAM, THE APPROPRIATE PARAMETERS ARE ADJUSTED. STABILITY IS AGAIN CHECKED AND THE NUMERICAL ANALYSIS IS CONTINUED. NEXT, A NEW DATA SET MAY BE READ TO CONTINUE THE ANALYSIS. ANY NUMBER OF DATA SETS MAY BE PROCESSED. WHEN NO MORE DATA SETS ARE AVAILABLE, THE PROGRAM IS TERMINATED.

THE INPUT AND OUTPUT OF DATA CAN BE CONTROLLED IN THREE WAYS--THE INPUT DATA ITSELF, THE BLOCK DATA SUBROUTINE, AND THE SUBROUTINE PRINT2. NO OTHER PART OF THE PROGRAM SHOULD REQUIRE CHANGING.

THE FORTRAN USED IN THIS PROGRAM USES SEVERAL OF THE OPTIONS AVAILABLE IN WATFOR AS DESCRIBED IN THE FOLLOWING TEXT**FORTRAN IV WITH WATFOR AND WATFIV**BY CRESS, DIRKSEN, & GRAHAM, PRENTICE-HALL, INC., 1970. THE PROGRAM CAN BE RUN ON A COMPUTER WITH A WATFOR OR WATFIV COMPILER. THE SYNTAX HAS BEEN KEPT COMPATIBLE WITH FORTRAN 6 EXCEPT FOR THE UNFORMATTED READ AND PRINT STATEMENTS.

```

1  INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
2  REAL KD,KDMAX,KDMIN,KDAYS,LOWER
3  COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
4  W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
5  DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
6  COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
7  KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
8  TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
9  VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
10 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
11 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
12
13 10 CALL DATA(&1000)
14 CALL STABLE
15 CALL PRINT1
16 CALL PRINT2
17 CALL ERROR(&1000)
18 25 IF(NZ.EQ.1) GO TO 310
19 CALL TWODEX
20 GO TO 315
21 310 CALL ONEDEX
22 315 IF(ITER2.EQ.0) GO TO 10
23 ITER=ITER2
24 ITER2=0
25 TPRINT=TPRINT*(DELT1/DELT2+.00001)
26 DELTAT=DELT2*86400.
27 CALL STABLE
28 IF(ITER.EQ.0) CALL ERROR(&1000)
29 GO TO 25
30 1000 CONTINUE
31 STOP
32 END

```

```

C-----
25 C      BLOCK DATA
C      *****
C      ***** BLOCK DATA *****
C      *****
C      THIS ROUTINE INITIALIZES SELECTED VARIABLES IN THE
C      COMMON BLOCKS. THIS SUBPROGRAM MAY BE USED TO ESTABLISH
C      INITIAL CONCENTRATION PROFILES IF THE USER DESIRES.
C
C-----
26 C      INTEGER X,Z,T,TNEXT,TPRINT,KDAYS,XPRINT,COUNT,OPTION
27 C      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
28 C      COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
C      W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
C      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
29 C      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
C      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
C      TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
C      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
C      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
C      MSU,MSL,TIME,NXMI,NXM2,NZMI,NZM2,ZSF,LTYPE,OPTION
C
C-----
30 C      ESTABLISH INITIAL CONCENTRATION IF DESIRED
C
C-----
31 C      DATA C/2142*0.0/
32 C      DATA VX,VZ,EX,EZ,CONCIN,DEMAND/6426*0.0/
33 C      DATA W,KDAYS,KD/153*0.0/
34 C      DATA EXMAX,EZMAX,VXMAX,VZMAX,WMAX,KDMAX,DEMAX,TIME/8*0.0/
35 C      DATA EXMIN,EZMIN,VXMIN,VZMIN,WMIN,KDMIN,DEMIN/7*1000000.0/
36 C      DATA INSECT/51*0/
37 C      DATA IPAGE/1/
38 C      DATA T/1/,TNEXT/2/
C      END

```

```

39 C -----
39 C SUBROUTINE DATA(*)
39 C *****
39 C ***** SUBROUTINE DATA *****
39 C *****
39 C
39 C THIS SUBROUTINE READS IN THE APPROPRIATE DATA SET AND ADJUSTS
39 C THE GRID SIZE IF NECESSARY. THE LOCATIONS OF THE UPPERMOST AND
39 C LOWERMOST PEAKS ARE DETERMINED, AND THE MINIMUM AND MAXIMUM VALUES
39 C FOR THE INPUT PARAMETERS ARE CALCULATED. ALL OF THE INPUT DATA IS
39 C PRINTED OUT UNFORMATTED. THE FORMAT IS READ IN FOR THE PRINTING
39 C OUT OF THE CALCULATED CONCENTRATIONS.
39 C
40 C -----
40 C INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CCOUNT,OPTION
41 C REAL KD,KDMAX,KDMIN,KDAYS,LOWER
42 C COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
43 C W(51),CCNCIN(51,21),INSECT(51),KDAY5(51),KD(51),
43 C DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
43 C COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
43 C KDMAX,KDMIN,ITER,ITER1,ITER2,DEL1,DEL2,TNEXT,
43 C TPRINT,XPRINT,STABLX,STABLZ,STABLR1,R2,VZMAX,SET,
43 C VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAZ,DELTAZ,
43 C DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
43 C MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
44 C READ(5,12,END=13) SET
45 C FORMAT(F10.0)
46 C GO TO 14
47 C CONTINUE
48 C RETURN
49 C CONTINUE
50 C IF(SET.EQ.1.) GO TO 400

```

```

C-----
C      CHOOSE CONCENTRATIONS FROM THE OLD GRID TO PUT INTO THE
C      APPROPRIATE PLACES IN THE NEW GRID .  THE GRID CAN ONLY BE
C      CHANGED IF LTYPE EQUALS 2.  OTHERWISE, THE DATA CARD IS READ
C      BUT IS NOT IMPLEMENTED.
C-----
51  READ ,NXSKIP,NZSKIP,NXTAKE,NZTAKE,NXPUT,NZPUT
52  IF(LTYPE.NE.2) GO TO 400
53  NXSTOP=(NX-1)/NXSKIP+1
54  NZSTOP=(NZ-1)/NZSKIP+1
55  IF(NZ.EQ.1) NZSTOP=1
56  DO 350 I=1,NXSTOP
57  X=NXPUT-1+I
58  II=NXTAKE+NXSKIP*(I-1)
59  DO 350 J=1,NZSTOP
60  Z=NZPUT-1+J
61  JJ=NZTAKE+NZSKIP*(J-1)
62  C(X,Z,TNEXT)=C(II,JJ,T)
63  350 CONTINUE
64  400 CONTINUE
C-----
C      READ AND PRINT INPUT DATA FOR HORIZONTAL DIRECTION
C-----

```

```

65 READ,NX,NZ
66 READ,DELMX,DELTAZ
67 READ,DELT1,ITER1
68 READ,DELT2,ITER2
69 READ,NC,NS
70 READ,XSTART,TSTART,XPRINT,TPRINT,NPPAGE
71 READ,LTYPE,OPTION
72 PRINT 1001
73 1001 FORMAT( IHI,IOX,'INPUT DATA',//,
74 1 IX,'NX,NZ, DELMX,DELTAZ, DELT1,ITER1')
75 PRINT,NX,NZ,DELMX,DELTAZ,DELT1,ITER1
76 PRINT 1002
77 1002 FORMAT(//,IX,'DELT2,ITER2, NC,NS, XSTART,TSTART')
78 PRINT,DELT2,ITER2,NC,NS,XSTART,TSTART
79 PRINT 1003
80 1003 FORMAT(//,IX,'XPRINT,TPRINT,NPPAGE, LTYPE,OPTION')
81 PRINT, XPRINT,TPRINT,NPPAGE,LTYPE,OPTION
82 IF(SET.EQ.1.) GO TO 500
83 DO 375 Z=1,NZ
84 DO 375 X=1,NX
85 C(X,Z,T)=C(X,Z,TNEXT)
86 C(X,Z,TNEXT)=0.0
87 375 CONTINUE
88 500 CONTINUE
89 NXM1=NX-1
90 NXM2=NX-2
91 NZM1=NZ-1
92 NZM2=NZ-2
93 DELTAT=DELT1*86400.
DELTAX=DELMX*5280.

```



```

94 XSF=XSTART#5280.
95 ITER=ITER1
96 TIME=TIME+TSTART#86400.
97 COUNT=(TSTART/DELTA+0.01)
98 COUNT=COUNT-(COUNT/TPRINT)*TPRINT
99 IF(SET.EQ.2) RETURN
100 READ,((VX(X,Z),X=1,NX),Z=1,NZ)
101 PRINT 2
102 2 FORMAT(/, 1X, '((VX(X,Z),X=1,NX),Z=1,NZ)')
103 PRINT,((VX(X,Z),X=1,NX),Z=1,NZ)
104 READ,((EX(X,Z),X=1,NX),Z=1,NZ)
105 PRINT 3
106 3 FORMAT(/, 1X, '((EX(X,Z),X=1,NX),Z=1,NZ)')
107 PRINT,((EX(X,Z),X=1,NX),Z=1,NZ)
108 READ,(W(X),X=1,NX)
109 PRINT 4
110 4 FORMAT(/, 1X, '(W(X),X=1,NX)')
111 PRINT, (W(X),X=1,NX)
112 READ,(KDAY(X),X=1,NX)
113 PRINT 5
114 5 FORMAT(/, 1X, '(KDAY(X),X=1,NX)')
115 PRINT, (KDAY(X),X=1,NX)
116 READ,((DEMAND(X,Z),X=1,NX),Z=1,NZ)
117 PRINT 6
118 6 FORMAT(/, 1X, '((DEMAND(X,Z),X=1,NX),Z=1,NZ)')
119 PRINT, ((DEMAND(X,Z),X=1,NX),Z=1,NZ)
120 IF(NC.EQ.0) GO TO 24
121 DO 10 I=1,NC
122 READ,INSECT(I)
123 PRINT 7

```

```

124 7 FORMAT(/, IX, 'INSECT(I)')
125 PRINT, INSECT(I)
126 READ, (CONCIN(I,J), J=1,NZ)
127 PRINT 8
128 8 FORMAT(/, IX, '(CONCIN(I,J),J=1,NZ)')
129 PRINT, (CONCIN(I,J),J=1,NZ)
130 10 CONTINUE
131 DO 15 I=1,NC
132 DO 15 Z=1,NZ
133 C(INSECT(I),Z,T)=CONCIN(I,Z) +C(INSECT(I),Z,T)
134 15 CONTINUE
C
C-----
C DETERMINE LOCATIONS OF UPPERMOST AND LOWERMOST
C PEAK CONCENTRATIONS
C-----
135 AMAX1=0.0
136 PEAKU=0.0
137 AMAX2=0.0
138 PEAKL=0.0
139 UPPER=XSF
140 LOWER=XSF
141 MSU=1
142 MSL=NX
143 DO 17 I=1,NC
144 DO 16 J=1,NZ
145 IF(CONCIN(I,J).LT.PEAKU) GO TO 16
146 PEAKU=CONCIN(I,J)
147 16 CONTINUE
148 16 IF(PEAKU.LE.AMAX1) GO TO 20

```

```

149 AMAX1=PEAKU
150 MSU=INSECT(I)
151 UPPER=(MSU-1)*DELTA + XSF
152 17 CONTINUE
153 20 CONTINUE
154 DO 22 II=1,NC
155 I=NC+1-II
156 DO 21 J=1,NZ
157 IF(CONCIN(I,J).LT.PEAKL) GO TO 21
158 PEAKL=CONCIN(I,J)
159 21 CONTINUE
160 IF(PEAKL.LE.AMAX2) GO TO 24
161 AMAX2=PEAKL
162 MSL=INSECT(I)
163 LOWER=(MSL-1)*DELTA + XSF
164 22 CONTINUE
165 24 CONTINUE

```

C
C
C
C

 DETERMINE MAXIMUM AND MINIMUM VALUES FOR INPUT DATA

```

166 DO 25 X=1,NX
167 KD(X)=KDAY(X)/86400.
168 IF(W(X).GT.WMAX)WMAX=W(X)
169 IF(W(X).LT.WMIN)WMIN=W(X)
170 IF(KD(X).GT.KDMAX)KDMAX=KD(X)
171 IF(KD(X).LT.KDMIN)KDMIN=KD(X)
172 DO 25 Z=1,NZ
173 ABSVX=ABS(VX(X,Z))
174 IF(ABSVX.GT.VXMAX)VXMAX=ABSVX

```

```

175 IF(ABSVX.LT.VXMIN)VXMIN=ABSVX
176 IF(EX(X,Z).GT.EXMAX)EXMAX=EX(X,Z)
177 IF(EX(X,Z).LT.EXMIN)EXMIN=EX(X,Z)
178 IF(DEMAND(X,Z).GT.DEMAX)DEMAX=DEMAND(X,Z)
179 IF(DEMAND(X,Z).LT.DEMIN)DEMIN=DEMAND(X,Z)
180 25 CONTINUE
181 IF(NZ.EQ.1)GO TO 200
C
C-----
C READ AND PRINT DATA FOR VERTICAL DIRECTION
C-----
182 READ,((VZ(X,Z),X=1,NX),Z=1,NZ)
183 PRINT 30
184 30 FORMAT(/, 1X, '((VZ((X,Z),X=1,NX),Z=1,NZ)')
185 PRINT,((VZ(X,Z),X=1,NX),Z=1,NZ)
186 READ,((EZ(X,Z),X=1,NX),Z=1,NZ)
187 PRINT 32
188 32 FORMAT(/, 1X, '((EZ(X,Z),X=1,NX),Z=1,NZ)')
189 PRINT, ((EZ(X,Z),X=1,NX),Z=1,NZ)
C
C-----
C DETERMINE MAXIMUM AND MINIMUM VALUES FOR INPUT DATA
C-----
190 DO 120 X=1,NX
191 DO 120 Z=1,NZ
192 ABSVZ=ABS(VZ(X,Z))
193 IF(ABSVZ.GT.VZMAX)VZMAX=ABSVZ
194 IF(ABSVZ.LT.VZMIN)VZMIN=ABSVZ
195 IF(EZ(X,Z).GT.EZMAX)EZMAX=EZ(X,Z)
196 IF(EZ(X,Z).LT.EZMIN)EZMIN=EZ(X,Z)
197 120 CONTINUE
198 200 CONTINUE

```

```
C-----  
C      READ AND PRINT FORMATS FOR OUTPUT  
C-----  
199 READ 205, (FMT1(I), I=1,40)  
200 PRINT 202  
201 FORMAT(//,1X, 'FORMAT FOR OUTPUT')  
202 PRINT 205, (FMT1(I), I=1,40)  
203 FORMAT (20A4,/,20A4)  
204 READ 205,(FMT2(I),I=1,40)  
205 PRINT 205, (FMT2(I), I=1,40)  
206 READ 207,(FMT3(I), I=1,20)  
207 PRINT 207, (FMT3(I), I=1,20)  
208 FORMAT(20A4)  
209 PRINT 259  
210 FORMAT(IH1)  
211 RETURN  
212 END
```

```

213 C-----
C     SUBROUTINE STABLE
C
C     *****
C     ***** SUBROUTINE STABLE *****
C     *****
C
C     THE RELATIONSHIP BETWEEN DISTANCE INCREMENTS, TIME INCREMENTS,
C     DISPERSION COEFFICIENTS, AND VELOCITIES IS NEEDED TO DETERMINE
C     THE STABILITY OF THE EXPLICIT FINITE-DIFFERENCE PROCEDURE. THIS
C     SUBROUTINE CALCULATES THE MAXIMUM ALLOWABLE INCREMENTS FOR TIME
C     AND DISTANCE. THE PROGRAM IS TERMINATED IF THE INPUT PARAMETERS
C     VIOLATE THIS CRITERIA.
C
C-----
214 C     INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
215 C     REAL KD,KDMAX,KDMIN,KDAYS,LOWER
216 C     COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
C     W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
C     DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
217 C     COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
C     1     KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
C     2     TPRINT,XPRINT,STABLX,STABLZ,STABLR1,R2,VZMAX,SET,
C     3     VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
C     4     DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
C     5     MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
C     STABLX = DELTAX
218 C     STABLZ = DELTAZ
219 C     STABLR = DELTAT
220 C

```

```

C
C-----
C          CALCULATE STABILITY CRITERIA FOR ONE DIMENSIONAL CASE
C-----
221      IF(NZ,NE,1) GO TO 500
222      IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
223      IF(STABLX.LT.DELTAX) ITER=0
224      TERM=2.*EXMAX+DELTAZ**2.*KDMAX
225      IF(TERM.NE.0.0) STABLT=(DELTAZ**2.)/TERM
226      IF(STABLT.LT.DELTAT) ITER=0
227      R1=DELTAZ/(DELTAZ**2.)
228      R2=0.
229      GO TO 800
230      500 CONTINUE
C
C-----
C          CALCULATE STABILITY CRITERIA FOR TWO DIMENSIONAL CASE
C-----
231      IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
232      IF(STABLX.LT.DELTAX) ITER=0
233      IF(EZMIN.GT.0.0.AND.VZMAX.GT.0.0) STABLZ=2.*EZMIN/VZMAX
234      IF(STABLZ.LT.DELTAZ) ITER=0
235      TERM=2.*(EXMAX*DELTAZ**2.+EZMAX*DELTAZ**2.)+KDMAX*(DELTAZ**2)*
236      $ (DELTAZ**2)
237      IF(TERM.NE.0.0) STABLT=(DELTAZ**2.)*(DELTAZ**2.)/TERM
238      IF(STABLT.LT.DELTAT) ITER=0
239      R1=DELTAZ/(DELTAZ**2.)
240      R2=DELTAZ/(DELTAZ**2.)
241      800 CONTINUE
242      RETURN
243      END

```

```

C-----
C          SUBROUTINE ONEDEX
C
C          *****
C          ***** SUBROUTINE ONEDEX *****
C          *****
C
C          THIS SUBROUTINE CALCULATES THE ONE DIMENSIONAL CONCENTRATION
C          PROFILE FOR THE ASSIGNED GRID AND THE ASSIGNED NUMBER OF ITERA-
C          TIONS.  A FINITE-DIFFERENCE,EXPLICIT SCHEME IS USED TO SOLVE THE
C          PARTIAL DIFFERENTIAL EQUATION FOR MASS TRANSPORT.
C
C-----

```

243

```

C-----
C          INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
C          REAL KD,KDMAX,KDMIN,KDAYS,LOWER
C          COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
C          W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
C          DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
C          COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
C          KOMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
C          TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
C          VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
C          DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
C          MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
C-----

```

244
245
246
247


```

248 Z=1
249 NXM1=NX-1
C
C-----
C          CALCULATE TERMS WHICH ARE USED REPEATEDLY
C-----
250 TWOXSQ=2.*DELTAZ**2.
251 TWOX=2.*DELTAZ
252 TWOZSQ=2.*DELTAZ**2.
253 TWOZ=2.*DELTAZ
C
C-----
C          CALCULATE NEW CONCENTRATION AT EACH POINT
C-----
254 DO 300 IT=1,ITER
255 TIME=TIME+DELTAT
256 DO 200 X=2,NXM1
257 TERM1A=W(X)*VX(X,Z)/TWOX
258 TERM1B=(W(X)*EX(X,Z)+W(X-1)*EX(X-1,Z))/TWOXSQ
259 TERM2B = (W(X+1)*EX(X+1,Z) + W(X)*EX(X,Z)) / TWOXSQ
          + (W(X)*EX(X,Z) + W(X-1)*EX(X-1,Z)) / TWOXSQ
          $
260 TERM3A=(-W(X)*VX(X,Z))/TWOX
261 TERM3B=(W(X+1)*EX(X+1,Z)+W(X)*EX(X,Z))/TWOXSQ
262 TERM1=C(X-1,Z,T)*((TERM1A+TERM1B)
263 TERM2=-C(X,Z,T)*((TERM2B+KD(X)*W(X))
264 TERM3=C(X+1,Z,T)*((TERM3A+TERM3B)
265 C(X,Z,TNEXT)=C(X,Z,T)+(DELTAT/W(X))*((TERM1+TERM2+TERM3)
          $ -DEMAND(X,Z)
266 IF(C(X,Z,TNEXT).LT.0.0) C(X,Z,TNEXT) = 0.0
267 200 CONTINUE

```

```

C
C-----
C      CALCULATE BOUNDARY VALUES
C-----
268      CALL RBOUND
269      Z=1
270      DO 250 X=1,NX
271      C(X,Z,T)=C(X,Z,TNEXT)
272      C(X,Z,TNEXT)=0.0
273      250 CONTINUE
274      COUNT=COUNT+1
275      IF(COUNT.GE.TPRINT) GO TO 292
276      GO TO 300

C
C-----
C      PRINT CONCENTRATIONS AT APPROPRIATE INTERVALS
C-----
277      292 CONTINUE
278      COUNT=0
279      IPAGE=IPAGE+1
280      IF(IPAGE.GE.NPPAGE) GO TO 295
281      GO TO 297
282      295 PRINT 296
283      296 FORMAT(1H1)
284      IPAGE = 0
285      297 CALL PRINT2
286      300 CONTINUE
287      RETURN
288      END

```

```

C-----
289 SUBROUTINE TWODEX
C
C *****
C ***** SUBROUTINE TWODEX *****
C *****
C
C
C THIS SUBROUTINE CALCULATES THE TWO-DIMENSIONAL CONCENTRATION
C PROFILE FOR THE ASSIGNED GRID AND THE ASSIGNED NUMBER OF ITERA-
C TIONS. A FINITE-DIFFERENCE, EXPLICIT SCHEME IS USED TO SOLVE THE
C PARTIAL DIFFERENTIAL EQUATION FOR MASS TRANSPORT.
C
C-----
290 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
291 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
292 COMMON/ARRAYS/ C(51,21,21),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
293 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
294 NXM1=NX-1
295 NZM1=NZ-1
296 NZ1=2

```

C
C
C
C

C
C
C
C

CALCULATE TERMS WHICH ARE USED REPEATEDLY

```

297 TWOXSQ=2.*DELTAZ**2.
298 TWOX=2.*DELTAZ
299 TWOZSQ=2.*DELTAZ**2.
300 TWOZ=2.*DELTAZ

```

C
C
C
C

C
C
C
C

CALCULATE NEW CONCENTRATION AT EACH POINT

```

301 DO 300 IT=1,ITER
302 TIME=TIME+DELTAZ
303 DO 200 X=2,NXMI
304 DO 100 Z=NZI,NZMI
305 TERM1A=W(X)*VX(X,Z)/TWOX
306 TERM1B=(W(X)*EX(X,Z)+W(X-1)*EX(X-1,Z))/TWOXSQ
307 TERM2B = (W(X+1)*EX(X+1,Z) + W(X)*EX(X,Z)) / TWOXSQ
          + (W(X)*EX(X,Z) + W(X-1)*EX(X-1,Z)) / TWOXSQ
          $
308 TERM3A=(-W(X)*VX(X,Z))/TWOX
309 TERM3B=(W(X+1)*EX(X+1,Z)+W(X)*EX(X,Z))/TWOXSQ
310 TERM4A=W(X)*VZ(X,Z)/TWCZ
311 TERM4B=(W(X)*EZ(X,Z)+W(X)*EZ(X,Z-1))/TWOZSQ
312 TERM5A=(-W(X)*VZ(X,Z))/TWOZ
313 TERM5B=(W(X)*EZ(X,Z+1)+W(X)*EZ(X,Z))/TWOZSQ
314 TERM6B = (W(X) * EZ(X,Z+1) + W(X)*EZ(X,Z)) / TWOZSQ
          + (W(X)*EZ(X,Z) + W(X)*EZ(X,Z-1)) / TWOZSQ
          $

```

```

315 TERM2=-C(X,Z,T)*(TERM2B+TERM6B+KD(X)*W(X))
316 TERM1=C(X-1,Z,T)*(TERM1A+TERM1B)
317 TERM3=C(X+1,Z,T)*(TERM3A+TERM3B)
318 TERM4=C(X,Z-1,T)*(TERM4A+TERM4B)
319 TERM5=C(X,Z+1,T)*(TERM5A+TERM5B)
320 C(X,Z,TNEXT)=C(X,Z,T)+(DELTA/T/W(X))*(TERM1+TERM2+TERM3+TERM4+TERM5
    $ )-DEMAND(X,Z)
321 IF(C(X,Z,TNEXT).LT.0.0) C(X,Z,TNEXT) = 0.0
322 100 CONTINUE
323 200 CONTINUE
C
C-----
C          CALCULATE BOUNDARY VALUES
C-----
324 CALL ROUND
325 DO 250 Z=1,NZ
326 DO 250 X=1,NX
327 C(X,Z,T)=C(X,Z,TNEXT)
328 C(X,Z,TNEXT)=0.0
329 250 CONTINUE

```

```
330 COUNT=COUNT+1
331 IF(COUNT.GE.TPRINT) GO TO 292
332 GO TO 300
C-----
C          PRINT CONCENTRATIONS AT APPROPRIATE INTERVALS
C-----
333 292 CONTINUE
334 COUNT=0
335 IPAGE=IPAGE+1
336 IF(IPAGE.GE.NPPAGE) GO TO 295
337 GO TO 297
338 295 PRINT 296
339 296 FORMAT(IH1)
340 IPAGE = 0
341 297 CALL PRINT2
342 300 CONTINUE
343 RETURN
344 END
```



```

354 GO TO (1001,1002,1003,1004,1005,1006,1006), OPTION
C
C-----
C      EXPONENTIAL EXTRAPOLATION
C-----
355 1003 CONTINUE
356 IF(NZ.EQ.1) GO TO 1001
357 DO 140 X=2,NXM1
358 IF(C(X,2,T+1).LE.0.0) GO TO 140
359 VZ1=VZ(X,1)
360 ZZ=-ZUP+DELTAZ+ZSF+VZ1*TSTART*86400.
361 EZ1=EZ(X,1)
362 C2=4.*EZ1*TIME
363 EXPON=(-2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)-2.*DELTAZ*VZ1*TIME)/C2
364 C(X,1,T+1)=C(X,2,T+1)*EXP(EXPON)
365 140 CONTINUE
366 DO 190 X=2,NXM1
367 IF(C(X,NZM1,T+1).LE.0.0) GO TO 190
368 VZNZ=VZ(X,NZ)
369 ZZ=(NZ-2)*DELTAZ-ZLOW+ZSF+VZNZ*TSTART*86400.
370 EZNZ=EZ(X,NZ)
371 C2=4.*EZNZ*TIME
372 EXPON=(-2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)+2.*DELTAZ*VZNZ*TIME)/C2
373 C(X,NZ,T+1)=C(X,NZM1,T+1)*EXP(EXPON)
374 190 CONTINUE
375 1001 CONTINUE
376 IF(LTYPE.EQ.1.AND.MSU.EQ.1) GO TO 350
377 DO 340 Z=1,NZ
378 IF(C(2,Z,T+1).LE.0.0) GO TO 340
379 VX1=VX(1,Z)
380 XX=-UPPER+DELTAZ+XSF+VX1*TSTART*86400.
381 EX1=EX(1,Z)

```



```

382 C2=4.*EX1*TIME
383 EXPON=( 2.*XX*DELTA-(DELTA*DELTA)-2.*DELTA*VX1*TIME)/C2
384 C(1,Z,T+1)=C(2,Z,T+1)*EXP(EXPON)
385
340 CONTINUE
350 CONTINUE
386 IF(LTYPE.EQ.1.AND.MSL.EQ.NZ) GO TO 400
387 DO 390 Z=1,NZ
388 IF(C(NX-1,Z,T+1).LE.0.0) GO TO 390
389 VXNX=VX(NX,Z)
390 XX=(NX-2)*DELTA-LOWER+XSF+VXNX*TSTART*86400.
391 EXNX=EX(NX,Z)
392 C2=4.*EXNX*TIME
393 EXPON=(-2.*XX*DELTA-(DELTA*DELTA)+2.*DELTA*VXNX*TIME)/C2
394 C(NX,Z,T+1)=C(NX-1,Z,T+1)*EXP(EXPON)
395
390 CONTINUE
400 CONTINUE
GO TO 2000

```

```

C
C
C CONSTANT SLOPE EXTRAPOLATION
C
1004 CONTINUE
399 IF(NZ.EQ.1) GO TO 1002
400 DO 540 X=2,NXM1
401 C(X,1,T+1)=2.*C(X,2,T+1)-C(X,3,T+1)
402 IF(C(X,1,T+1).LT.0.0) C(X,1,T+1)=0.0
403 C(X,NZ,T+1)=2.*C(X,NZM1,T+1)-C(X,NZM2,T+1)
404 IF(C(X,NZ,T+1).LT.0.0) C(X,NZ,T+1)=0.0
405
540 CONTINUE

```

```

407 1002 CONTINUE
408   DO 590 Z=1,NZ
409     C(1,Z,T+1)=2.*C(2,Z,T+1)-C(3,Z,T+1)
410     IF(C(1,Z,T+1).LT.0.0) C(1,Z,T+1)=0.0
411     C(NX,Z,T+1)=2.*C(NXM1,Z,T+1)-C(NXM2,Z,T+1)
412     IF(C(NX,Z,T+1).LT.0.0) C(NX,Z,T+1)=0.0
413   590 CONTINUE
414     GO TO 2000
415 1005 CONTINUE
C
C-----
C   THIS OPTION ALLOWS THE PROGRAM USER TO SUBSTITUTE HIS OWN
C   EXTRAPOLATION ROUTINE
C-----
416   ZZ=-40.
417   EZ1=0.025
418   VZ1=0.
419   C2=4.*EZ1*TIME
420   EXPON=(+2.*ZZ*DELTAZ-(DELTAZ**2.))-2*DELTAZ*VZ1*TIME)/C2
421   DO 240 X=1,NX
422     C(X,1,T+1)=C(X,2,T+1)*EXP(EXPON)
423     C(X,NZ,T+1)=C(X,1,T+1)
424   240 CONTINUE
425     GO TO 1001
C
C-----
C   INVERTED DIFFERENCES EXTRAPOLATION (ACFI)
C-----
426 1006 CALL EXTRAP
427     GO TO 2000
428 2000 CONTINUE

```

```

429 IF(NZ.EQ.1) GO TO 2005
430 IF(OPTION.EQ.3.OR.OPTION.EQ.4.OR.OPTION.EQ.5) GO TO 2005
431 DO 700 X=1,NX
432 C(X,1,T+1)=C(X,2,T+1)
433 C(X,NZ,T+1)=C(X,NZM1,T+1)
434 700 CONTINUE
435 2005 CONTINUE
C
C-----
C IF LTYPE EQUALS 1, SET CONCENTRATIONS AT SOURCE POINTS.
C IF LTYPE EQUALS 3, THE USER CAN INCLUDE A SPECIAL SUBROUTINE
C OR SET OF CALCULATIONS IN THIS PART OF THE PROGRAM.
C-----
436 IF(LTYPE.EQ.1.AND.NS.LE.2) GO TO 275
437 IF(LTYPE.EQ.1.AND.NS.GT.2) GO TO 281
438 GO TO 290
439 275 CONTINUE
440 DO 280 Z=1,NZ
441 C(MSU,Z,T+1)=C(MSU,Z,T)
442 C(MSL,Z,T+1)=C(MSL,Z,T)
443 280 CONTINUE
444 GO TO 290
445 281 CONTINUE
446 DO 285 I=1,NC
447 DO 285 Z=1,NZ
448 C(INSECT(I),Z,T+1)=CONCIN(I,Z)
449 285 CONTINUE
450 290 CONTINUE
451 RETURN
452 END

```

```

C-----
C          SUBROUTINE EXTRAP
C
C          *****
C          ***** SUBROUTINE EXTRAP *****
C          *****
C
C          THIS SUBROUTINE EXTRAPOLATES THE CONCENTRATION PROFILE BY
C          USING A CONTINUED FRACTIONS AND INVERTED DIFFERENCES SCHEME. IT IS
C          A MODIFIED VERSION OF THE IBM SCIENTIFIC SUBPROGRAM ACFI.
C
C          IF THE EXTRAPOLATED CONCENTRATION IS GREATER THAN THE
C          ADJACENT CONCENTRATION OR IF IT IS NEGATIVE, THEN THE BOUNDARY
C          CONCENTRATION IS EXTRAPOLATED ACCORDING TO THE PROPORTION
C          BETWEEN THE CONCENTRATIONS AT THE TWO ADJACENT INTERNAL POINTS
C          (I.E.  $C1/C2 = C2/C3$  ).
C
C-----
C          INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
C          REAL KD,KDMAX,KDMIN,KDAYS,LOWER
C          COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
C          W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
C          DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
C          COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
C          KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
C          TPRINT,XPRINT,STABLX,STABLZ,STABL,R1,R2,VZMAX,SET,
C          VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
C          DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
C          MSU,MSL,TIME,NXMI,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
C          DIMENSION ARG(10), VALY(10)
C-----
453          SUBROUTINE EXTRAP
454          INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
455          REAL KD,KDMAX,KDMIN,KDAYS,LOWER
456          COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
457          W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
458          DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)

```

```

459 NZ1=2
460 IF(NZ.EQ.1) NZ1=1
461 IF(NZ.EQ.1)NZMI=1
462 IF(MSU.EQ.1.AND.LTYPE.EQ.1) GO TO 228
C
C-----
C      EXTRAPOLATE THE PROFILE IN THE X DIRECTION
C-----
463 DO 225 Z=NZ1,NZMI
464   IF(C(2, Z, T+1) . LE. C.00) GO TO 220
C
C-----
C      CHOOSE THE NUMBER OF POINTS (NDIM) TO BE USED IN THE
C      EXTRAPOLATION.
C-----
465 NDIM=3
466 NSTOP=NDIM+1
C
C-----
C      PLACE CONCENTRATIONS IN PROPER ORDER FOR SUBROUTINE ACFI
C-----
467 DO 210 X=2,NSTOP
468   ARG(X - 1) = FLOAT(X)
469   VALY(X-1) = C(X, Z, T+1)
470   IF(VALY(X-1).LE.0.0) GO TO 205
471   GO TO 210
472 205 NDIM = X-1
473   GO TO 211
474 210 CONTINUE

```

```

475 211 CONTINUE
476 EPS = VALY(I)/1000.
477 CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
478 C(1, Z, T+1)=Y
C
C-----
C IF THE EXTRAPOLATED VALUE IS TOO HIGH OR IS NEGATIVE, THEN USE
C A PROPORTION EQUATION.
C-----
479 IF(Y.LE.0.0.OR.Y.GT.C(2,Z,T+1)) C(1,Z,T+1)=C(2,Z,T+1)
480 $ /C(3,Z,T+1)
481 GO TO 225
482 C(1, Z, T+1) = 0.0
483 CONTINUE
484 CONTINUE
485 IF(LTYPE.EQ.1.AND.MSL.EQ.NX) GO TO 251
486 DO 250 Z=NZ1,NZM1
487 IF ( C(NX-1, Z, T+1) . LE. 0.00) GO TO 245
488 NDIM=3
489 NSTOP=NDIM
490 DO 235 I=1,NSTOP
491 IBACK = NX - I
492 ARG(I) = FLOAT(I+1)
493 VALY(I) = C( IBACK, Z, T+1)
494 IF( VALY(I) . LE. 0.00) GO TO 230
495 GO TO 235
496 230 NDIM = I
497 GO TO 236
498 235 CONTINUE
499 236 CONTINUE
EPS = VALY(I)/1000.

```

```

500 CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
501 C( NX, Z, T+1) = Y
502 IF( Y.LE.0.0.OR.Y.GT.C(NX-1,Z,T+1))C(NX,Z,T+1)=C(NXMI,Z,T+1)*
$ C(NXMI,Z,T+1)/C(NXM2,Z,T+1)
503 GO TO 250
504 C(NX, Z, T+1) = 0.0
505 250 CONTINUE
506 251 CONTINUE
507 IF(NZ.EQ.1.OR.OPTION.EQ.6) RETURN
C
C-----
C      EXTRAPOLATE THE PROFILE IN THE Z DIRECTION
C-----
508 DO 275 X = 1, NX
509 IF(C(X, 2, T+1) . LE. 0.00) GO TO 270
510 NDIM=3
511 NSTOP=NDIM
512 DO 260 Z = 1, NSTOP
513 ARG(Z) = FLOAT(Z+1)
514 VALY(Z) = C(X, Z+1, T+1)
515 IF( VALY(Z) . LE. 0.00) GO TO 255
516 GO TO 260
517 255 NDIM = Z
518 GO TO 261
519 260 CONTINUE
520 261 CONTINUE
521 EPS = VALY(1)/1000.
522 CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
523 C ( X, 1, T+1) = Y

```

```

524 IF(Y.LE.0.0.OR.Y.GT.C(X,2,T+1))C(X,1,T+1)=C(X,2,T+1)*C(X,2,T+1)
$ /C(X,3,T+1)
525 IF(C(X,1,T+1).LT.0.0) C(X,1,T+1)=0.0
526 GO TO 275
527 270 C(X, 1, T+1) = 0.00
528 275 CONTINUE
529 DO 295 X = 1, NX
530 IF (C(X, NZ-1, T+1) . LE. 0.00) GO TO 292
531 NDIM=3
532 NSTOP=NDIM
533 DO 285 I = 1, NSTOP
534 ARG(I) = FLOAT(I+1)
535 IBACK = NZ - I
536 VALY(I) = C(X, IBACK, T+1)
537 IF(VALY(I). LE. 0.00) GO TO 280
538 GO TO 285
539 280 NDIM = I
540 GO TO 286
541 285 CONTINUE
542 286 CONTINUE
543 EPS = VALY(1)/ 1000.
544 CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
545 C(X, NZ, T+1) = Y
546 IF(Y.LE.0.0.OR.Y.GT.C(X,NZ-1,T+1)) C(X,NZ,T+1)=C(X,NZM1,T+1)*
$ C(X,NZM1,T+1)/C(X,NZM2,T+1)
GO TO 295
547 292 C(X, NZ, T+1)=0.0
548 295 CONTINUE
549 RETURN
550 END
551

```


552

```

C-----
C SUBROUTINE ACFI ( X, ARG, VALY, Y, NDIM, EPS, IER, J )
C
C *****
C ***** SUBROUTINE ACFI *****
C *****
C .....
C .....
C
C PURPOSE
C TO INTERPOLATE FUNCTION VALUE Y FOR A GIVEN ARGUMENT VALUE
C X USING A GIVEN TABLE (ARG,VAL) OF ARGUMENT AND FUNCTION
C VALUES.
C
C DESCRIPTION OF PARAMETERS
C X - THE ARGUMENT VALUE SPECIFIED BY INPUT.
C ARG - THE INPUT VECTOR (DIMENSION NDIM) OF ARGUMENT
C VALUES OF THE TABLE (POSSIBLY DESTROYED).
C VAL - THE INPUT VECTOR (DIMENSION NDIM) OF FUNCTION
C VALUES OF THE TABLE (DESTROYED).
C Y - THE RESULTING INTERPOLATED FUNCTION VALUE.
C NDIM - AN INPUT VALUE WHICH SPECIFIES THE NUMBER OF
C POINTS IN TABLE (ARG,VAL).
C EPS - AN INPUT CONSTANT WHICH IS USED AS UPPER BOUND
C FOR THE ABSOLUTE ERROR.
C IER - A RESULTING ERROR PARAMETER.
C

```

C REMARKS
C (1) TABLE (ARG,VAL) SHOULD REPRESENT A SINGLE-VALUED
C FUNCTION AND SHOULD BE STORED IN SUCH A WAY, THAT THE
C DISTANCES ABS(ARG(I)-X) INCREASE WITH INCREASING
C SUBSCRIPT I. TO GENERATE THIS ORDER IN TABLE (ARG,VAL),
C SUBROUTINES ATSG, ATSM OR ATSE COULD BE USED IN A
C PREVIOUS STAGE.
C (2) NO ACTION BESIDES ERROR MESSAGE IN CASE NDIM LESS
C THAN 1.
C (3) INTERPOLATION IS TERMINATED EITHER IF THE DIFFERENCE
C BETWEEN TWO SUCCESSIVE INTERPOLATED VALUES IS
C ABSOLUTELY LESS THAN TOLERANCE EPS, OR IF THE ABSOLUTE
C VALUE OF THIS DIFFERENCE STOPS DIMINISHING, OR AFTER
C (NDIM-1) STEPS (THE NUMBER OF POSSIBLE STEPS IS
C DIMINISHED IF AT ANY STAGE INFINITY ELEMENT APPEARS IN
C THE DOWNWARD DIAGONAL OF INVERTED-DIFFERENCES-SCHEME
C AND IF IT IS IMPOSSIBLE TO ELIMINATE THIS INFINITY
C ELEMENT BY INTERCHANGING OF TABLE POINTS).
C FURTHER IT IS TERMINATED IF THE PROCEDURE DISCOVERS TWO
C ARGUMENT VALUES IN VECTOR ARG WHICH ARE IDENTICAL.
C DEPENDENT ON THESE FOUR CASES, ERROR PARAMETER IER IS
C CODED IN THE FOLLOWING FORM
C IER=0 - IT WAS POSSIBLE TO REACH THE REQUIRED
C ACCURACY (NO ERROR).
C IER=1 - IT WAS IMPOSSIBLE TO REACH THE REQUIRED
C ACCURACY BECAUSE OF ROUNDING ERRORS.
C IER=2 - IT WAS IMPOSSIBLE TO CHECK ACCURACY BECAUSE
C NDIM IS LESS THAN 2, OR THE REQUIRED ACCURACY
C COULD NOT BE REACHED BY MEANS OF THE GIVEN
C TABLE. NDIM SHOULD BE INCREASED.
C IER=3 - THE PROCEDURE DISCOVERED TWO ARGUMENT VALUES
C IN VECTOR ARG WHICH ARE IDENTICAL.
C


```

566 C START INTERPOLATION LOOP
567 DO 16 I=2,NDIM
568 II=0
569 P1=P2
570 P2=P3
571 Q1=Q2
572 Q2=Q3
573 Z=Y
574 DELT1=DELT2
575 JEND=I-1
576
577 C COMPUTATION OF INVERTED DIFFERENCES
578 DO 10 J=1,JEND
579 AUX=VAL(I)
580 H=VAL(I)-VAL(J)
581 IF (ABS(H)-1.E-6*ABS(VAL(I)))4,4,9
582 IF (ARG(I)-ARG(J))5,17,5
583 IF (J-JEND)8,6,6
584
585 C INTERCHANGE ROW I WITH ROW I+II
586 II=II+1
587 III=I+II
588 IF (III-NDIM)7,7,19
589 VAL(I)=VAL(III)
590 VAL(III)=AUX
591 AUX=ARG(I)
592 ARG(I)=ARG(III)
593 ARG(III)=AUX
594 GOTO 3
595
596 C

```

```

C      COMPUTATION OF VAL(I) IN CASE VAL(I)=VAL(J) AND J LESS THAN I-1
590 VAL(I)=1.E75
591 GOTO 10
C
C      COMPUTATION OF VAL(I) IN CASE VAL(I) NOT EQUAL TO VAL(J)
592 VAL(I)=(ARG(I)-ARG(J))/H
593 10 CONTINUE
C      INVERTED DIFFERENCES ARE COMPUTED
C
C      COMPUTATION OF NEW Y
594 P3=VAL(I)*P2+(X-ARG(I-1))*P1
595 Q3=VAL(I)*Q2+(X-ARG(I-1))*Q1
596 IF(Q3)11,12,11
597 11 Y=P3/Q3
598 GOTO 13
599 12 Y=1.E75
600 13 DELT2=ABS(Z-Y)
601 IF(DELT2-EPS)19,19,14
602 14 IF(I-8)16,15,15
603 15 IF(DELT2-DELT1)16,18,18
604 16 CONTINUE
C      END OF INTERPOLATION LOOP
C
C      RETURN
605 RETURN
C
C      THERE ARE TWO IDENTICAL ARGUMENT VALUES IN VECTOR ARG
606 17 IER=3
607 RETURN
C

```

```
608 C TEST VALUE DELT2 STARTS OSCILLATING
609   18 Y=Z
610   IER=1
      RETURN
611 C
612   C TMERE IS SATISFACTORY ACCURACY WITHIN NDIM-1 STEPS
613   19 IER=0
      20 RETURN
      END
```

```

C-----
614 SUBROUTINE PRINT1
C
C *****
C ***** SUBROUTINE PRINT1 *****
C *****
C
C THIS SUBROUTINE CALCULATES THE CONVERSION VALUES FOR MANY OF
C THE INPUT PARAMETERS AND PRINTS OUT A SUMMARY OF THE INPUT DATA
C AND STABILITY CRITERIA.
C-----
615 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CCUNT,OPTION
616 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
617 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
618 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DEL T1,DEL T2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAZ,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
619 UNITS1 = 86400./5280.
620 UNITS2 = 86400./15280.*5280.1
621 DELDAY = DELTAT/86400.
622 SECS1=DEL T1*86400.
623 SECS2=DEL T2*86400.
624 DELMX=DELTAZ/5280.
625 DELMZ=DELTAZ/5280.

```

```
626 TOTALX=(NX-1)*DELTAX
627 TOTMX=TOTALX/5280.
628 TOTALZ=(NZ-1)*DELTAZ
629 TOTMZ=TOTALZ/5280.
630 VX1=VXMAX*UNITSI
631 VX2=VXMIN*UNITSI
632 VZ1=VZMAX*UNITSI
633 IF(VZMIN.EQ.1000000.) VZMIN=0.
634 VZ2=VZMIN*UNITSI
635 EX1=EXMAX*UNITSI
636 EX2=EXMIN*UNITSI
637 EZ1=EZMAX*UNITSI
638 IF(EZMIN.EQ.1000000.) EZMIN=0.
639 EZ2=EZMIN*UNITSI
640 XKD1=KDMAX*86400.
641 XKD2=KDMIN*86400.
642 SXM=STABLX/5280.
643 SZM=STABLZ/5280.
644 STD=STABLT/86400.
645 PRINT 500
646 PRINT 505
647 PRINT 507
648 PRINT 510
649 PRINT 507
650 IF(NZ.EQ.1) PRINT 515
651 IF(NZ.NE.1) PRINT 520
652 PRINT 507
653 IF(LTYPE.EQ.1) GO TO 7
654 IF(LTYPE.EQ.2) GO TO 5
```



```

655 PRINT 530
656 GO TO 10
657 5 PRINT 525
658 GO TO 10
659 7 PRINT 523
660 10 CONTINUE
661 PRINT 507
662 PRINT 535
663 PRINT 507
664 PRINT 505
665 PRINT 540, DELTAX, DELMX
666 PRINT 545, DELTAZ, DELMZ
667 PRINT 550, SECS1, DELT1
668 PRINT 551, ITER1
669 PRINT 552, SECS2, DELT2
670 PRINT 553, ITER2
671 PRINT 555, NX
672 PRINT 560, NZ
673 PRINT 565, VXMAX, VX1
674 PRINT 570, VXMIN, VX2
675 PRINT 572, VZMAX, VZ1
676 PRINT 573, VZMIN, VZ2
677 PRINT 575, EXMAX, EX1
678 PRINT 580, EXMIN, EX2
679 PRINT 585, EZMAX, EZ1
680 PRINT 590, EZMIN, EZ2
681 PRINT 600, WMAX
682 PRINT 605, WMIN
683 PRINT 610, XKD1, KDMAX
684 PRINT 615, XKD2, KDMIN
685 PRINT 620, DEMAX
686 PRINT 630, NC

```

```

C-----
C      XUPEAK AND XLPEAK ARE CALCULATED ASSUMING DISTANCES ARE
C      DECREASING IN THE DOWNSTREAM DIRECTION
C-----
687      XUPEAK = XSTART - (MSU-1)*DELMX
688      XLPEAK = XSTART - (MSL-1)*DELMX
689      PRINT 635, XUPEAK,MSU
690      PRINT 637, XLPEAK,MSL
691      PRINT 500
692      PRINT 505
693      PRINT 502
694      PRINT 640
695      PRINT 645, STABLX,SXM
696      PRINT 650, STABLZ,SZM
697      PRINT 655, STABLT,STD
698      PRINT 660,R1
699      PRINT 665, R2
700      PRINT 505
701      IF(NC.EQ.0) GO TO 100
702      PRINT 500
703      PRINT 700
704      IF(NZ.EQ.1) GO TO 50
705      DO 25 I=1,NC
C-----
C      WHERE IS CALCULATED ASSUMING DISTANCES ARE DECREASING IN
C      THE DOWNSTREAM DIRECTION
C-----

```

```

706 WHERE=XSTART-(INSECT(I)-1)*DELMX
707 PRINT 710, INSECT(I),WHERE,DELTAZ
708 DO 25 Z=1,NZ
709 PRINT 715,INSECT(I),Z,CONCIN(I,Z)
710 25 CONTINUE
711 GO TO 100
712
713 50 DO 75 I=1,NC
714 WHERE=XSTART-(INSECT(I)-1)*DELMX
715 PRINT 705, INSECT(I), WHERE, CCNCIN(I,I)
716 PRINT 502
717 75 CONTINUE
718 100 CONTINUE
719 PRINT 500
720 FORMAT(IH1)
721 500 FORMAT(/)
722 505 FORMAT(T45, 38('**'))
723 507 FORMAT(T45,'**',T82,'**')
724 510 FORMAT(T45,'**',T56,'ESTUARY SIMULATION', T82, '**')
725 515 FORMAT(T45, '**', T48, 'ONE DIMENSIONAL EXPLICIT METHOD',T82,'**')
726 520 FORMAT(T45, '**', T48, 'TWO DIMENSIONAL EXPLICIT METHOD',T82,'**')
727 523 FORMAT(T45, '**', T53, 'CONSTANT CONCENTRATION',T82,'**')
728 525 FORMAT(T45, '**', T54, 'INSTANTANEOUS RELEASE', T82,'**')
729 530 FORMAT(T45, '**', T56, 'CONTINUOUS LOADING', T82, '**')
730 535 FORMAT(T45, '**', T51, 'PROGRAMMER -- JONATHAN YOUNG', T82,'**')
731 540 FORMAT( /, T53, 'X INCREMENT =', F7.0, ' FEET (', F8.5, ' MILES
732 $),', /)
733 545 FORMAT(T53, 'Z INCREMENT =', F7.0, ' FEET (', E10.3, ' MILES )' /)
734 550 FORMAT(T42, 'INITIAL TIME INCREMENT =',F7.0, ' SECONDS (', F6.4,
735 $ , DAYS )' /)
736 551 FORMAT(T44,'NUMBER OF ITERATIONS = ',I5,/)

```

```

734 552 FORMAT( T42, 'REVISED TIME INCREMENT = ', F7.0, ' SECONDS(', F6.4,
    $ ' DAYS )', /)
735 553 FORMAT(T44, 'NUMBER OF ITERATIONS = ', I5, /)
736 555 FORMAT(T37, 'NUMBER OF HORIZONTAL POINTS = ', I4, /)
737 560 FORMAT(T39, 'NUMBER OF VERTICAL POINTS = ', I4, /)
738 565 FORMAT(T37, 'MAXIMUM HORIZONTAL VELOCITY = ', F5.2, ' FEET/SECOND
    $(', F6.2, ' MILES/DAY )', /)
739 570 FORMAT(T37, 'MINIMUM HORIZONTAL VELOCITY = ', F5.2, ' FEET/SECOND
    $(', F6.2, ' MILES/DAY )', /)
740 572 FORMAT(T39, 'MAXIMUM VERTICAL VELOCITY = ', E10.3, ' FEET/SECOND (',
    $ ' E10.3, ' MILES/DAY )', /)
741 573 FORMAT(T39, 'MINIMUM VERTICAL VELOCITY = ', E10.3, ' FEET/SECOND (',
    $ ' E10.3, ' MILES/DAY )', /)
742 575 FORMAT(T35, 'MAXIMUM HORIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
    $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )', /)
743 580 FORMAT(T35, 'MINIMUM HORIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
    $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )', /)
744 585 FORMAT(T37, 'MAXIMUM VERTICAL DISPERSION = ', F9.3, ' FEET SQUARED
    $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )', /)
745 590 FORMAT(T37, 'MINIMUM VERTICAL DISPERSION = ', F9.3, ' FEET SQUARED
    $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )', /)
746 600 FORMAT(T51, 'MAXIMUM WIDTH = ', F5.0, ' FEET', /)
747 605 FORMAT(T51, 'MINIMUM WIDTH = ', F5.0, ' FEET', /)
748 610 FORMAT(T46, 'MAXIMUM DECAY RATE = ', F5.3, ' PER DAY ( ', E10.3,
    $ ' PER SECOND )', /)
749 615 FORMAT(T46, 'MINIMUM DECAY RATE = ', F5.3, ' PER DAY ( ', E10.3,
    $ ' PER SECOND )', /)
750 620 FORMAT(T42, 'OTHER DEMANDS, MAXIMUM = ', F5.2, /)
751 630 FORMAT(T26, 'NUMBER OF INITIAL CONCENTRATION VALUES = ', I2, /)

```

```

752 635 FORMAT(T16, 'INITIAL LOCATION OF UPPERMOST PEAK CONCENTRATION = ',
    $ F6.2, ' MILES (SECTION NUMBER ', I2,')',/)
753 637 FORMAT(T16, 'INITIAL LOCATION OF LOWERMOST PEAK CONCENTRATION = ',
    $ F6.2, ' MILES (SECTION NUMBER ', I2,')',)
754 640 FORMAT(T56, 'STABILITY CRITERIA',/)
755 645 FORMAT(T35, 'MAXIMUM ALLOWABLE X INCREMENT = ', F7.0, ' FEET (' ,
    $ F8.5, ' MILES )',/)
756 650 FORMAT(T35, 'MAXIMUM ALLOWABLE Z INCREMENT = ', F7.0, ' FEET (' ,
    $ F8.5, ' MILES )',/)
757 655 FORMAT(T32, 'MAXIMUM ALLOWABLE TIME INCREMENT = ', F7.0, ' SECONDS
    $ ( , E10.3, ' DAYS )',/)
758 660 FORMAT(T38, 'ACTUAL DELTAT/(DELTAX**2.) = ', E10.3,/)
759 665 FORMAT(T38, 'ACTUAL DELTAT/(DELTAZ**2.) = ', E10.3,/)
760 700 FORMAT(T25, 'LOCATIONS OF INITIAL CONCENTRATIONS',/)
761 705 FORMAT( 'A WASTE SOURCE IS LOCATED AT STATION ', I2, ' (MILE ',
    $ F6.3, ' )'. THE CONCENTRATION IS', F10.2, ' PPM.' )
762 710 FORMAT( 1X, 'AN INITIAL CONCENTRATION IS FOUND AT STATION ', I2,
    1 ' (MILE ', F6.3, ' )'.', ' THE CONCENTRATIONS AT ', F7.1,
    2 ' FOOT INTERVALS WITH DEPTH ARE' )
763 715 FORMAT(T15, 'C( ', I2, ', I2', 1) = ', F9.2, ' PPM' )
764 RETURN
765 END

```

```

766 C-----
      SUBROUTINE PRINT2
C
C *****
C ***** SUBROUTINE PRINT2 *****
C *****
C
C THIS SUBROUTINE PRINTS OUT THE TIME AND THE CONCENTRATION
C PROFILE ACCORDING TO A FORMAT PREVIOUSLY READ INTO THE PROGRAM.
C THIS SUBROUTINE CAN BE CHANGED ACCORDING TO THE NEEDS OF THE USER.
C **WARNING--DO NOT CHANGE ANY VARIABLE OCCURRING IN A COMMON BLOCK.
C
C
767 C-----
      INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
768 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
769 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
770 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      MSU,MSL,TIME,NXMI,NXM2,NZMI,NZM2,ZSF,LTYPE,OPTION
      DIMENSION NZREV(20)
      DIMENSION R(51,21)
      DAYS = TIME/86400.
      HOURS=DAYS*24.
      WRITE(6,FMT1) HOURS,DAYS

```

```

776      DO 100 IZ = 1, NZ
777      NZREV(IZ)=IFIX(DELTAZ*(IZ-2))
778      100 CONTINUE
779      WRITE(6,FMT2) (NZREV(I),I=2,NZMI)
C-----
C      ALLOW FOR A BACKGROUND CONCENTRATION OF 10.0
C-----
780      DO 150 X=1,NX
781      DO 150 Z=1,NZ
782      R(X,Z)=C(X,Z,T)+10.0
783      150 CONTINUE
784      XFIRST=XSTART-(NX-1)*DELTAZ/5280.
785      DO 200 X=1,NX,XPRINT
786      M=NX+1-X.
787      XMILE = XFIRST + ((X-1)*DELTAZ*XPRINT)/5280.
788      WRITE(6,FMT3) XMILE, (R(M,IZ), IZ=1,NZ)
789      200 CONTINUE
790      RETURN
791      END

```

```

792 C-----SUBROUTINE ERROR(*)
C
C *****
C ***** SUBROUTINE ERROR *****
C *****
C
C
C THIS SUBROUTINE CORRECTS SOME OF THE MOST COMMON ERRORS
C IN THE INPUT DATA AND PRINTS OUT APPROPRIATE ERROR MESSAGES.
C-----
793 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
794 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
795 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
796 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABTZ,STABLR1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
797 IF(OPTION.LT.8) GO TO 200
798 PRINT 181
799 181 FORMAT(////, ' *****PROGRAM TERMINATED BECAUSE OPTION IS GREATER TH
$AN 7****,/,IHI)
800 RETURN
801 200 IF(LTYPE.LT.3) GO TO 300
802 PRINT 201

```



```

803 201 FORMAT(////, ' ***PROGRAM TERMINATED BECAUSE LTYPE IS GREATER TH
      $AN OR EQUAL TO 3.,/, ' IF LTYPE EQUALS 3, THE USER MUST SUPPLY A
      $ SUBROUTINE FOR THE LOADING CONDITIONS****',/, IHL)
804     RETURN I
805     300 CONTINUE
806     IF(ITER.NE.0) GO TO 82C
C-----
C     TERMINATE PROGRAM WITH PRINTED MESSAGE IF INPUT DATA
C     VIOLATES STABILITY CRITERIA
C-----
807     PRINT 815
808     815 FORMAT(////, IX, 10(' '), 'PROGRAM TERMINATED BECAUSE STABILITY CCNDI
      $TIONS WERE VIOLATED****', /, IHL)
809     RETURN I
810     820 CONTINUE
811     RETURN
812     END

```

Input Data for MASSTRANS-I and MASSTRANS-II

Each line represents a new card unless single spaced.

1.

33, 11

0.25, 5.0

0.006944444, 2

0.0, 0

21, 1

17., 0.166667, 1, 1, 1

2, 2

33*0.0, 33*0.366667, 33*0.420, 33*0.440, 33*0.440, 33*0.433333,

33*0.389333, 33*0.273333, 33*0.233333, 33*0.233333, 33*0.0

363*450.

497., 510., 524., 537., 550., 562., 575., 562., 550., 537.,
525., 568., 611., 654., 697

756., 814., 872., 931., 863., 796., 728., 661., 657., 654.,
650., 647., 633., 619., 605.,

591, 586., 582.

33*2.50

363*0.0

1

3.2, 3.2, 2.0, 1.7, 2.0, 3.0, 3.0, 1.8, 0.2, 0.0, 0.0

2

4.0, 4.0, 3.5, 2.0, 3.0, 4.0, 4.0, 2.1, 0.3, 0.0, 0.0

3

6.2, 6.2, 5.0, 4.0, 4.0, 5.0, 5.0, 2.3, 0.4, 0.0, 0.0

4

9.5, 9.5, 7.0, 6.0, 6.0, 7.0, 6.0, 2.5, 0.5, 0.0, 0.0

5

90.0, 90.0, 9.0, 9.0, 9.0, 10.0, 7.0, 2.7, 0.6, 0.0, 0.0

6

115.0, 115.0, 70.0, 40.0, 30.0, 25.0, 9.0, 3.0, 0.8, 0.0, 0.0

7

165., 165., 210., 290., 120., 55., 12., 3.5, 1.0, 0.0, 0.0

8

290., 290., 490., 440., 165., 70., 20., 4., 0.8, 0.0, 0.0

9

740., 740., 890., 320., 140., 55., 14., 3.3, 0.6, 0.0, 0.0

10

1990., 1990., 540., 215., 90., 30., 10., 2.6, 0.4, 0.0, 0.0

11

990., 990., 290., 120., 50., 15., 7., 2., 0.3, 0.0, 0.0

12

290., 290., 190., 75., 20., 8., 4., 1.6, 0.2, 0.0, 0.0

13

165., 165., 75., 25., 9., 5., 2., 1.2, 0.1, 0.0, 0.0

14

90., 90., 10., 9., 5., 2., 1.5, 0.8, 0.0, 0.0, 0.0

```

15
20., 20., 8., 7., 2., 1.7, 1.0, 0.4, 0.0, 0.0, 0.0
16
9., 9., 7., 5., 1.8, 1.5, 0.7, 4*0.0
17
7., 7., 6., 4., 1.6, 1.2., 0.4, 4*0.0
18
5., 5., 5., 3., 1.4, 1., 0.2, 4*0.0
19
4., 4., 4., 2., 1.3, 0.7, 5*0.0
20
3., 3., 3., 1.8, 1.2, 0.5, 5*0.0
21
2.5, 2.5, 2.0, 1.6, 1.1, 0.2, 5*0.0
363*0.0
33*0.00333333, 33*0.00333333, 33*0.00382, 33*0.004, 33*0.004,
33*0.00394,
33*0.00354, 33*0.002485, 33*0.00212, 33*0.00212, 33*0.00212
(///,12X,100('*'), /, 12X, '*',/, 12X, '*',2X,
'CONCENTRATIONS AT TIME = ',
F7.2, ' HOURS ( ', F6.4, ' DAYS)', / ,12X, '*',/, 12X,
100('*') )
12X, '*', /, 12X, '*', 42X, 'DEPTH', /, 12X, '*', /, 12X,
'*,3X, 'IMAGE', 4X,
9(12, ' FEET '), 'IMAGE',/, 1X, 111('*'),/, 12X, '*' )
(1X, 'MILE', F5.2, 1X, '*', 2X, 11(F6.1, 4X))
// END OF DATA FOR MASSTRANS-I //

```

33*0.0, 33*0.366667, 33*0.420, 33*0.440, 33*0.440, 33*0.433333,
33*0.389333, 33*0.273333, 33*0.233333, 33*0.233333, 33*0.0
363*450.
363*0.0
33*0.00333333, 33*0.00333333, 33*0.00382, 33*0.004, 33*0.004,
33*0.00394,
33*0.00354, 33*0.002485, 33*0.00212, 33*0.00212, 33*0.00212

// THIS PAGE OF DATA USED FOR MASSTRANS-II ONLY //

COMPUTER PROGRAM FOR
MASSTRANS-II

Object Code = 67,944 bytes

Array Area = 62,924 bytes

Total = 130,868 bytes

C A USER OF THIS PROGRAM MUST BE FAMILIAR WITH THE LIMITATIONS
C ON ACCURACY AND STABILITY INHERENT IN THE TYPE OF NUMERICAL PROCEDURE
C USED IN THESE CALCULATIONS. A SUBROUTINE WITHIN THE PROGRAM
C PRINTS OUT THE PROPER INCREMENTS WHICH WILL INSURE STABILITY.
C ANOTHER SUBROUTINE EXTRAPOLATES THE CONCENTRATIONS AT THE
C BOUNDARIES--SEVERAL METHODS CAN BE USED FOR THESE EXTRAPOLATIONS
C DEPENDING ON THE TYPE OF PROFILE BEING ANALYZED AND THE CHOICE OF
C THE USER. A SUBROUTINE IS ALSO INCLUDED WHICH PRINTS OUT ERROR
C MESSAGES AND TERMINATES THE PROGRAM IF CERTAIN INCONSISTENCIES
C OCCUR IN THE INPUT DATA.
C

C THIS PROGRAM WAS DEVELOPED PRIMARILY TO ANALYZE PARTIALLY
C STRATIFIED ESTUARIES WHICH HAVE BEEN DREDGED OUT TO A FAIRLY
C CONSTANT DEPTH AT THE CENTERLINE OF THE CHANNEL--THESE ESTUARIES
C ARE COMMON IN THE GULF COAST REGION. APPLICATION OF THIS PROGRAM
C TO PARTIALLY STRATIFIED ESTUARIES WITH VARIABLE DEPTHS WOULD
C REQUIRE MODERATE REVISIONS TO THE PROGRAM AND WOULD MAKE THE
C PROGRAM ESTUARY-DEPENDENT.
C

C THIS COMPUTER PROGRAM CAN ALSO BE APPLIED TO ESTUARIES WHICH
C ARE WELL-MIXED IN THE VERTICAL DIRECTION. THIS OPTION ALLOWS FOR
C VARYING WIDTH OR VARYING DEPTH AND USES MOST OF THE ROUTINES
C AVAILABLE TO THE TWO-DIMENSIONAL ANALYSIS.
C

C QUESTIONS REGARDING THIS PROGRAM MAY BE REFERRED TO
C JONATHAN YOUNG AT HYDROSCIENCE, INC, 363 OLD HOOK ROAD,
C WESTWOOD, NEW JERSEY 07675 PHONE 201 / 666-2600
C

C

*****NOTE--VALUES FOR VZ AND EZ ARE TO BE INCLUDED
 WITH THE DATA ONLY IF NZ IS GREATER THAN 1
 VZ(X,Z) = VERTICAL VELOCITIES (FT/SEC)
 EZ(X,Z) = VERTICAL DISPERSION (FT**2/SEC)
 VXNEXT(X,Z) = HORIZONTAL VELOCITY,NEXT TIME STEP (FT/SEC)
 EXNEXT(X,Z) = HORIZONTAL DISPERSION,NEXT TIME STEP (FT**2/SEC)
 VZNEXT(X,Z) = VERTICAL VELOCITY,NEXT TIME STEP (FT/SEC)
 EZNEXT(X,Z) = VERTICAL DISPERSION,NEXT TIME STEP (FT**2/SEC)
 FMT1(I) = FORMAT FOR HEADING FOR COMPUTED PROFILES
 FMT2(I) = FORMAT FOR DEPTH NOTATION FOR PROFILES
 FMT3(I) = FORMAT FOR OUTPUT OF COMPUTED PROFILES

*** GRID ADJUSTMENT PARAMETERS ***

NXSKIP = NUMBER OF X INTERVALS SKIPPED IN OLD GRID
 NZSKIP = NUMBER OF Z INTERVALS SKIPPED IN OLD GRID
 NXTAKE = LOWEST GRID NUMBER IN X DIRECTION IN THE OLD GRID
 FROM WHICH A CONCENTRATION IS TAKEN
 NZTAKE = LOWEST GRID NUMBER IN Z DIRECTION IN THE OLD GRID
 FROM WHICH A CONCENTRATION IS TAKEN
 NXPUT = LOWEST GRID NUMBER IN THE X DIRECTION IN THE NEW
 GRID INTO WHICH A CONCENTRATION IS PLACED
 NZPUT = LOWEST GRID NUMBER IN THE Z DIRECTION IN THE NEW
 GRID INTO WHICH A CONCENTRATION IS PLACED

 ***** MAIN PROGRAM *****

THE MAIN PROGRAM CALLS FOR THE DATA TO BE READ IN, THEN CALLS FOR THE STABILITY CONDITIONS TO BE EVALUATED, AND NEXT CALLS FOR THE PRINTING OF A SUMMARY OF INPUT DATA. THE NUMERICAL ANALYSIS SUBROUTINE IS THEN CALLED FOR EITHER THE ONE-DIMENSIONAL OR TWO-DIMENSIONAL CASE. IF THE TIME INCREMENT IS TO BE CHANGED DURING THIS PART OF THE PROGRAM, THE APPROPRIATE PARAMETERS ARE ADJUSTED. STABILITY IS AGAIN CHECKED AND THE NUMERICAL ANALYSIS IS CONTINUED. NEXT, A NEW DATA SET MAY BE READ TO CONTINUE THE ANALYSIS. ANY NUMBER OF DATA SETS MAY BE PROCESSED. WHEN NO MORE DATA SETS ARE AVAILABLE, THE PROGRAM IS TERMINATED.

THE INPUT AND OUTPUT OF DATA CAN BE CONTROLLED IN THREE WAYS--THE INPUT DATA ITSELF, THE BLOCK DATA SUBROUTINE, AND THE SUBROUTINE PRINT2. NO OTHER PART OF THE PROGRAM SHOULD REQUIRE CHANGING.

THE FORTRAN USED IN THIS PROGRAM USES SEVERAL OF THE OPTIONS AVAILABLE IN WATFOR AS DESCRIBED IN THE FOLLOWING TEXT**FORTRAN IV WITH WATFOR AND WATFIV**BY CRESS, DIRKSEN, GRAHAM, PRENTICE-HALL, INC., 1970. THE PROGRAM CAN BE RUN ON A COMPUTER WITH A WATFOR OR WATFIV COMPILER. THE SYNTAX HAS BEEN KEPT COMPATIBLE WITH FORTRAN 6 EXCEPT FOR THE UNFORMATTED READ AND PRINT STATEMENTS.

```

972 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,XPRINT,CCOUNT,OPTION
973 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
974 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
975 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
976 COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
      VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
      G(51),D(51),V(51)
977 10 CALL IDATA(61000)
978 CALL STABLI
979 CALL PRINT1
980 CALL PRINT2
981 CALL ERRORI(61000)
982 25 IF(NZ.EQ.1) GO TO 310
983 CALL TWODIM
984 GO TO 315
985 310 CALL ONEDIM
986 315 IF(ITER2.EQ.0) GO TO 10
987 ITER=ITER2
988 ITER2=0
989 DELTAT=DELT2*86400.
990 GO TO 25
991 1000 CONTINUE
992 STOP
993 END

```

```

994 C-----
      BLOCK DATA
C
C *****
C ***** BLOCK DATA *****
C *****
C
C
C THIS ROUTINE INITIALIZES SELECTED VARIABLES IN THE
C COMMON BLOCKS. THIS SUBPROGRAM MAY BE USED TO ESTABLISH
C INITIAL CONCENTRATION PROFILES IF THE USER DESIRES.
C-----
995 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
996 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
997 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
998 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAZ,DELTAZ,
      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
999 COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
      VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
      G(51),D(51),V(51)

```

```

C
C-----
C      ESTABLISH INITIAL CONCENTRATION IF DESIRED
C-----
1000 DATA C/2142*0.0/
1001 DATA VX,VZ,EX,EZ,CONCIN,DEMAND/6426*0.0/
1002 DATA W,KDAYS,KD/153*0.0/
1003 DATA EXMAX,EZMAX,VXMAX,VZMAX,WMAX,KDMAX,DEMAX,TIME/8*0.0/
1004 DATA EXMIN,EZMIN,VXMIN,VZMIN,WMIN,KDMIN,DEMIN/7*1000000./
1005 DATA INSECT/51*0/
1006 DATA IPAGE/1/
1007 DATA T/1/,TNEXT/2/
1008 DATA VXNEXT,VZNEXT,EXNEXT,EZNEXT,CSTAR/5355*0.0/
1009 DATA A,B,G,D,V/255*0.0/
1010 END

```



```

1011      SUBROUTINE IDATA(*)
C-----
C
C      *****
C      ***** SUBROUTINE IDATA *****
C      *****
C
C      THIS SUBROUTINE READS IN THE APPROPRIATE DATA SET AND ADJUSTS
C      THE GRID SIZE IF NECESSARY.  THE LOCATIONS OF THE UPPERMOST AND
C      LOWERMOST PEAKS ARE DETERMINED, AND THE MINIMUM AND MAXIMUM VALUES
C      FOR THE INPUT PARAMETERS ARE CALCULATED.  ALL OF THE INPUT DATA IS
C      PRINTED OUT UNFORMATTED.  THE FORMAT IS READ IN FOR THE PRINTING
C      OUT OF THE CALCULATED CONCENTRATIONS.
C-----
C
1012      INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CGOUNT,OPTION
1013      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
1014      COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1015      W(51),CGNCIN(51,21),INSECT(51),KDAY(51),KD(51),
2      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
C      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2      TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
3      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5      MSU,MSL,TIME,NXMI,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
C      COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
1      VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
2      G(51),D(51),V(51)

```

```

1017 READ(5,12,END=13) SET
1018 FORMAT(F10.0)
1019 GO TO 14
1020 13 CONTINUE
1021 RETURN
1022 14 CONTINUE
1023 IF(SET.EQ.1.) GO TO 400

```

C
C
C
C
C
C
C

 CHOOSE CONCENTRATIONS FROM THE OLD GRID TO PUT INTO THE
 APPROPRIATE PLACES IN THE NEW GRID . THE GRID CAN ONLY BE
 CHANGED IF LTYPE EQUALS 2. OTHERWISE, THE DATA CARD IS READ
 BUT IS NOT IMPLEMENTED.

```

1024 READ ,NXSKIP,NZSKIP,NXTAKE,NZTAKE,NXPUT,NZPUT
1025 IF(LTYPE.NE.2) GO TO 400
1026 NXSTOP=(NX-1)/NXSKIP+1
1027 NZSTOP=(NZ-1)/NZSKIP+1
1028 IF(NZ.EQ.1) NZSTOP=1
1029 DO 350 I=1,NXSTOP
1030 X=NXPUT-1+I
1031 II=NXTAKE+NXSKIP*(I-1)
1032 DO 350 J=1,NZSTOP
1033 Z=NZPUT-1+J
1034 JJ=NZTAKE+NZSKIP*(J-1)
1035 C(X,Z,INEXT)=C(II,JJ,T)
1036 350 CONTINUE
1037 400 CONTINUE

```

C
C

C-----
C READ AND PRINT INPUT DATA FOR HORIZONTAL DIRECTION
C-----

```

1038 READ,NX,NZ
1039 READ,DELMX,DELTIAZ
1040 READ,DELT1,ITER1
1041 READ,DELT2,ITER2
1042 READ,NC,NS
1043 READ,XSTART,TSTART,XPRINT,TPRINT,NPPAGE
1044 READ,LTYPE,OPTION
1045 PRINT 1001
1046 1001 FORMAT( IHL,IOX,'INPUT DATA',//,
1 IX,'NX,NZ, DELMX,DELTIAZ, DELT1,ITER1')
PRINT 1002
1047
1048 1002 FORMAT(//,IX,'DELT2,ITER2, NC,NS, XSTART,TSTART')
PRINT, DELT2,ITER2,NC,NS,XSTART,TSTART
PRINT 1003
1049
1050 1003 FORMAT(//,IX,'XPRINT,TPRINT,NPPAGE, LTYPE,OPTION')
PRINT, XPRINT,TPRINT,NPPAGE,LTYPE,OPTION
IF(SET.EQ.1.) GO TO 500
DO 375 Z=1,NZ
DO 375 X=1,NX
C(X,Z,I)=C(X,Z,TNEXT)
C(X,Z,TNEXT)=0.0
375 CONTINUE
500 CONTINUE
NXM1=NX-1
NXM2=NX-2
NZM1=NZ-1
1051
1052
1053
1054
1055
1056
1057
1058
1059
1060
1061
1062
1063

```

```

1064 NZM2=NZ-2
1065 DELTAT=DELT1*86400.
1066 DELTAX=DELMX*5280.
1067 XSF=XSTART*5280.
1068 ITER=ITER1
1069 TIME=TIME+TSTART*86400.
1070 COUNT=(TSTART/DELTAT+0.01)
1071 COUNT=COUNT-(COUNT/TPRINT)*TPRINT
1072 READ,((VX(X,Z),X=1,NX),Z=1,NZ)
1073 PRINT 2
1074 2 FORMAT(/, IX, '((VX(X,Z),X=1,NX),Z=1,NZ)')
1075 PRINT,((VX(X,Z),X=1,NX),Z=1,NZ)
1076 READ,((EX(X,Z),X=1,NX),Z=1,NZ)
1077 PRINT 3
1078 3 FORMAT(/, IX, '((EX(X,Z),X=1,NX),Z=1,NZ)')
1079 PRINT,((EX(X,Z),X=1,NX),Z=1,NZ)
1080 READ,(W(X),X=1,NX)
1081 PRINT 4
1082 4 FORMAT(/, IX, '(W(X),X=1,NX)')
1083 PRINT, (W(X),X=1,NX)
1084 READ,(KDAY(X),X=1,NX)
1085 PRINT 5
1086 5 FORMAT(/, IX, '(KDAY(X),X=1,NX)')
1087 PRINT, (KDAY(X),X=1,NX)
1088 READ,((DEMAND(X,Z),X=1,NX),Z=1,NZ)
1089 PRINT 6
1090 6 FORMAT(/, IX, '((DEMAND(X,Z),X=1,NX),Z=1,NZ)')
1091 PRINT, ((DEMAND(X,Z),X=1,NX),Z=1,NZ)
1092 IF(NC.EQ.0) GO TO 24
1093 DO 10 I=1,NC

```

```

1094 READ,INSECT(I)
1095 PRINT 7
1096 7 FORMAT(/, IX, 'INSECT(I)')
1097 PRINT,INSECT(I)
1098 READ, (CONCIN(I,J), J=1,NZ)
1099 PRINT 8
1100 8 FORMAT(/, IX, '(CONCIN(I,J),J=1,NZ)')
1101 PRINT,(CONCIN(I,J),J=1,NZ)
1102 10 CONTINUE
1103 DO 15 I=1,NC
1104 DO 15 Z=1,NZ
1105 C(INSECT(I),Z,T)=CONCIN(I,Z) +C(INSECT(I),Z,T)
1106 15 CONTINUE
C
C-----
C DETERMINE LOCATIONS OF UPPERMOST AND LOWERMOST
C PEAK CONCENTRATIONS
C-----
1107 AMAX1=0.0
1108 PEAKU=0.0
1109 AMAX2=0.0
1110 PEAKL=0.0
1111 UPPER=XSF
1112 LOWER=XSF
1113 MSU=1
1114 MSL=NX
1115 DO 17 I=1,NC
1116 DO 16 J=1,NZ
1117 IF(CONCIN(I,J).LT.PEAKU) GO TO 16
1118 PEAKU=CONCIN(I,J)
1119 16 CONTINUE

```

```

1120 IF(PEAKU.LE.AMAX1) GO TO 20
1121 AMAX1=PEAKU
1122 MSU=INSECT(I)
1123 UPPER=(MSU-1)*DELTA X +XSF
1124 17 CONTINUE
1125 20 CONTINUE
1126 DO 22 II=1,NC
1127 I=NC+1-II
1128 DO 21 J=1,NZ
1129 IF(CONCIN(I,J).LT.PEAKL) GO TO 21
1130 PEAKL=CONCIN(I,J)
1131 21 CONTINUE
1132 IF(PEAKL.LE.AMAX2) GO TO 24
1133 AMAX2=PEAKL
1134 MSL=INSECT(I)
1135 LOWER=(MSL-1)*DELTA X +XSF
1136 22 CONTINUE
1137 24 CONTINUE
C-----
C DETERMINE MAXIMUM AND MINIMUM VALUES FOR INPUT DATA
C-----
1138 DO 25 X=1,NX
1139 KD(X)=KDAY S(X)/86400.
1140 IF(W(X).GT.WMAX)WMAX=W(X)
1141 IF(W(X).LT.WMIN)WMIN=W(X)
1142 IF(KD(X).GT.KDMAX)KDMAX=KD(X)
1143 IF(KD(X).LT.KDMIN)KDMIN=KD(X)
1144 DO 25 Z=1,NZ

```

```

1145 ABSVX=ABS(VX(X,Z))
1146 IF(ABSVX.GT.VXMAX)VXMAX=ABSVX
1147 IF(ABSVX.LT.VXMIN)VXMIN=ABSVX
1148 IF(EX(X,Z).GT.EXMAX)EXMAX=EX(X,Z)
1149 IF(EX(X,Z).LT.EXMIN)EXMIN=EX(X,Z)
1150 IF(DEMAND(X,Z).GT.DEMAX)DEMAX=DEMAND(X,Z)
1151 IF(DEMAND(X,Z).LT.DEMIN)DEMIN=DEMAND(X,Z)
1152 25 CONTINUE
1153 IF(NZ.EQ.1)GO TO 200
C
C-----
C          READ AND PRINT DATA FOR VERTICAL DIRECTION
C-----
1154 READ,((VZ(X,Z),X=1,NX),Z=1,NZ)
1155 PRINT 30
1156 30 FORMAT(/,/, 1X, ' ((VZ((X,Z),X=1,NX),Z=1,NZ)')
1157 PRINT,((VZ(X,Z),X=1,NX),Z=1,NZ)
1158 READ,((EZ(X,Z),X=1,NX),Z=1,NZ)
1159 PRINT 32
1160 32 FORMAT(/,/, 1X, ' ((EZ((X,Z),X=1,NX),Z=1,NZ)')
1161 PRINT, ((EZ(X,Z),X=1,NX),Z=1,NZ)
C
C-----
C          DETERMINE MAXIMUM AND MINIMUM VALUES FOR INPUT DATA
C-----
1162 DO 120 X=1,NX
1163 DO 120 Z=1,NZ
1164 ABSVZ=ABS(VZ(X,Z))
1165 IF(ABSVZ.GT.VZMAX)VZMAX=ABSVZ
1166 IF(ABSVZ.LT.VZMIN)VZMIN=ABSVZ

```

```

1167 IF(EZ(X,Z).GT.EZMAX)EZMAX=EZ(X,Z)
1168 IF(EZ(X,Z).LT.EZMIN)EZMIN=EZ(X,Z)
1169 120 CONTINUE
1170 200 CONTINUE

```

C
C
C
C

READ AND PRINT FORMATS FOR OUTPUT

```

1171 READ 205, (FMT1(I), I=1,40)
1172 PRINT 202
1173 202 FORMAT('//,1X, 'FORMAT FOR OUTPUT')
1174 PRINT 205, (FMT1(I), I=1,40)
1175 205 FORMAT (20A4,/,20A4)
1176 READ 205,(FMT2(I),I=1,40)
1177 PRINT 205, (FMT2(I), I=1,40)
1178 READ 207,(FMT3(I), I=1,20)
1179 PRINT 207, (FMT3(I), I=1,20)
1180 207 FORMAT(20A4)

```

C
C
C
C
C

READ AND PRINT VELOCITIES AND DISPERSION COEFFICIENTS FOR
NEXT TIME STEP

```

1181 READ,((VXNEXT(X,Z),X=1,NX),Z=1,NZ)
1182 PRINT 210
1183 210 FORMAT('//,1X, '((VXNEXT(X,Z),X=1,NX),Z=1,NZ)')
1184 PRINT,((VXNEXT(X,Z),X=1,NX),Z=1,NZ)
1185 READ,((EXNEXT(X,Z),X=1,NX),Z=1,NZ)
1186 PRINT 215
1187 215 FORMAT('//,1X, '((EXNEXT(X,Z),X=1,NX),Z=1,NZ)')

```



```
1188 PRINT, ((EXNEXT(X,Z),X=1,NX),Z=1,NZ)
1189 IF(NZ.EQ.1) GO TO 250
1190 READ, ((VZNEXT(X,Z),X=1,NX),Z=1,NZ)
1191 PRINT 220
1192 FORMAT(/,1X,((VZNEXT(X,Z),X=1,NX),Z=1,NZ),)
1193 PRINT, ((VZNEXT(X,Z),X=1,NX),Z=1,NZ)
1194 READ, ((EZNEXT(X,Z),X=1,NX),Z=1,NZ)
1195 PRINT 225
1196 FORMAT(/,1X,((EZNEXT(X,Z),X=1,NX),Z=1,NZ),)
1197 PRINT, ((EZNEXT(X,Z),X=1,NX),Z=1,NZ)
1198 250 CONTINUE
1199 PRINT 259
1200 FORMAT(1H1)
1201 RETURN
1202 END
```

```

1203 C-----
      SUBROUTINE STABLI
C
C *****
C ***** SUBROUTINE STABLI *****
C *****
C
C THE RELATIONSHIP BETWEEN DISTANCE INCREMENTS, TIME INCREMENTS,
C DISPERSION COEFFICIENTS, AND VELOCITIES IS NEEDED TO DETERMINE
C THE STABILITY OF THE IMPLICIT FINITE-DIFFERENCE PROCEDURE. THIS
C SUBROUTINE CALCULATES THE MAXIMUM ALLOWABLE INCREMENTS FOR TIME
C AND DISTANCE WHICH GENERALLY GUARANTEE STABILITY.
C-----
      INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
      COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CCNCIN(51,21),INSECT(51),KDAY5(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
1 VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
2 G(51),D(51),V(51)
      STABLX = DELTAX
1209 STABLZ = DELTAZ
1210 STABLT = DELTAT
1211

```

```

C-----
C          CALCULATE STABILITY CRITERIA FOR ONE DIMENSIONAL CASE
C-----
1212      IF(NZ.NE.1) GO TO 500
1213      IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
1214      TERM=2.*EXMAX+DELTAZ**2.*KDMAX
1215      IF(TERM.NE.0.0) STABLT=(DELTAZ**2.)/TERM*2.
1216      R1=DELTAZ/(DELTAZ**2.)
1217      R2=0.
1218      GO TO 800
1219      500 CONTINUE

C-----
C          CALCULATE STABILITY CRITERIA FOR TWO DIMENSIONAL CASE
C-----
1220      IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
1221      IF(EZMIN.GT.0.0.AND.VZMAX.GT.0.0) STABLZ=2.*EZMIN/VZMAX
1222      TERM=2.*(EXMAX*DELTAZ**2.+EZMAX*DELTAZ**2.)*KDMAX*(DELTAZ**2)*
          $ (DELTAZ**2)
1223      IF(TERM.NE.0.0) STABLT=(DELTAZ**2.)*((DELTAZ**2.)/TERM *2.
1224      R1=DELTAZ/(DELTAZ**2.)
1225      R2=DELTAZ/(DELTAZ**2.)
1226      800 CONTINUE
1227      RETURN
1228      END

```

```

1229 C-----
      C SUBROUTINE ONEDIM
      C *****
      C ***** SUBROUTINE ONEDIM *****
      C *****
      C
      C THIS SUBPROGRAM CALLS FOR THE APPROPRIATE ROUTINES TO SOLVE
      C FOR THE CONCENTRATION PROFILE IN AN ESTUARY WHERE ONE-DIMENSIONAL
      C BEHAVIOR CAN BE ASSUMED. IN ADDITION, WIDTH OF THE ESTUARY CAN
      C VARY THROUGHOUT.
      C
      C THE SOLUTION MATRIX IS SET UP ACCORDING TO A CRANK-NICOLSON
      C IMPLICIT SCHEME IN SUBROUTINE ARRAY3 AND THIS TRIDIAGONAL MATRIX
      C IS SOLVED BY SUBROUTINE TRIDAG.
      C-----
1230 C INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
1231 C REAL KD,KDMAX,KDMIN,KDAYS,LOWER
1232 C COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      C W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
      C DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1233 C COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      C KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      C TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
      C VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      C DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      C MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      C COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
      C VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
      C G(51),D(51),V(51)
1234 C

```

```

1235 DO 300 IT=1,ITER
1236 TIME=TIME+DELTAT
1237 CALL ARRAY3
1238 CALL IBOUND
1239 Z=1
1240 DO 250 X=1,NX
1241 IF(C(X,Z,TNEXT).LT.0.0) C(X,Z,TNEXT)=0.0
1242 C(X,Z,T)=C(X,Z,TNEXT)
1243 C(X,Z,TNEXT)=0.0
1244 250 CONTINUE
1245 COUNT=COUNT+1
1246 IF(COUNT.GE.TPRINT) GO TO 292
1247 GO TO 300

```

C-----
C PRINT CONCENTRATIONS AT APPROPRIATE INTERVALS
C-----

```

1248 292 CONTINUE
1249 COUNT=0
1250 IPAGE=IPAGE+1
1251 IF(IPAGE.GE.NPPAGE) GO TO 295
1252 GO TO 297
1253 295 PRINT 296
1254 296 FORMAT(IH1)
1255 IPAGE = 0
1256 297 CALL PRINT2
1257 300 CONTINUE
1258 RETURN
1259 END

```

```

1260 C-----
      C SUBROUTINE TWODIM
      C *****
      C ***** SUBROUTINE TWODIM *****
      C *****
      C
      C THIS SUBPROGRAM CALLS FOR THE APPROPRIATE ROUTINES TO SOLVE
      C FOR THE CONCENTRATION PROFILE IN AN ESTUARY WHERE TWO DIMENSIONAL
      C BEHAVIOR CAN BE ASSUMED. IN ADDITION, WIDTH OF THE ESTUARY CAN
      C BE VARIED THROUGHOUT.
      C
      C THE SOLUTION MATRIX IS SET UP ACCORDING TO A CRANK-NICOLSON
      C IMPLICIT SCHEME WHICH IS SOLVED BY AN IMPLICIT,ALTERNATING
      C DIRECTION METHOD. SUBROUTINE ARRAY1 SETS UP THE MATRIX FOR THE
      C FIRST HALF OF THE SOLUTION AND SUBROUTINE ARRAY2 SETS UP THE
      C MATRIX FOR THE SECOND HALF. SUBROUTINE TRIDAG SOLVES BOTH OF
      C THESE MATRICES.
      C-----
1261 C INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
1262 C REAL KD,KDMAX,KDMIN,KDAYS,LCWER
1263 C COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      C W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
      C DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1264 C COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      C KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      C TPRINT,XPRINT,STABLX,STABLZ,STABL,R1,R2,VZMAX,SET,
      C VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      C DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      C MSU,MSL,TIME,NXMI,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION

```

```

1265 COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
1     VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
2     G(51),D(51),V(51)
1266 DO 300 IT=1,ITER
1267   TIME=TIME+DELTAT
1268   CALL ARRAY1
1269   CALL ARRAY2
1270   CALL IBOUND
1271   DO 250 Z=1,NZ
1272     DO 250 X=1,NX
1273       IF(C(X,Z,TNEXT).LT.0.0) C(X,Z,TNEXT)=0.0
1274       C(X,Z,T)=C(X,Z,TNEXT)
1275       C(X,Z,TNEXT)=0.0
1276     250 CONTINUE
1277     COUNT=COUNT+1
1278     IF(COUNT.GE.TPRINT) GO TO 292
1279     GO TO 300
C-----
C     PRINT CONCENTRATIONS AT APPROPRIATE INTERVALS
C-----
1280 292 CONTINUE
1281   COUNT=0
1282   IPAGE=IPAGE+1
1283   IF(IPAGE.GE.NPPAGE) GO TO 295
1284   GO TO 297
1285 295 PRINT 296
1286 296 FORMAT(1H1)

```

```
1287 IPAGE = 0  
1288 297 CALL PRINT2  
1289 300 CONTINUE  
1290 RETURN  
1291 END
```



```

C-----
1292      SUBROUTINE ARRAY1
C
C      *****
C      ***** SUBROUTINE ARRAY1 *****
C      *****
C
C-----
1293      INTEGER X,Z,T,TAEXT,TPRINT,XPRINT,COUNT,OPTION
1294      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
1295      COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      W(51),CCNCIN(51,21),INSECT(51),KDAY(51),KD(51),
      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1296      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      TPRINT,XPRINT,STABLX,STABLZ,STABL,RI,R2,VZMAX,SET,
      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTIGN
1297      COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
      VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
      G(51),D(51),V(51)
C
1298      ZUP=UPPER
1299      ZLOW=LOWER
1300      ZSF=XSF
1301      FOURX=4.*DELTAZ
1302      FORXSQ=4.*DELTAZ**2
1303      FOURZ=4.*DELTAZ
1304      FORZSQ=4.*DELTAZ**2

```

```

C
C-----
C      SET UP THE ARRAY ACCORDING TO A CRANK-NICOLSON
C      IMPLICIT SCHEME
C-----
1305      DO 1005 Z=2,NZM1
1306      DO 100 X=2,NXM1
1307      A(X)=(DELTA/W(X))*(-W(X)*VXNEXT(X,Z)/FOURX-(W(X)*EXNEXT(X,Z)
          $   +W(X-1)*EXNEXT(X-1,Z))/FORXSQ)
1308      B(X)=1.+(DELTA/W(X))*((W(X+1)*EXNEXT(X+1,Z)+2.*W(X)*EXNEXT(X,Z)
          $   +W(X-1)*EXNEXT(X-1,Z))/FORXSQ)
1309      G(X)=(DELTA/W(X))*W(X)*VXNEXT(X,Z)/FOURX-(W(X+1)*EXNEXT(X+1,Z)
          $   +W(X)*EXNEXT(X,Z))/FORXSQ)
1310      D(X)=C(X,Z,T)+(DELTA/W(X))*(-(W(X)*VX(X,Z)/FOURX)*(C(X+1,Z,T)
          1   -C(X-1,Z,T))+((W(X+1)*EX(X+1,Z)+W(X)*EX(X,Z))/FORXSQ)
          2   *(C(X+1,Z,T)-C(X,Z,T))-((W(X) )*EX(X,Z)+W(X-1)*EX(X-1,Z)
          3   /FORXSQ)*(C(X,Z,T)-C(X-1,Z,T)))
1311      D(X)=D(X)+(DELTA/W(X))*(-2.*(W(X)*VZ(X,Z)/FOURZ)*(C(X,Z+1,T)
          1   -C(X,Z-1,T))+2.*((W(X)*EZ(X,Z+1)+W(X)*EZ(X,Z))/FORZSQ)
          2   *(C(X,Z+1,T)-C(X ,Z,T))-2.*((W(X)*EZ(X,Z)+W(X)*EZ(X,Z-1)
          3   /FORZSQ)*(C(X,Z,T)-C(X,Z-1,T)))
1312      100 CONTINUE
1313      NFLAG=1
C-----
C      EVALUATE BOUNDARY VALUES BY EXPONENTIAL EXTRAPOLATION OR BY
C      CONSTANT SLOPE EXTRAPOLATION DEPENDING ON VALUE OF OPTION
C-----

```

```

1314 GO TO (101,202,101,202), OPTION
1315 CONTINUE
1316 VX1=VXNEXT(1,Z)
1317 XX=-UPPER+DELTA*XS*VX1*START*86400.
1318 EX1=EXNEXT(1,Z)
1319 C2=4.*EX1*TIME
1320 EXPON=( 2.*XX*DELTA-(DELTA*DELTA)-2.*DELTA*VX1*TIME)/C2
1321 P=EXP(EXPON)
1322 B(2)=B(2)+A(2)*P
1323 CSTAR(1,Z)=CSTAR(2,Z)*P
1324 CONTINUE
1325 VXNX=VXNEXT(NX,Z)
1326 XX=(NX-2)*DELTA-LOWER+XS*VXNX*START*86400.
1327 EXNX=EXNEXT(NX,Z)
1328 C2=4.*EXNX*TIME
1329 EXPON=(-2.*XX*DELTA-(DELTA*DELTA)+2.*DELTA*VXNX*TIME)/C2
1330 P=EXP(EXPON)
1331 B(NXM1)=B(NXM1)+G(NXM1)*P
1332 CSTAR(NX,Z)=CSTAR(NXM1,Z)*P
1333 GO TO 1000
1334 CONTINUE
1335 B(2)=B(2)+2.*A(2)
1336 G(2)=G(2)-A(2)
1337 A(NX-1)=A(NX-1)-G(NX-1)
1338 B(NX-1)=B(NX-1)+2.*A(NX-1)
1339 CSTAR(1,Z)=2.*CSTAR(2,Z)-CSTAR(3,Z)
1340 IF(CSTAR(1,Z).LT.0.) CSTAR(1,Z)=0.
1341 CSTAR(NX,Z)=2.*CSTAR(NXM1,Z)-CSTAR(NXM2,Z)
1342 IF(CSTAR(NX,Z).LT.0.) CSTAR(NX,Z)=0.
1343 CONTINUE
1000 CONTINUE

```

```
1344 IF(NFLAG.GT.1) GO TO 1005
1345 NFLAG=2
C-----
C SOLVE TRIDIAGONAL MATRIX
C-----
1346 CALL TRIDAG(2,NXMI)
C-----
C STORE HALF-STEP SOLUTION IN CSTAR
C-----
1347 DO 1004 X=2,NXMI
1348 CSTAR(X,Z) =V(X)
1349 1004 CONTINUE
1350 GO TO (101,202,101,202),CPTION
1351 1005 CONTINUE
1352 RETURN
1353 END
```

```

1354 C-----
      SUBROUTINE ARRAY2
C
C *****
C ***** SUBROUTINE ARRAY2 *****
C *****
C
C
C-----
      INTEGER X,Z,T,INEXT,TPRINT,XPRINT,CCOUNT,OPTION
      REAL KD,KD MAX,KDMIN,KDAYS,LOWER
      COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      1 KD MAX,KDMIN,ITER,ITER1,ITER2,DEL T1,DEL T2,TNEXT,
      2 TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
      3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DEL TAX,DELTAZ,
      4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
      1 VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
      2 G(51),O(51),V(51)
      ZUP=UPPER
      ZLOW=LOWER
      ZSF=XSF
      FOURX=4.*DEL TAX
      FORXSQ=4.*DEL TAX**2
      FOURZ=4.*DELTAZ
      FORZSQ=4.*DELTAZ**2
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C-----
C      SET UP THE ARRAY ACCORDING TO A CRANK-NICOLSON
C      IMPLICIT SCHEME
C-----
1367 DO 1005 X=2,NXMI
1368 DO 100 Z=2,NZMI
1369 A(Z)=(DELTA/W(X))*(-W(X)*VZNEXT(X,Z)/FOURZ-(W(X)*EZNEXT(X,Z)
      $ +W(X)*EZNEXT(X,Z-1))/FORZSQ)
1370 B(Z)=1.+(DELTA/W(X))*(W(X)*EZNEXT(X,Z+1)+2.*W(X)*EZNEXT(X,Z)
      $ +W(X)*EZNEXT(X,Z-1))/FORZSQ)
1371 G(Z)=(DELTA/W(X))*(W(X)*VZNEXT(X,Z)/FOURZ-(W(X)*EZNEXT(X,Z+1)
      $ +W(X)*EZNEXT(X,Z))/FORZSQ)
1372 D(Z)=C(X,Z,T)+(DELTA/W(X))*(-(W(X)*VXNEXT(X,Z)/FOURX)*(CSTAR(
      1 X+1,Z)-CSTAR(X-1,Z))-(W(X)*VX(X,Z)/FOURX)*(C(X+1,Z,T)
      2 -C(X-1,Z,T))-(W(X)*VZ(X,Z)/FOURZ)*(C(X,Z+1,T)-C(X,Z-1,T))
      3 +((W(X+1)*EXNEXT(X+1,Z)+W(X)*EXNEXT(X,Z))/FORXSQ)*(CSTAR(
      4 X+1,Z)-CSTAR(X,Z))+((W(X+1)*EX(X+1,Z)+W(X)*EX(X,Z))/FORXSQ)
      5 *(C(X+1,Z,T)-C(X,Z,T)))
1373 D(Z)=D(Z)+(DELTA/W(X))*(-(W(X)*EXNEXT(X,Z)+W(X-1)*EXNEXT(X-1,Z))
      1 /FORXSQ)*(CSTAR(X,Z)-CSTAR(X-1,Z))-((W(X)*EX(X,Z)+W(X-1)
      2 *EX(X-1,Z))/FORXSQ)*(C(X,Z,T)-C(X-1,Z,T))+((W(X)*EZ(X,Z+1)
      3 +W(X)*EZ(X,Z))/FORZSQ)*(C(X,Z+1,T)-C(X,Z,T))-((W(X)*EZ(X,Z)
      4 +W(X)*EZ(X,Z-1))/FORZSQ)*(C(X,Z,T)-C(X,Z-1,T)))
1374 100 CONTINUE
C-----
C      EVALUATE BOUNDARY VALUES BY EXPONENTIAL EXTRAPOLATION OR BY
C      CONSTANT SLOPE EXTRAPOLATION DEPENDING ON VALUE OF OPTION
C-----

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1375 GO TO (303,303,101,202), CPTION
1376 CONTINUE
1377 VZ1=VZNEXT(X,1)
1378 ZZ=-ZUP+DELTAZ+ZSF+VZ1*TSTART*86400.
1379 EZ1=EZNEXT(X,1)
1380 C2=4.*EZ1*TIME
1381 EXPON=( 2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)-2.*DELTAZ*VZ1*TIME)/C2
1382 B(2)=B(2)+A(2)*EXP(EXPCN)
1383 CONTINUE
1384 VZNZ=VZNEXT(X,NZ)
1385 ZZ=(NZ-2)*DELTAZ-ZLOW+ZSF+VZNZ*TSTART*86400.
1386 EZNZ=EZNEXT(X,NZ)
1387 C2=4.*EZNZ*TIME
1388 EXPON=(-2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)+2.*DELTAZ*VZNZ*TIME)/C2
1389 B(NZ-1)=B(NZ-1)+G(NZ-1)*EXP(EXPON)
1390 GO TO 1000
1391 CONTINUE
1392 B(2)=B(2)+2.*A(2)
1393 G(2)=G(2)-A(2)
1394 A(NZ-1)=A(NZ-1)-G(NZ-1)
1395 B(NZ-1)=B(NZ-1)+2.*A(NZ-1)
1396 GO TO 1000
1397 B(2)=B(2)+A(2)
1398 B(NZM1)=B(NZM1)+G(NZM1)
1399 CONTINUE

```

```
C-----  
C      SOLVE TRIDIAGONAL MATRIX  
C-----  
1400      CALL TRIDAG(2,NZM1)  
C-----  
C      SUBTRACT DECAY AND OTHER DEMANDS AND  
C      PLACE FINAL SOLUTION IN C(X,Z,T+1)  
C-----  
1401      DO 1004 Z=2,NZM1  
1402          C(X,Z,T+1)=V(Z)-DELTA*(KD(X)/2.*(V(Z)+C(X,Z,T)))-DEMAND(X,Z)  
1403          1004 CONTINUE  
1404          1005 CONTINUE  
1405          RETURN  
1406          END
```



```

1407 C-----
      SUBROUTINE ARRAY3
C
C *****
C ***** SUBROUTINE ARRAY3 *****
C *****
C
C-----
1408 C      INTEGER X,Z,T,TNEXT,IPRINT,XPRINT,COUNT,OPTION
1409 C      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
1410 C      COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      C      W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
      C      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1411 C      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      C      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      C      TPRINT,XPRINT,STABLX,STABLZ,STABLI,R1,R2,VZMAX,SET,
      C      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      C      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      C      MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTICN
1412 C      COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
      C      VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
      C      G(51),D(51),V(51)
1413 C      FOURX=4.*DELTAX
1414 C      FORXSQ=4.*DELTAX**2
1415 C      Z=1
C-----
C      SET UP THE ARRAY ACCORDING TO A CRANK--NICOLSON
C      IMPLICIT SCHEME
C-----
1416 C      DO 100 X=2,NXM1
1417 C      A(X)=(DELTAT/W(X))*(-W(X)*VXNEXT(X,Z)/FOURX-(W(X)*EXNEXT(X,Z)
      C      $      +W(X-1)*EXNEXT(X-1,Z))/FORXSQ)

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1418 B(X)=1.+(DELTA/W(X))*((W(X+1)*EXNEXT(X+1,Z)+2.*W(X)*EXNEXT(X,Z)
$      +W(X-1)*EXNEXT(X-1,Z))/FORXSQ+KD(X)/2.*W(X))
1419 G(X)=(DELTA/W(X))*((W(X)*VXNEXT(X,Z))/FOURX-(W(X+1)*EXNEXT(X+1,Z)
$      +W(X)*EXNEXT(X,Z))/FORXSQ)
1420 D(X)=C(X,Z,T)+(DELTA/W(X))*(-(W(X)*VX(X,Z))/FOURX)*{C(X+1,Z,T)
1      -C(X-1,Z,T)}+(W(X+1)*EX(X+1,Z)+W(X)*EX(X,Z))/FORXSQ)
2      *(C(X+1,Z,T)-C(X,Z,T))-((W(X) )*EX(X,Z)+W(X-1)*EX(X-1,Z))
3      /FORXSQ)*{C(X,Z,T)-C(X-1,Z,T)}-(KD(X)/2.)*C(X,Z,T)*W(X))
1421 100 CONTINUE
C
C-----
C      EVALUATE BOUNDARY VALUES BY EXPONENTIAL EXTRAPOLATION OR BY
C      CONSTANT SLOPE EXTRAPOLATION DEPENDING ON VALUE OF OPTION
C-----
1422 GO TO (101,202,101,202), OPTION
1423 101 CONTINUE
1424 VX1=VXNEXT(1,Z)
1425 XX=-UPPER+DELTA*XSF+VX1*TSTART*86400.
1426 EX1=EXNEXT(1,Z)
1427 C2=4.*EX1*TIME
1428 EXPON=( 2.*XX*DELTA-(DELTA*DELTA)-(DELTA*VX1*TIME)/C2
1429 B(2)=B(2)+A(2)*EXP(EXPCN)
1430 150 CONTINUE
1431 VXNX=VXNEXT(NX,Z)

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1432 XX=(NX-2)*DELTA-LOWER+XSF+VXNX*TSTART*86400.
1433 EXNX=EXNEXT(NX,Z)
1434 C2=4.*EXNX*TIME
1435 EXPON=(-2.*XX*DELTA-(DELTA*DELTA)+2.*DELTA*VXNX*TIME)/C2
1436 B(NX-1)=B(NX-1)+G(NX-1)*EXP(EXPON)
1437 GO TO 1000
1438 202 CONTINUE
1439 B(2)=B(2)+2.*A(2)
1440 G(2)=G(2)-A(2)
1441 A(NX-1)=A(NX-1)-G(NX-1)
1442 B(NX-1)=B(NX-1)+2.*A(NX-1)
1443 1000 CONTINUE
C
C-----
C      SOLVE TRIDIAGONAL MATRIX
C-----
C
1444 CALL TRIDAG(2,NXM1)
C-----
C      SUBTRACT OTHER DEMANDS AND PLACE FINAL SOLUTION IN C(X,Z,T+1)
C-----
1445 DO 1004 X=2,NXM1
1446 C(X,Z,T+1)=V(X)-DEMAND(X,Z)
1447 1004 CONTINUE
1448 RETURN
1449 END

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```

1450 C-----
      C SUBROUTINE TRIDAG(IF,L)
      C
      C *****
      C ***** SUBROUTINE TRIDAG *****
      C *****
      C
      C THIS SUBROUTINE SOLVES A SYSTEM OF LINEAR SIMULTANEOUS
      C EQUATIONS HAVING A TRIDIAGONAL COEFFICIENT MATRIX.
      C THE EQUATIONS ARE NUMBERED FROM IF THROUGH L, AND THEIR
      C SUB-DIAGONAL, DIAGONAL, AND SUPER-DIAGONAL COEFFICIENTS
      C ARE STORED IN THE ARRAYS A, B, AND G. THE COMPUTED
      C SOLUTION VECTOR V(IF) TO V(L) IS STORED IN THE ARRAY V.
      C
      C THIS SUBPROGRAM IS A MODIFIED VERSION OF A PROGRAM FOUND IN THE
      C FOLLOWING TEXT**APPLIED NUMERICAL METHODS**BY CARNAHAN, LUTHER,
      C AND WILKES, JOHN WILEY&SONS, INC., 1969, P446.
      C
      C-----
1451 C INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
1452 C REAL KD,KD MAX,KDMIN,KDAYS,LOWER
1453 C COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      C W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
      C DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      C COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      C KDMAX,KDMIN,ITER,ITER1,ITER2,DELTA1,DELTA2,TNEXT,
      C TPRINT,XPRINT,STABLX,STABLZ,STABLR1,R2,VZMAX,SET,
      C VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAZ,DELTAZ,
      C DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      C MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
1454

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1455 COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
1     VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
2     G(51),D(51),V(51)
1456 DIMENSION BETA(51),GAMMA(51)
C
C-----
C     COMPUTE THE INTERMEDIATE ARRAYS RETA AND GAMMA
C-----
1457 BETA(IF)=B(IF)
1458 GAMMA(IF)=D(IF)/BETA(IF)
1459 IFP1=IF+1
1460 DO 1 I=IFP1,L
1461 BETA(I)=B(I)-A(I)*G(I-1)/BETA(I-1)
1462 GAMMA(I)=(D(I)-A(I)*GAMMA(I-1))/BETA(I)
C
C-----
C     COMPUTE FINAL SOLUTION VECTOR V
C-----
1463 V(L)=GAMMA(L)
1464 LAST=L-IF
1465 DO 2 K=1,LAST
1466 I=L-K
1467 V(I)=GAMMA(I)-G(I)*V(I+1)/BETA(I)
1468 RETURN
1469 END

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C-----
1470 SUBROUTINE IBOUND
C
C *****
C ***** SUBROUTINE IBOUND *****
C *****
C
C THIS FORM OF SUBROUTINE BOUND EXTRAPOLATES THE CONCENTRATIONS
C AT THE BOUNDARIES BY USING AN EXPONENTIAL EQUATION APPROPRIATE TO
C CONCENTRATION PROFILES IN AN ESTUARY OR BY A CONSTANT SLOPE
C EXTRAPOLATION. THE TYPE OF EXTRAPOLATION AT THE BOUNDARIES
C DEPENDS ON THE VALUE FOR THE VARIABLE OPTION.
C-----
1471 INTEGER X,Z,T,TNEXT,TPRINT,TADMIN,KDAYS,LOWER
1472 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
1473 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1474 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABL1,STABL2,STABL3,STABL4,STABL5,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
1475 COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
1 VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
2 G(51),D(51),V(51)

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1476 IF(NZ.EQ.1) NZM1=1
1477 ZUP=UPPER
1478 ZLOW=LOWER
1479 ZSF=XSF
1480 GO TO (1001,1002,1003,1004,1005,1002,1004), OPTION
C
C-----
C      EXPONENTIAL EXTRAPOLATION
C-----
1003 CONTINUE
1481 IF(NZ.EQ.1) GO TO 1001
1482 DO 140 X=2,NXM1
1483 IF(C(X,2,T+1).LE.0.0) GO TO 140
1484 VZ1=VZNEXT(X,1)
1485 ZZ=-ZUP+DELTAZ+ZSF+VZ1*TSTART*86400.
1486 EZ1=EZNEXT(X,1)
1487 C2=4.*EZ1*TIME
1488 EXPON=(-2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)-2.*DELTAZ*VZ1*TIME)/C2
1489 C(X,1,T+1)=C(X,2,T+1)*EXP(EXPON)
1490
1491 140 CONTINUE
1492 DO 190 X=2,NXM1
1493 IF(C(X,NZM1,T+1).LE.0.0) GO TO 190
1494 VZNZ=VZNEXT(X,NZ)
1495 ZZ=(NZ-2)*DELTAZ-ZLOW+ZSF+VZNZ*TSTART*86400.
1496 EZNZ=EZNEXT(X,NZ)
1497 C2=4.*EZNZ*TIME
1498 EXPON=(-2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)+2.*DELTAZ*VZNZ*TIME)/C2
1499 C(X,NZ,T+1)=C(X,NZM1,T+1)*EXP(EXPON)
1500
1501 190 CONTINUE
1502 IF(LTYPE.EQ.1.AND.MSU.EQ.1) GO TO 350

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1503 DO 340 Z=1,NZ
1504 IF(C(2,Z,T+1).LE.0.0) GO TO 340
1505 VX1=VXNEXT(1,Z)
1506 XX=-UPPER+DELTA*XS*VF+VX1*START*86400.
1507 EX1=EXNEXT(1,Z)
1508 C2=4.*EX1*TIME
1509 EXPON=(2.*XX*DELTA-(DELTA*DELTA)-2.*DELTA*VX1*TIME)/C2
1510 C(1,Z,T+1)=C(2,Z,T+1)*EXP(EXPON)
1511 CONTINUE
1512 CONTINUE
1513 IF(LTYPE.EQ.1.AND.MSL.EQ.NZ) GO TO 400
1514 DO 390 Z=1,NZ
1515 IF(C(NX-1,Z,T+1).LE.0.0) GO TO 390
1516 VXNX=VXNEXT(NX,Z)
1517 XX=(NX-2)*DELTA-LOWER+XS*VF+VXNX*START*86400.
1518 EXNX=EXNEXT(NX,Z)
1519 C2=4.*EXNX*TIME
1520 EXPON=(-2.*XX*DELTA-(DELTA*DELTA)+2.*DELTA*VXNX*TIME)/C2
1521 C(NX,Z,T+1)=C(NX-1,Z,T+1)*EXP(EXPON)
1522 CONTINUE
1523 GO TO 2000
1524
C
C-----
C CONSTANT SLOPE EXTRAPOLATION
C-----
1004 CONTINUE
IF(NZ.EQ.1) GO TO 1002

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1527 DO 540 X=2,NXMI
1528 C(X,1,T+1)=2.*C(X,2,T+1)-C(X,3,T+1)
1529 IF(C(X,1,T+1).LT.0.0) C(X,1,T+1)=0.0
1530 C(X,NZ,T+1)=2.*C(X,NZM1,T+1)-C(X,NZM2,T+1)
1531 IF(C(X,NZ,T+1).LT.0.0) C(X,NZ,T+1)=0.0
1532 540 CONTINUE
1533 1002 CONTINUE
1534 DO 590 Z=1,NZ
1535 C(1,Z,T+1)=2.*C(2,Z,T+1)-C(3,Z,T+1)
1536 IF(C(1,Z,T+1).LT.0.0) C(1,Z,T+1)=0.0
1537 C(NX,Z,T+1)=2.*C(NXMI,Z,T+1)-C(NXM2,Z,T+1)
1538 IF(C(NX,Z,T+1).LT.0.0) C(NX,Z,T+1)=0.0
1539 590 CONTINUE
1540 GO TO 2000
C
C-----
C THIS OPTION ALLOWS THE PROGRAM USER TO SUBSTITUTE HIS OWN
C EXTRAPOLATION ROUTINE. THE USER MUST BE CAREFUL TO CHANGE
C THE APPROPRIATE ARRAY VALUES.
C-----
1541 1005 CONTINUE
1542 OPTION=2
1543 GO TO 1002
1544 2000 CONTINUE
1545 IF(NZ.EQ.1) GO TO 2005
1546 IF(OPTION.EQ.3.OR.OPTION.EQ.4.OR.OPTION.EQ.5) GO TO 2005
1547 DO 700 X=1,NX
1548 C(X,1,T+1)=C(X,2,T+1)
1549 C(X,NZ,T+1)=C(X,NZM1,T+1)
1550 700 CONTINUE
1551 2005 CONTINUE
C

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C-----
C   IF LTYPE EQUALS 1, SET CONCENTRATIONS AT SOURCE POINTS.
C   IF LTYPE EQUALS 3, THE USER CAN INCLUDE A SPECIAL SUBROUTINE
C   OR SET OF CALCULATIONS IN THIS PART OF THE PROGRAM.
C-----
      IF(LTYPE.EQ.1.AND.NS.LE.2) GO TO 275
      IF(LTYPE.EQ.1.AND.NS.GT.2) GO TO 281
      GO TO 290
275 CONTINUE
      DO 280 Z=1,NZ
      C(MSU,Z,T+1)=C(MSU,Z,T)
      C(MSL,Z,T+1)=C(MSL,Z,T)
280 CONTINUE
      GO TO 290
281 CONTINUE
      DO 285 I=1,NC
      DO 285 Z=1,NZ
      C(INSECT(I),Z,T+1)=CGNCIN(I,Z)
285 CONTINUE
290 CONTINUE
      RETURN
      END

```

```

C-----
1569 SUBROUTINE PRINT1
C
C *****
C ***** SUBROUTINE PRINT1 *****
C *****
C
C THIS SUBROUTINE CALCULATES THE CONVERSION VALUES FOR MANY OF
C THE INPUT PARAMETERS AND PRINTS OUT A SUMMARY OF THE INPUT DATA
C-----
1570 INTEGER X,Z,T,INEXT,TPRINT,XPRINT,CCOUNT,OPTION
1571 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
1572 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1573 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DEL1,DEL2,INEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABLR1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAZ,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTIGN
1574 COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
1 VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
2 G(51),D(51),V(51)
C-----
C CALCULATE CONVERSION UNITS
C-----

```

1575 UNITS1 = 86400./5280.
 1576 UNITS2 = 86400./5280.*5280.)
 1577 DELDAY = DELTAT/86400.
 1578 SECS1=DELT1*86400.
 1579 SECS2=DELT2*86400.
 1580 DELMX=DELTAX/5280.
 1581 DELMZ=DELTAZ/5280.
 1582 TOTALX=(NX-1)*DELTAX
 1583 TOTMX=TOTALX/5280.
 1584 TOTALZ=(NZ-1)*DELTAZ
 1585 TOTMZ=TOTALZ/5280.
 1586 VX1=VXMAX*UNITS1
 1587 VX2=VXMIN*UNITS1
 1588 VZ1=VZMAX*UNITS1
 1589 IF(VZMIN.EQ.1000000.) VZMIN=0.
 1590 VZ2=VZMIN*UNITS1
 1591 EX1=EXMAX*UNITS2
 1592 EX2=EXMIN*UNITS2
 1593 EZ1=EZMAX*UNITS2
 1594 IF(EZMIN.EQ.1000000.) EZMIN=0.
 1595 EZ2=EZMIN*UNITS2
 1596 XKD1=KDMAX*86400.
 1597 XKD2=KDMIN*86400.
 1598 SXM=STABLX/5280.
 1599 SZM=STABLZ/5280.
 1600 STD=STABLT/86400.

C-----
 C PRINT DATA SUMMARY
 C-----

```

1601 PRINT 500
1602 PRINT 505
1603 PRINT 507
1604 PRINT 510
1605 PRINT 507
1606 IF(NZ.EQ.1) PRINT 515
1607 IF(NZ.NE.1) PRINT 520
1608 PRINT 507
1609 IF(LTYPE.EQ.1) GO TO 7
1610 IF(LTYPE.EQ.2) GC TC 5
1611 PRINT 530
1612 GO TO 10
1613 5 PRINT 525
1614 GO TO 10
1615 7 PRINT 523
1616 10 CONTINUE
1617 PRINT 507
1618 PRINT 535
1619 PRINT 507
1620 PRINT 505
1621 PRINT 540, DELTAX, DELMX
1622 PRINT 545, DELTAZ, DELMZ
1623 PRINT 550, SECSI, DELTI
1624 PRINT 551, ITER1
1625 PRINT 552, SECS2, DELT2
1626 PRINT 553, ITER2
1627 PRINT 555, NX
1628 PRINT 560, NZ
1629 PRINT 565, VXMAX, VX1
1630 PRINT 570, VXMIN, VX2

```

```

1631 PRINT 572, VZMAX, VZ1
1632 PRINT 573, VZMIN, VZ2
1633 PRINT 575, EXMAX, EX1
1634 PRINT 580, EXMIN, EX2
1635 PRINT 585, EZMAX, EZ1
1636 PRINT 590, EZMIN, EZ2
1637 PRINT 600, WMAX
1638 PRINT 605, WMIN
1639 PRINT 610, XKD1, KDMAX
1640 PRINT 615, XKD2, KOMIN
1641 PRINT 620, DEMAX
1642 PRINT 630, NC

```

C

C

 C XUPEAK AND XLPEAK ARE CALCULATED ASSUMING DISTANCES ARE
 C DECREASING IN THE DOWNSTREAM DIRECTION
 C-----

```

1643 XUPEAK = XSTART - (MSU-1)*DELMX
1644 XLPEAK = XSTART - (MSL-1)*DELMX
1645 PRINT 635, XUPEAK, MSU
1646 PRINT 637, XLPEAK, MSL
1647 PRINT 500
1648 PRINT 505
1649 PRINT 502
1650 PRINT 640
1651 PRINT 645, STABLX, SXM
1652 PRINT 650, STABLZ, SZM
1653 PRINT 655, STABLT, STD
1654 PRINT 660, R1
1655 PRINT 665, R2

```

```

1656 PRINT 505
1657 IF (NC.EQ.0) GO TO 100
1658 PRINT 500
1659 PRINT 700
1660 IF (NZ.EQ.1) GO TO 50
1661 DO 25 I=1,NC
C
C-----
C WHERE IS CALCULATED ASSUMING DISTANCES ARE DECREASING IN
C THE DOWNSTREAM DIRECTION
C-----
1662 WHERE=XSTART-(INSECT(I)-1)*DELMX
1663 PRINT 710, INSECT(I),WHERE,DELTAZ
1664 DO 25 Z=1,NZ
1665 PRINT 715,INSECT(I),Z,CONCIN(I,Z)
1666 25 CONTINUE
1667 GO TO 100
1668 50 DO 75 I=1,NC
1669 WHERE=XSTART-(INSECT(I)-1)*DELMX
1670 PRINT 705, INSECT(I), WHERE, CONCIN(I,1)
1671 PRINT 502
1672 75 CONTINUE
1673 100 CONTINUE
1674 PRINT 500
1675 500 FORMAT(IH1)
1676 502 FORMAT(/)
1677 505 FORMAT(T45, 38('**'))
1678 507 FORMAT(T45,'**',T82,'**')
1679 510 FORMAT(T45,'**',T56,'ESTUARY SIMULATION', T82, '**')
1680 515 FORMAT(T45, '**', T48, 'ONE DIMENSIONAL IMPLICIT METHOD',T82,'**')

```

```

1681      FORMAT(T45, '**', T48, 'TWO DIMENSIONAL IMPLICIT METHOD', T82, '**')
1682      FORMAT(T45, '**', T53, 'CONSTANT CONCENTRATION', T82, '**')
1683      FORMAT(T45, '**', T54, 'INSTANTANEOUS RELEASE', T82, '**')
1684      FORMAT(T45, '**', T56, 'CONTINUOUS LOADING', T82, '**')
1685      FORMAT(T45, '**', T51, 'PROGRAMMER -- JONATHAN YOUNG', T82, '**')
1686      FORMAT( /, T53, 'X INCREMENT =', F7.0, ' FEET (', F8.5, ' MILES
          $)', /)
1687      FORMAT(T53, 'Z INCREMENT =', F7.0, ' FEET (', E10.3, ' MILES )', /)
1688      FORMAT(T42, 'INITIAL TIME INCREMENT =', F7.0, ' SECONDS (', F6.4,
          $ ' DAYS )', /)
1689      FORMAT(T44, 'NUMBER OF ITERATIONS =', I5, /)
1690      FORMAT( T42, 'REVISED TIME INCREMENT =', F7.0, ' SECONDS( ', F6.4,
          $ ' DAYS )', /)
1691      FORMAT(T44, 'NUMBER OF ITERATIONS =', I5, /)
1692      FORMAT(T37, 'NUMBER OF HORIZONTAL POINTS =', I4, /)
1693      FORMAT(T39, 'NUMBER OF VERTICAL POINTS =', I4, /)
1694      FORMAT(T37, 'MAXIMUM HORIZONTAL VELOCITY =', F5.2, ' FEET/SECOND
          $(, F6.2, ' MILES/DAY )', /)
1695      FORMAT(T37, 'MINIMUM HORIZONTAL VELOCITY =', F5.2, ' FEET/SECCND
          $(, F6.2, ' MILES/DAY )', /)
1696      FORMAT(T39, 'MAXIMUM VERTICAL VELOCITY =', E10.3, ' FEET/SECOND ( '
          $ , E10.3, ' MILES/DAY )', /)
1697      FORMAT(T39, 'MINIMUM VERTICAL VELOCITY =', E10.3, ' FEET/SECOND ( '
          $ , E10.3, ' MILES/DAY )', /)
1698      FORMAT(T35, 'MAXIMUM HORIZONTAL DISPERSION =', F6.0, ' FEET SQUAR
          $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )', /)
1699      FORMAT(T35, 'MINIMUM HORIZONTAL DISPERSION =', F6.0, ' FEET SQUAR
          $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )', /)
1700      FORMAT(T37, 'MAXIMUM VERTICAL DISPERSION =', F9.3, ' FEET SQUARED
          $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )', /)

```



```

1701 590 FORMAT(T37, 'MINIMUM VERTICAL DISPERSION = ', F9.3, ' FEET SQUARED
      $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )', //)
1702 600 FORMAT(T51, 'MAXIMUM WIDTH = ', F5.0, ' FEET', //)
1703 605 FORMAT(T51, 'MINIMUM WIDTH = ', F5.0, ' FEET', //)
1704 610 FORMAT(T46, 'MAXIMUM DECAY RATE = ', F5.3, ' PER DAY ( ', E10.3,
      $ , ' PER SECOND )', //)
1705 615 FORMAT(T46, 'MINIMUM DECAY RATE = ', F5.3, ' PER DAY ( ', E10.3,
      $ , ' PER SECOND )', //)
1706 620 FORMAT(T42, 'OTHER DEMANDS, MAXIMUM = ', F5.2, //)
1707 630 FORMAT(T26, 'NUMBER OF INITIAL CONCENTRATION VALUES = ', I2, //)
1708 635 FORMAT(T16, 'INITIAL LOCATION OF UPPERMOST PEAK CONCENTRATION = ',
      $ F6.2, ' MILES (SECTION NUMBER ', I2, ')', //)
1709 637 FORMAT(T16, 'INITIAL LOCATION OF LOWERMOST PEAK CONCENTRATION = ',
      $ F6.2, ' MILES (SECTION NUMBER ', I2, ')', //)
1710 640 FORMAT(T56, 'STABILITY CRITERIA', //)
1711 645 FORMAT(T35, 'MAXIMUM ALLOWABLE X INCREMENT = ', F7.0, ' FEET ( ',
      $ F8.5, ' MILES )', //)
1712 650 FORMAT(T35, 'MAXIMUM ALLOWABLE Z INCREMENT = ', F7.0, ' FEET ( ',
      $ F8.5, ' MILES )', //)
1713 655 FORMAT(T32, 'MAXIMUM ALLOWABLE TIME INCREMENT = ', F7.0, ' SECONDS
      $ ( ', F10.3, ' DAYS )', //)
1714 660 FORMAT(T38, 'ACTUAL DELTAT/(DELTAX**2.) = ', E10.3, //)
1715 665 FORMAT(T38, 'ACTUAL DELTAT/(DELTAZ**2.) = ', E10.3, //)
1716 700 FORMAT(T25, 'LOCATIONS OF INITIAL CONCENTRATIONS', //)
1717 705 FORMAT( 'A WASTE SOURCE IS LOCATED AT STATION ', I2, ' (MILE ',
      $ F6.3, ' ). THE CONCENTRATION IS', F10.2, ' PPM.' )
1718 710 FORMAT( 1X, 'AN INITIAL CONCENTRATION IS FOUND AT STATION ', I2,
      1 ' (MILE ', F6.3, ' ).', //, ' THE CONCENTRATIONS AT ', F7.1,
      2 ' FOOT INTERVALS WITH DEPTH ARE')
1719 715 FORMAT(T15, 'C( ', I2, ', I2, ', 1) = ', F9.2, ' PPM.' )
1720 RETURN
1721 END

```

```

1722 C-----
      C SUBROUTINE PRINT2
      C
      C *****
      C ***** SUBROUTINE PRINT2 *****
      C *****
      C
      C THIS SUBROUTINE PRINTS OUT THE TIME AND THE CONCENTRATION
      C PROFILE ACCORDING TO A FORMAT PREVIOUSLY READ INTO THE PROGRAM.
      C THIS SUBROUTINE CAN BE CHANGED ACCORDING TO THE NEEDS OF THE USER.
      C ***WARNING--DO NOT CHANGE ANY VARIABLE OCCURRING IN A COMMON BLOCK.
      C
      C-----
      C
      C INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
      C REAL KD,KDMAX,KDMIN,KDAYS,LOWER
      C COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      C W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
      C DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      C COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      C KDMAX,KDMIN,ITER,ITER1,ITER2,DELTI,DELT2,TNEXT,
      C TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
      C VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAZ,DELTAZ,
      C DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      C MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      C COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
      C VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
      C G(51),D(51),V(51)
      C DIMENSION NZREV(20)
      C DIMENSION R(51,21)

```

```

1730 DAYS = TIME/86400.
1731 HOURS=DAYS*24.
1732 WRITE(6,FMT1) HCURS,DAYS
1733 DO 100 IZ = 1, NZ
1734 NZREV(IZ)=IFIX(DELTAZ*(IZ-2))
1735 100 CONTINUE
1736 WRITE(6,FMT2) (NZREV(I),I=2,NZM1)
C
C-----
C      ALLOW FOR A BACKGROUND CONCENTRATION OF 10.0
C-----
1737 DO 150 X=1,NX
1738 DO 150 Z=1,NZ
1739 R(X,Z)=C(X,Z,T)+10.0
1740 150 CONTINUE
1741 XFIRST=XSTART-(NX-1)*DELTAZ/5280.
1742 DO 200 X=1,NX,XPRINT
1743 M=NX+1-X
1744 XMILE = XFIRST + ((X-1)*DELTAZ*XPRINT)/5280.
1745 WRITE(6,FMT3) XMILE, (R(M,IZ),IZ=1,NZ)
1746 200 CONTINUE
1747 RETURN
1748 END

```

```

1749 SUBROUTINE ERRORI(*)
C-----
C
C *****
C ***** SUBROUTINE ERRORI *****
C *****
C
C
C THIS SUBROUTINE CORRECTS SOME OF THE MOST COMMON ERRORS
C IN THE INPUT DATA AND PRINTS OUT APPROPRIATE ERROR MESSAGES.
C
C-----
1750 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CCOUNT,OPTION
1751 REAL KD,KD MAX,KDMIN,KDAYS,LOWER
1752 COMMON/ARRAYS/ C(51,21,2),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1753 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
KD MAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELT AZ,
DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
MSU,MSL,TIME,NXMI,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
1754 COMMON/IMPLCT/EXNEXT(51,21),EZNEXT(51,21),CSTAR(51,21),
VXNEXT(51,21),VZNEXT(51,21),A(51),B(51),
G(51),D(51),V(51)
1755 IF(OPTION.GT.7) GO TO 180
1756 IF(OPTION.EQ.7) GO TO 140
1757 IF(OPTION.EQ.6) GO TO 120
1758 GO TO 200

```

```

1759 120 CONTINUE
1760 OPTION=2
1761 PRINT 130
1762 130 FORMAT(////, ' ****OPTION IS SET EQUAL TO 2. OPTION WAS EQUAL TO
      $6 ****,/,1H1)
1763 IPAGE = 0
1764 GO TO 200
1765 140 CONTINUE
1766 OPTION=4
1767 PRINT 150
1768 150 FORMAT(////, ' **** OPTION IS SET EQUAL TO 4. OPTION WAS EQUAL TO
      $ 7 ****,/,1H1)
1769 IPAGE=0
1770 GO TO 200
1771 180 CONTINUE
1772 PRINT 181
1773 181 FORMAT(////, ' **** PROGRAM TERMINATED BECAUSE OPTION WAS GREATER
      $ THAN 7 ****,/,1H1)
      RETURN
1774
1775 200 IF(LTYPE.LT.3) GO TO 300
1776 PRINT 201
1777 201 FORMAT(////, ' **** PROGRAM TERMINATED BECAUSE LTYPE IS GREATER T
      $HAN OR EQUAL TO 3.,/, ' IF LTYPE EQUALS 3, THE USER MUST SUPPLY
      $A SUBROUTINE FOR THE LCADING CONDITIONS.,/,1H1)
      RETURN
1778
1779 300 CONTINUE
1780 RETURN
1781 END

```

Input Data for MASSTRANS-II

See Input Data for MASSTRANS-I.

COMPUTER PROGRAM FOR
OXTRANS-I

Object Code = 66,312 bytes
Array Area = 58,372 bytes
Total = 124,684 bytes

OXTRANS-I PERFORMS TWO BASIC CALCULATIONS DURING EACH TIME STEP. THE FIRST CALCULATION IS TO DETERMINE THE PROFILE FOR THE PRIMARY POLLUTANT, SUCH AS BOD, BY AN EXPLICIT FINITE DIFFERENCE APPROXIMATION TO THE MASS TRANSPORT EQUATION. THE SECOND CALCULATION USES INFORMATION FROM THE FIRST CALCULATION TO DETERMINE THE VALUES FOR THE DISSOLVED GAS. THIS PROGRAM HAS OPTIONS FOR ANAEROBIC CONDITIONS AND FOR MECHANICAL REAERATION. DURING A CALCULATION FOR BOD AND DO, THE PROGRAM AUTOMATICALLY SWITCHES TO ANAEROBIC DECAY RATES WHEREVER THE CONCENTRATION OF OXYGEN IS ZERO. REAERATION TERMS CAN BE APPLIED TO ANY POINT ON THE GRID AND, THUS, THE EFFECTS OF MECHANICAL AERATION AT ANY POINT OR POINTS CAN BE INVESTIGATED. AT THE USERS OPTION, THE PROFILES FOR THE PRIMARY POLLUTANTS ALONE CAN BE CALCULATED, AND THE CALCULATIONS FOR THE SECONDARY PARAMETERS CAN BE BY-PASSED.

THIS PROGRAM WAS DEVELOPED TO ANALYZE ESTUARIES WHOSE CHARACTERISTICS DO NOT VARY SIGNIFICANTLY WITH WIDTH. THE WIDTH OF THE ESTUARY MAY BE VARIED THROUGHOUT BUT THE CONCENTRATIONS OF DISSOLVED MATERIALS AT EACH CROSS SECTION ARE CONSIDERED TO BE UNCHANGING IN THE LATERAL DIRECTION. ALL PHYSICAL AND HYDRODYNAMIC CHARACTERISTICS MAY VARY WITH TIME IN THE LONGITUDINAL AND VERTICAL DIRECTIONS. THE PROGRAM CAN BE APPLIED WITH EQUAL EASE TO THE TWO HORIZONTAL DIRECTIONS, ALLOWING DEPTH TO VARY RATHER THAN WIDTH.

CONCENTRATION PROFILES CAN BE CALCULATED BY THIS PROGRAM FOR CONTINUOUS OR INSTANTANEOUS RELEASES. INPUT DATA MAY INCLUDE GRID DIMENSIONS, DISTANCE INCREMENTS, TIME INCREMENTS, WIDTHS, LOADING PARAMETERS, VELOCITIES, DISPERSION COEFFICIENTS, DECAY RATES, BENTHAL DEMANDS, REAERATION COEFFICIENTS, AEROBIC DECAY RATES, ANAEROBIC DECAY RATES, AND OTHER OXYGEN DEMANDS OR SOURCES.

C THIS PROGRAM WAS DEVELOPED PRIMARILY TO ANALYZE PARTIALLY
C STRATIFIED ESTUARIES WHICH HAVE BEEN DREDGED OUT TO A FAIRLY
C CONSTANT DEPTH AT THE CENTERLINE OF THE CHANNEL---THESE ESTUARIES
C ARE COMMON IN THE GULF COAST REGION. APPLICATION OF THIS PROGRAM
C TO PARTIALLY STRATIFIED ESTUARIES WITH VARIABLE DEPTHS WOULD
C REQUIRE MODERATE REVISIONS TO THE PROGRAM AND WOULD MAKE THE
C PROGRAM ESTUARY-DEPENDENT.
C

C THIS COMPUTER PROGRAM CAN ALSO BE APPLIED TO ESTUARIES WHICH
C ARE WELL-MIXED IN THE VERTICAL DIRECTION. THIS OPTION ALLOWS FOR
C VARYING WIDTH OR VARYING DEPTH AND USES MOST OF THE ROUTINES
C AVAILABLE TO THE TWO-DIMENSIONAL ANALYSIS.
C

C QUESTIONS REGARDING THIS PROGRAM MAY BE REFERRED TO
C JONATHAN YOUNG AT HYDROSCIENCE, INC, 363 OLD HOOK ROAD,
C WESTWOOD, NEW JERSEY 07675 PHCNE 201 / 666-2600
C

C C LTYPE = TYPE CF LOADING (INSTANTANEOUS OR CONTINUOUS)
 C C 1 MEANS CONSTANT CONCENTRATION AT OUTFALL
 C C (THIS CAN BE USED FOR STEADY-STATE PROFILES)
 C C 2 MEANS AN INSTANTANEOUS RELEASE
 C C 3 MEANS CONTINUOUS LOADING
 C C OPTION = OPTION FOR BOUNDARY CONDITIONS
 C C 1 MEANS NO TRANSFER ACROSS Z BOUNDARIES AND
 C C EXPONENTIAL EXTRAPOLATION IN X DIRECTION
 C C 2 MEANS NO TRANSFER ACROSS Z BOUNDARIES AND
 C C CONSTANT SLOPE EXTRAPOLATION IN X DIRECTION
 C C 3 MEANS A SYMMETRICAL DISTRIBUTION WITH
 C C EXPONENTIAL EXTRAPOLATION
 C C 4 MEANS A CONSTANT SLOPE EXTRAPOLATION IN
 C C BOTH DIRECTIONS
 C C 5 MEANS A SPECIAL CASE DEFINED BY THE USER
 C C 6 MEANS NO TRANSFER ACROSS Z BOUNDARIES AND
 C C EXTRAPOLATION IN X DIRECTION BY INVERTED
 C C DIFFERENCES (ACFI)
 C C 7 MEANS EXTRAPOLATION IN BOTH DIRECTIONS BY
 C C INVERTED DIFFERENCES (ACFI)
 C C VX(X,Z) = HORIZONTAL VELOCITY (FT/SEC)
 C C EX(X,Z) = HORIZONTAL DISPERSION (FT**2/SEC)
 C C W(X) = WIDTH (FEET) FOR THE TWO DIMENSIONAL CASE OR
 C C CROSS SECTIONAL AREA (FT**2) FOR THE ONE
 C C DIMENSIONAL CASE
 C C KDAY(X) = DECAY RATE (PER DAY)
 C C DEMAND(X,Z) = OTHER SOURCES AND SINKS (CONCENTRATION UNITS)
 C C INSECT(I) = SECTIONS INTO WHICH ARE ADDED LOADS
 C C OR CONCENTRATIONS
 C C CONCIN(I,J) = INITIAL CONCENTRATIONS ASSOCIATED WITH INSECT(I)

```

C *****NOTE---VALUES FOR VZ AND EZ ARE TO BE INCLUDED
C WITH THE DATA ONLY IF NZ IS GREATER THAN 1
C VZ(X,Z) = VERTICAL VELOCITIES ( FT/SEC )
C EZ(X,Z) = VERTICAL DISPERSION ( FT**2/SEC )
C
C FMT1(I) = FORMAT FOR HEADING FOR PRIMARY POLLUTANT PROFILES
C FMT2(I) = FORMAT FOR DEPTH NOTATION FOR PRIMARY POLLUTANTS
C FMT3(I) = FORMAT FOR PRIMARY POLLUTANT PROFILES
C
C *****NOTE---THE FOLLOWING VALUES ARE TO BE INCLUDED
C IN THE INPUT DATA ONLY IF SECOND = .TRUE.
C FMT4(I) = FORMAT FOR HEADING FOR SECONDARY PARAMETERS
C FMT5(I) = FORMAT FOR DEPTH NOTATION FOR SECONDARY PROFILES
C FMT6(I) = FORMAT FOR SECONDARY PARAMETER PROFILES
C RDAYS(X,Z) = REAERATION COEFFICIENTS ( /DAY )
C AKDAYS(X) = ANAEROBIC DECAY RATE ( /DAY )
C OXSAT(X) = OXYGEN SATURATION VALUES (CONCENTRATION UNITS )
C OXOUT(X,Z) = OXYGEN SINKS (CONCENTRATION UNITS )
C *****NOTE---INITIAL CONCENTRATIONS MAY ALSO BE INPUT
C THROUGH THE BLOCK DATA SUBPROGRAM
C
C -----
C

```



```

1977 LOGICAL SECOND
1978 INTEGER X,Z,T,TNEXT,TPRINT,KDAYS,LOWER
1979 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
1980 COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
1981 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      MSU,MSL,TIME,NXMI,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTICN
1982 COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
      AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
      SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN
1983 10 CALL DATA(&I000)
1984 CALL STABLE
1985 CALL PRINT1
1986 CALL PRINT2
1987 T=3
1988 PRINT 15
1989 15 FORMAT(IH1)
1990 CALL PRINT2
1991 CALL ERROR(&I000)
1992 25 IF(NZ.EQ.1) GC TO 310
1993 CALL TMODEX
1994 GO TO 315
1995 310 CALL ONEDEX
1996 315 IF(ITER2.EQ.0) GC TO 10

```

```
1997  
1998  
1999  
2000  
2001  
2002  
2003  
2004  
2005  
2006  
  
ITER=ITER2  
ITER2=0  
TPRINT=TPRINT*(DELTA1/DELTA2+.00001)  
DELTA1=DELTA2*86400.  
CALL STABLE  
IF(ITER.EQ.0) CALL ERROR(81000)  
GO TO 25  
1000 CONTINUE  
STOP  
END
```

```

C-----
C      BLOCK DATA
C
C      *****
C      ***** BLOCK DATA *****
C      *****
C
C      THIS ROUTINE INITIALIZES SELECTED VARIABLES IN THE
C      COMMON BLOCKS.  THIS SUBPROGRAM MAY BE USED TO ESTABLISH
C      INITIAL CONCENTRATION PROFILES IF THE USER DESIRES.
C-----

```

2007

```

C-----
C      LOGICAL SECOND
C      INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CCOUNT,OPTION
C      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
C      COMMON/ARRAYS/  C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
C      W(51),CCNCIN(51,21),INSECT(51),KDAY5(51),KD(51),
C      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
C      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
C      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
C      TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
C      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
C      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
C      MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTICN
C      COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAY5(51,21),OXSAT(51),AK(51),
C      AKDAY5(51),FMT4(40),FMT5(40),FMT6(20),
C      SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN
C-----

```

2008

2009

2010

2011

2012

2013

```

C-----
C ESTABLISH INITIAL CONCENTRATION IF DESIRED
C-----
2014 DATA C / 9*13.,42*0.,9*13.,42*0.,9*13.,42*0.,9*13.,42*0.,
      1 9*13.,42*0.,9*13.,42*0.,9*13.,42*0., 1785*0.0,
      2 9*0., 42*8., 9*0., 42*8., 9*0.,42*8., 9*0., 42*8.,
      3 9*0., 42*8., 9*0., 42*8., 9*0.,42*8., 1785*8. /
2015 DATA VX,VZ,EX,EZ,CONCIN,DEMAND/6426*0.0/
2016 DATA W,KDAYS,KD/153*0.0/
2017 DATA EXMAX,EZMAX,VXMAX,VZMAX,WMAX,KDMAX,DEMAX,TIME/8*0.0/
2018 DATA EXMIN,EZMIN,VXMIN,VZMIN,WMIN,KDMIN,DEMIN/7*1000000./
2019 DATA INSECT/51*0/
2020 DATA IPAGE/1/
2021 DATA T/1/,TNEXT/2/
2022 DATA AKMAX,RMAX,OUTMAX/3*0.0/
2023 DATA AKMIN,RMIN,OUTMIN/3*1000000./
2024 END

```

```

2025 C-----
      SUBROUTINE DATA(*)
C
C *****
C ***** SUBROUTINE DATA *****
C *****
C
C
C      THIS SUBROUTINE READS IN THE APPROPRIATE DATA SET AND ADJUSTS
C      THE GRID SIZE IF NECESSARY.  THE LOCATIONS OF THE UPPERMOST AND
C      LOWERMOST PEAKS ARE DETERMINED, AND THE MINIMUM AND MAXIMUM VALUES
C      FOR THE INPUT PARAMETERS ARE CALCULATED.  ALL OF THE INPUT DATA IS
C      PRINTED OUT UNFORMATTED.  THE FORMAT IS READ IN FOR THE PRINTING
C      OUT OF THE CALCULATED CONCENTRATIONS.
C
C-----
2026 LOGICAL SECOND
2027 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CCOUNT,OPTION
2028 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
2029 COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
2030 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      KDMAX,KDMIN,ITER,ITER1,ITER2,DEL1,DEL2,TNEXT,
      TPRINT,XPRINT,STABLX,STABLZ,STABL,RI,R2,VZMAX,SET,
      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      MSU,MSL,TIME,NXMI,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
      AKDAY(51),FMT4(40),FMT5(40),FMT6(20),
      SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN

```

```

2032 READ(5,12,END=13) SET
2033 12 FORMAT(F10.0)
2034 GO TO 14
2035 13 CONTINUE
2036 RETURN
2037 14 CONTINUE
2038 READ,SECOND
2039 IF(SET.EQ.1.) GO TO 400
C
C-----
C CHOOSE CONCENTRATIONS FROM THE OLD GRID TO PUT INTO THE
C APPROPRIATE PLACES IN THE NEW GRID. THE GRID CAN ONLY BE
C CHANGED IF LTYPE EQUALS 2. OTHERWISE, THE DATA CARD IS READ
C BUT IS NOT IMPLEMENTED.
C-----
2040 READ ,NXSKIP,NZSKIP,NXTAKE,NZTAKE,NXPUT,NZPUT
2041 IF(LTYPE.NE.2) GO TO 400
2042 NXSTOP=(NX-1)/NXSKIP+1
2043 NZSTOP=(NZ-1)/NZSKIP+1
2044 IF(NZ.EQ.1) NZSTOP=1
2045 DO 350 I=1,NXSTOP
2046 X=NXPUT-1+I
2047 II=NXTAKE+NXSKIP*(I-1)
2048 DO 350 J=1,NZSTOP
2049 Z=NZPUT-1+J
2050 JJ=NZTAKE+NZSKIP*(J-1)
2051 C(X,Z,TNEXT)=C(II,JJ,T)
2052 350 CONTINUE
2053 400 CONTINUE

```

```

C-----
C      READ AND PRINT INPUT DATA FOR HCRIZONTAL DIRECTION
C-----
2054 READ,NX,NZ
2055 READ,DELMX,DELTAZ
2056 READ,DELTI,ITER1
2057 READ,DELTI2,ITER2
2058 READ,NC,NS
2059 READ,XSTART,TSTART,XPRINT,TPRINT,NPPAGE
2060 READ,LTYPE,OPTION
2061 PRINT 1001
2062 1001 FORMAT( 1H1,10X,'INPUT DATA',///,
1      1X,'NX,NZ,   DELMX,DELTAZ,  DELTI,ITER1')
PRINT,NX,NZ,DELMX,DELTAZ,DELTI,ITER1
PRINT 1010
1010 FORMAT(//,1X,'SECOND')
PRINT,SECOND
PRINT 1002
1002 FORMAT(//,1X,'DELTI2,ITER2,   NC,NS,   XSTART,TSTART')
PRINT, DELTI2,ITER2,NC,NS,XSTART,TSTART
PRINT 1003
1003 FORMAT(//,1X,'XPRINT,TPRINT,NPPAGE,  LTYPE,OPTION')
PRINT, XPRINT,TPRINT,NPPAGE,LTYPE,OPTION
IF(SET.EQ.1.) GC TO 500
DO 375 Z=1,NZ
DO 375 X=1,NX
C(X,Z,T)=C(X,Z,TNEXT)
C(X,Z,TNEXT)=0.0
375 CONTINUE
500 CONTINUE

```

```

2080 NXM1=NX-1
2081 NXM2=NX-2
2082 NZM1=NZ-1
2083 NZM2=NZ-2
2084 DELTAT=DELT1*86400.
2085 DELTAX=DELMX*5280.
2086 XSF=XSTART*5280.
2087 ITER=ITER1
2088 TIME=TIME+TSTART*86400.
2089 COUNT=(TSTART/DELTAT+0.01)
2090 COUNT=COUNT-(COUNT/TPRINT)*TPRINT
2091 IF(SET.EQ.2) RETURN
2092 READ,((VX(X,Z),X=1,NX),Z=1,NZ)
2093 PRINT 2
2094 2 FORMAT(//, 1X, '((VX(X,Z),X=1,NX),Z=1,NZ)')
2095 PRINT,((VX(X,Z),X=1,NX),Z=1,NZ)
2096 READ,((EX(X,Z),X=1,NX),Z=1,NZ)
2097 PRINT 3
2098 3 FORMAT(//, 1X, '((EX(X,Z),X=1,NX),Z=1,NZ)')
2099 PRINT,((EX(X,Z),X=1,NX),Z=1,NZ)
2100 READ,(W(X),X=1,NX)
2101 PRINT 4
2102 4 FORMAT(//, 1X, '(W(X),X=1,NX)')
2103 PRINT, (W(X),X=1,NX)
2104 READ,(KDAY(X),X=1,NX)
2105 PRINT 5
2106 5 FORMAT(//, 1X, '(KDAY(X),X=1,NX)')
2107 PRINT, (KDAY(X),X=1,NX)

```



```

2108 READ, ((DEMAND(X,Z), X=1, NX), Z=1, NZ)
2109 PRINT 6
2110 6 FORMAT(/, IX, '(DEMAND(X,Z), X=1, NX), Z=1, NZ)')
2111 PRINT, ((DEMAND(X,Z), X=1, NX), Z=1, NZ)
2112 IF(NC.EQ.0) GO TO 24
2113 DO 10 I=1, NC
2114 READ, INSECT(I)
2115 PRINT 7
2116 7 FORMAT(/, IX, 'INSECT(I)')
2117 PRINT, INSECT(I)
2118 READ, (CONCIN(I,J), J=1, NZ)
2119 PRINT 8
2120 8 FORMAT(/, IX, '(CONCIN(I,J), J=1, NZ)')
2121 PRINT, (CONCIN(I,J), J=1, NZ)
2122 10 CONTINUE
2123 DO 15 I=1, NC
2124 DO 15 Z=1, NZ
2125 C(INSECT(I), Z, T)=CONCIN(I, Z)
2126 15 CONTINUE
C
C-----
C DETERMINE LOCATIONS OF UPPERMOST AND LOWERMOST
C PEAK CONCENTRATIONS
C-----
2127 AMAX1=0.0
2128 PEAKU=0.0
2129 AMAX2=0.0
2130 PEAKL=0.0
2131 UPPER=XSF
2132 LOWER=XSF

```

```

2133 MSU=1
2134 MSL=NX
2135 DO 17 I=1,NC
2136 DO 16 J=1,NZ
2137 IF(CONCIN(I,J)).LT.PEAKU) GO TO 16
2138 PEAKU=CONCIN(I,J)
2139 16 CONTINUE
2140 IF(PEAKU.LE.AMAX1) GO TO 20
2141 AMAX1=PEAKU
2142 MSU=INSECT(I)
2143 UPPER=(MSU-1)*DELTA X + XSF
2144 17 CONTINUE
2145 20 CONTINUE
2146 DO 22 II=1,NC
2147 I=NC+1-II
2148 DO 21 J=1,NZ
2149 IF(CONCIN(I,J)).LT.PEAKL) GO TO 21
2150 PEAKL=CONCIN(I,J)
2151 21 CONTINUE
2152 IF(PEAKL.LE.AMAX2) GO TO 24
2153 AMAX2=PEAKL
2154 MSL=INSECT(I)
2155 LOWER=(MSL-1)*DELTA X + XSF
2156 22 CONTINUE
2157 24 CONTINUE
C
C-----
C DETERMINE MAXIMUM AND MINIMUM VALUES FOR INPUT DATA
C-----
DO 25 X=1,NX
2158

```

```

2159 KD(X)=KDAY5(X)/86400.
2160 IF(W(X).GT.WMAX)WMAX=W(X)
2161 IF(W(X).LT.WMIN)WMIN=W(X)
2162 IF(KD(X).GT.KDMAX)KDMAX=KD(X)
2163 IF(KD(X).LT.KDMIN)KDMIN=KD(X)
2164 DO 25 Z=1,NZ
2165 ABSVX=ABS(VX(X,Z))
2166 IF(ABSVX.GT.VXMAX)VXMAX=ABSVX
2167 IF(ABSVX.LT.VXMIN)VXMIN=ABSVX
2168 IF(EX(X,Z).GT.EXMAX)EXMAX=EX(X,Z)
2169 IF(EX(X,Z).LT.EXMIN)EXMIN=EX(X,Z)
2170 IF(DEMAND(X,Z).GT.DEMAX)DEMAX=DEMAND(X,Z)
2171 IF(DEMAND(X,Z).LT.DEMIN)DEMIN=DEMAND(X,Z)
2172 25 CONTINUE
2173 IF(NZ.EQ.1)GO TO 200
C-----
C      READ AND PRINT DATA FOR VERTICAL DIRECTION
C-----
2174 READ,((VZ(X,Z),X=1,NX),Z=1,NZ)
2175 PRINT 30
2176 30 FORMAT(/,/, 1X, ' ((VZ((X,Z),X=1,NX),Z=1,NZ)')
2177 PRINT,((VZ(X,Z),X=1,NX),Z=1,NZ)
2178 READ,((EZ(X,Z),X=1,NX),Z=1,NZ)
2179 PRINT 32
2180 32 FORMAT(/,/, 1X, ' ((EZ(X,Z),X=1,NX),Z=1,NZ)')
2181 PRINT, ((EZ(X,Z),X=1,NX),Z=1,NZ)
C-----
C      DETERMINE MAXIMUM AND MINIMUM VALUES FOR INPUT DATA
C-----

```

```

2182 DO 120 X=1,NX
2183 DO 120 Z=1,NZ
2184 ABSVZ=ABS(VZ(X,Z))
2185 IF(ABSVZ.GT.VZMAX)VZMAX=ABSVZ
2186 IF(ABSVZ.LT.VZMIN)VZMIN=ABSVZ
2187 IF(EZ(X,Z).GT.EZMAX)EZMAX=EZ(X,Z)
2188 IF(EZ(X,Z).LT.EZMIN)EZMIN=EZ(X,Z)
2189 120 CONTINUE
2190 200 CONTINUE

```

C
C
C
C

 READ AND PRINT FORMATS FOR OUTPUT

```

2191 READ 205, (FMT1(I), I=1,40)
2192 PRINT 202
2193 202 FORMAT(/,1X, 'FORMAT FOR OUTPUT')
2194 PRINT 205, (FMT1(I), I=1,40)
2195 205 FORMAT (20A4,/,20A4)
2196 READ 205,(FMT2(I),I=1,40)
2197 PRINT 205, (FMT2(I), I=1,40)
2198 READ 207,(FMT3(I), I=1,20)
2199 PRINT 207, (FMT3(I), I=1,20)
2200 207 FORMAT(20A4)
2201 IF(.NOT.SECOND) GO TO 250
2202 READ 205, (FMT4(I),I=1,40)
2203 PRINT 205, (FMT4(I),I=1,40)
2204 READ 205, (FMT5(I),I=1,40)
2205 PRINT 205, (FMT5(I),I=1,40)
2206 READ 207, (FMT6(I),I=1,20)

```

```

2207 PRINT 207, (FMT6(I),I=1,20)
2208 READ, ((RDAYS(X,Z),X=1,NX),Z=1,NZ)
2209 PRINT 210
2210 FORMAT(//,1X,*((RDAYS(X,Z),X=1,NX),Z=1,NZ)* )
2211 PRINT, ((RCAYS(X,Z),X=1,NX),Z=1,NZ)
2212 READ, (AKDAYS(X),X=1,NX)
2213 PRINT 215
2214 FORMAT(//,1X,*(AKDAYS(X),X=1,NX)* )
2215 PRINT, (AKDAYS(X),X=1,NX)
2216 READ, (OXSAT(X),X=1,NX)
2217 PRINT 220
2218 FORMAT(//,1X,*(OXSAT(X),X=1,NX)* )
2219 PRINT, (OXSAT(X),X=1,NX)
2220 READ, ((OXOUT(X,Z),X=1,NX),Z=1,NZ)
2221 PRINT 222
2222 FORMAT(//,1X,*((OXOUT(X,Z),X=1,NX),Z=1,NZ)* )
2223 PRINT, ((OXOUT(X,Z),X=1,NX),Z=1,NZ)
2224 DO 225 X=1,NX
2225 AK(X)=AKDAYS(X)/86400.
2226 IF(AK(X).GT.AKMAX) AKMAX=AK(X)
2227 IF(AK(X).LT.AKMIN) AKMIN=AK(X)
2228 DO 225 Z=1,NZ
2229 R(X,Z)=RDAYS(X,Z)/86400.
2230 IF(R(X,Z).GT.RMAX) RMAX=R(X,Z)
2231 IF(R(X,Z).LT.RMIN) RMIN=R(X,Z)
2232 IF(OXOUT(X,Z).GT.OUTMAX) OUTMAX=OXOUT(X,Z)
2233 IF(OXOUT(X,Z).LT.OUTMIN) OUTMIN=OXOUT(X,Z)
2234 CONTINUE
2235 CONTINUE
2236 PRINT 259
2237 FORMAT(1H1)
2238 RETURN
2239 END

```

```

C-----
2240 SUBROUTINE STABLE
C
C *****
C ***** SUBROUTINE STABLE *****
C *****
C *****
C
C THE RELATIONSHIP BETWEEN DISTANCE INCREMENTS, TIME INCREMENTS,
C DISPERSION COEFFICIENTS, AND VELOCITIES IS NEEDED TO DETERMINE
C THE STABILITY OF THE EXPLICIT FINITE-DIFFERENCE PROCEDURE. THIS
C SUBROUTINE CALCULATES THE MAXIMUM ALLOWABLE INCREMENTS FOR TIME
C AND DISTANCE. THE PROGRAM IS TERMINATED IF THE INPUT PARAMETERS
C VIOLATE THIS CRITERIA.
C-----

```

```

2241 LOGICAL SECOND
2242 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CCOUNT,OPTION
2243 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
2244 COMMON/ARRAYS/ C(51,21,4),VX(51,21),INSECT(51),KDAYS(51),KD(51),
1 W(51),CONCIN(51,21),FMT1(40),FMT2(40),FMT3(20)
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
3 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABTZ,STABLT,R1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
C COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
1 AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
2 SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN

```

2247 STABLX = DELTAX
 2248 STABLZ = DELTAZ
 2249 STABLT = DELTAT

C
 C
 C
 C

 CALCULATE STABILITY CRITERIA FOR ONE DIMENSIONAL CASE

2250 IF(NZ.NE.1) GO TO 500
 2251 IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
 2252 IF(STABLX.LT.DELTAX) ITER=0
 2253 TERM=2.*EXMAX+DELTAX**2.*KDMAX
 2254 IF(TERM.NE.0.0) STABLT=(DELTAX**2.)/TERM
 2255 IF(STABLT.LT.DELTAT) ITER=0
 2256 R1=DELTAT/(DELTAX**2.)
 2257 R2=0.
 2258 GO TO 800
 2259 500 CONTINUE

C
 C
 C
 C

 CALCULATE STABILITY CRITERIA FOR TWO DIMENSIONAL CASE

2260 IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
 2261 IF(STABLX.LT.DELTAX) ITER=0
 2262 IF(EZMIN.GT.0.0.AND.VZMAX.GT.0.0) STABLZ=2.*EZMIN/VZMAX
 2263 IF(STABLZ.LT.DELTAZ) ITER=0
 2264 TERM=2.*(EXMAX*DELTAX**2.+EZMAX*DELTAX**2.)+KDMAX*(DELTAX**2)*
 \$ (DELTAZ**2)
 2265 IF(TERM.NE.0.0) STABLT=(DELTAX**2.)*(DELTAX**2.)/TERM
 2266 IF(STABLT.LT.DELTAT) ITER=0
 2267 R1=DELTAT/(DELTAX**2.)
 2268 R2=DELTAT/(DELTAZ**2.)
 2269 800 CONTINUE
 2270 RETURN
 2271 END

```

2272 C-----
      SUBROUTINE ONEDEX
      C
      C *****
      C ***** SUBROUTINE ONEDEX *****
      C *****
      C
      C THIS SUBROUTINE CALCULATES THE ONE DIMENSIONAL CONCENTRATION
      C PROFILE FOR THE ASSIGNED GRID AND THE ASSIGNED NUMBER OF ITERA-
      C TIONS. A FINITE-DIFFERENCE, EXPLICIT SCHEME IS USED TO SOLVE THE
      C PARTIAL DIFFERENTIAL EQUATION FOR MASS TRANSPORT.
      C
      C-----
      LOGICAL SECOND
      INTEGER X,Z,T,TNEXT,TPRINT,KDAYS,XPRINT,COUNT,OPTION
      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
      COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      1 W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
      2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      2 TPRINT,XPRINT,STABLX,STABLZ,STABL,R1,R2,VZMAX,SET,
      3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
      4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
      1 AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
      2 SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN
      Z=1
      NXM1=NX-1
2273
2274
2275
2276
2277
2278
2279
2280

```



```

C
C-----
C          CALCULATE TERMS WHICH ARE USED REPEATEDLY
C-----
2281      TWOXSQ=2.*DELTA**2.
2282      TWOX=2.*DELTA
C
C-----
C          CALCULATE NEW CONCENTRATION AT EACH POINT
C-----
2283      DO 300 IT=1,ITER
2284      TIME=TIME+DELTA
2285      T=1
2286      TNEXT=2
2287      10 DO 200 X=2,NXMI
2288          DECAY=KD(X)
2289          IF(.NOT.SECOND) GO TO 15
2290          IF(C(X,Z,3).LE.0.) DECAY=AK(X)
2291          15 CONTINUE
2292          TERM1A=W(X)*VX(X,Z)/TWCX
2293          TERM1R=(W(X)*EX(X,Z)+W(X-1)*EX(X-1,Z))/TWOXSQ
2294          TERM2B = (W(X+1)*EX(X+1,Z) + W(X)*EX(X,Z)) / TWOXSQ
                + (W(X)*EX(X,Z) + W(X-1)*EX(X-1,Z)) / TWOXSQ
2295          TERM3A=(-W(X)*VX(X,Z))/TWOX
2296          TERM3B=(W(X+1)*EX(X+1,Z)+W(X)*EX(X,Z))/TWOXSQ
2297          TERM1=C(X-1,Z,T)*(TERM1A+TERM1B)
2298          TERM2=-C(X,Z,T)*TERM2B
2299          TERM3=C(X+1,Z,T)*(TERM3A+TERM3B)
2300          IF(T.EQ.3) GO TO 50

```

```

2301 C(X,Z,2)=C(X,Z,1)+(DELTA/W(X))*((TERM1+TERM2+TERM3)
      $ -DEMAND(X,Z)-DECAY*DELTA*C(X,Z,1)
2302 GO TO 75
2303 50 CONTINUE
2304 C(X,Z,4)= -DECAY*DELTA*C(X,Z,4)-OXOUT(X,Z)
      $ +R(X,Z)*((OXSAT(X)-C(X,Z,3))*DELTA
2305 C(X,Z,4)=C(X,Z,3)+(DELTA/W(X))*((TERM1+TERM2+TERM3) +C(X,Z,4)
2306 75 IF(C(X,Z,TNEXT).LT.0.0) C(X,Z,TNEXT)=0.0
2307 200 CONTINUE
C
C-----
C          CALCULATE BOUNDARY VALUES
C-----
2308 CALL BOUND
2309 Z=1
2310 DO 250 X=1,NX
2311 IF(TNEXT.EQ.2) C(X,Z,4)=C(X,Z,1)
2312 C(X,Z,T)=C(X,Z,TNEXT)
2313 IF(C(X,Z,3).GT.OXSAT(X)) C(X,Z,3) = OXSAT(X)
2314 C(X,Z,TNEXT)=0.0
2315 250 CONTINUE
2316 IF(T.EQ.1) COUNT=COUNT+1
2317 IF(COUNT.GE.TPRINT) GO TO 292
2318 GO TO 298
C
C-----
C          PRINT CONCENTRATIONS AT APPROPRIATE INTERVALS
C-----
2319 292 CONTINUE
2320 IF(T.EQ.3.OR.(.NOT.SECOND)) COUNT=0

```

```
2321 IPAGE=IPAGE+1
2322 IF(IPAGE.GE.NPPAGE) GO TO 295
2323 GO TO 297
2324 295 PRINT 296
2325 296 FORMAT(IH1)
2326 IPAGE = 0
2327 297 CALL PRINT 2
2328 298 CONTINUE
2329 IF((.NOT.SECOND).OR.(T.EQ.3)) GO TO 300
2330 T=3
2331 TNEXT=4
2332 GO TO 10
2333 300 CONTINUE
2334 RETURN
2335 END
```

```

2336 C-----
      C SUBROUTINE TWODEX
      C
      C *****
      C ***** SUBROUTINE TWODEX *****
      C *****
      C
      C THIS SUBROUTINE CALCULATES THE TWO-DIMENSIONAL CONCENTRATION
      C PROFILE FOR THE ASSIGNED GRID AND THE ASSIGNED NUMBER OF ITERA-
      C TIONS. A FINITE-DIFFERENCE, EXPLICIT SCHEME IS USED TO SOLVE THE
      C PARTIAL DIFFERENTIAL EQUATION FOR MASS TRANSPORT.
      C
      C-----
      C LOGICAL SECOND
      C INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
      C REAL KD,KDMAX,KDMIN,KDAYS,LOWER
      C COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
      C 1 W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
      C 2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      C COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
      C 1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
      C 2 TPRINT,XPRINT,STABLX,STABLY,STABLT,STABLR,R1,R2,VZMAX,SET,
      C 3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAY,
      C 4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
      C 5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      C COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
      C 1 AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
      C 2 SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN
      C
      C NXM1=NX-1
      C NZM1=NZ-1
      C NZ1=2

```

```

C-----
C          CALCULATE TERMS WHICH ARE USED REPEATEDLY
C-----
2346      TWOXSQ=2.*DELTAX**2.
2347      TWOX=2.*DELTAX
2348      TWOZSQ=2.*DELTAZ**2.
2349      TWOZ=2.*DELTAZ
C-----
C          CALCULATE NEW CONCENTRATION AT EACH POINT
C-----
2350      DO 300 IT=1,ITER
2351      TIME=TIME+DELTAT
2352      T=1
2353      TNEXT=2
2354      10 DO 200 X=2,NXM1
2355         DO 100 Z=NZ1,NZM1
2356         DECAY=KD(X)
2357         IF(.NOT.SECOND) GO TO 15
2358         IF(C(X,Z,3).LE.0.) DECAY=AK(X)
2359         15 CONTINUE
2360         TERM1A=W(X)*VX(X,Z)/TWCX
2361         TERM1B=(W(X)*EX(X,Z)+W(X-1))*EX(X-1,Z)/TWOXSQ
2362         TERM2B = (W(X+1)*EX(X+1,Z) + W(X)*EX(X,Z)) / TWOXSQ
                $      +(W(X)*EX(X,Z) + W(X-1)*EX(X-1,Z)) / TWOXSQ
2363         TERM3A=(-W(X)*VX(X,Z))/TWOX
2364         TERM3B=(W(X+1)*EX(X+1,Z)+W(X)*EX(X,Z))/TWOXSQ

```

```

2365 TERM4A=W(X)*VZ(X,Z)/TMCZ
2366 TERM4B=(W(X)*EZ(X,Z)+W(X)*EZ(X,Z-1))/TWOZSQ
2367 TERM5A=(-W(X)*VZ(X,Z))/TWOZ
2368 TERM5B=(W(X)*EZ(X,Z+1)+W(X)*EZ(X,Z))/TWOZSQ
2369 TERM6B = (W(X) * EZ(X,Z+1) + W(X)*EZ(X,Z))/ TWOZSQ
          + (W(X)*EZ(X,Z) + W(X)*EZ(X,Z-1)) / TWOZSQ
2370 TERM1=C(X-1,Z,T)*(TERM1A+TERM1B)
2371 TERM2=-C(X,Z,T)*(TERM2B+TERM6B)
2372 TERM3=C(X+1,Z,T)*(TERM3A+TERM3B)
2373 TERM4=C(X,Z-1,T)*(TERM4A+TERM4B)
2374 TERM5=C(X,Z+1,T)*(TERM5A+TERM5B)
2375 IF(T.EQ.3) GO TO 50
2376 C(X,Z,2)=C(X,Z,1)+(DELTA/W(X))*(TERM1+TERM2+TERM3+TERM4+TERM5)
          -DEMAND(X,Z)-DECAY*DELTA*C(X,Z,1)
2377 GO TO 75
2378 50 CONTINUE
2379 C(X,Z,4)=
          +R(X,Z)*(OXSAT(X)-C(X,Z,3))*DELTA
          -DECAY*DELTA*C(X,Z,4)-OXOUT(X,Z)
2380 C(X,Z,4)=C(X,Z,3)+(DELTA/W(X))*(TERM1+TERM2+TERM3+TERM4+TERM5)
          +C(X,Z,4)
2381 75 IF(C(X,Z,TNEXT).LT.0.0) C(X,Z,TNEXT)=0.0
2382 100 CONTINUE
2383 200 CONTINUE
C
C-----
C          CALCULATE BOUNDARY VALUES
C-----
2384 CALL BOUND
2385 DO 250 Z=1,NZ
2386 DO 250 X=1,NX

```

```

2387 IF(TNEXT.EQ.2) C(X,Z,4)=C(X,Z,1)
2388 C(X,Z,T)=C(X,Z,TNEXT)
2389 IF(C(X,Z,3).GT.OXSAT(X)) C(X,Z,3) = CXSAT(X)
2390 C(X,Z,TNEXT)=0.0
2391 250 CONTINUE
2392 IF(T.EQ.1) COUNT=COUNT+1
2393 IF(COUNT.GE.TPRINT) GO TO 292
2394 GO TO 298

```

C

C

 C PRINT CONCENTRATIONS AT APPROPRIATE INTERVALS
 C-----

```

2395 292 CONTINUE
2396 IF(T.EQ.3.OR.(.NOT.SECND)) COUNT=0
2397 IPAGE=IPAGE+1
2398 IF(IPAGE.GE.NPPAGE) GO TO 295
2399 GO TO 297
2400 295 PRINT 296
2401 296 FORMAT(1H1)
2402 IPAGE = 0
2403 297 CALL PRINT2
2404 298 CONTINUE
2405 IF((.NOT.SECND).OR.(T.EQ.3)) GO TO 300
2406 T=3
2407 TNEXT=4
2408 GO TO 10
2409 300 CONTINUE
2410 RETURN
2411 END

```

```

2412 C-----
      SUBROUTINE BOUND
C
C *****
C ***** SUBROUTINE BOUND *****
C *****
C
C
C      SUBROUTINE BOUND PROVIDES FOR EXTRAPOLATION OF CONCENTRATIONS
C      AT THE BOUNDARIES. THE TYPE OF EXTRAPOLATION DEPENDS ON THE VALUE
C      OF THE VARIABLE OPTION.
C
C-----
      LOGICAL SECOND
      INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
      COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1      W(51),CONCIN(51,21),INSECT(51),KDAY(51),KD(51),
2      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1      KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2      TPRINT,XPRINT,STABLX,STABLY,STABTZ,STABLR1,R2,VZMAX,SET,
3      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAY,
4      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5      MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
1      AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
2      SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN

```



```

2419 IF(NZ.EQ.1) NZM1=1
2420 ZUP=UPPER
2421 ZLOW=LOWER
2422 ZSF=XSF
2423 GO TO (1001,1002,1003,1004,1005,1006,1006), OPTION
C
-----
C      EXPONENTIAL EXTRAPOLATION
C
-----
2424 1003 CONTINUE
2425 IF(NZ.EQ.1) GO TO 1001
2426 DO 140 X=2,NXM1
2427 IF(C(X,2,T+1).LE.0.0) GO TO 140
2428 VZ1=VZ(X,1)
2429 ZZ=-ZUP+DELTAZ+ZSF+VZ1*TSTART*86400.
2430 EZ1=EZ(X,1)
2431 C2=4.*EZ1*TIME
2432 EXPON=(-2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)-2.*DELTAZ*VZ1*TIME)/C2
2433 C(X,1,T+1)=C(X,2,T+1)*EXP(EXPON)
2434 140 CONTINUE
2435 DO 190 X=2,NXM1
2436 IF(C(X,NZM1,T+1).LE.0.0) GO TO 190
2437 VZNZ=VZ(X,NZ)
2438 ZZ=(NZ-2)*DELTAZ-ZLOW+ZSF+VZNZ*TSTART*86400.
2439 EZNZ=EZ(X,NZ)
2440 C2=4.*EZNZ*TIME
2441 EXPON=(-2.*ZZ*DELTAZ-(DELTAZ*DELTAZ)+2.*DELTAZ*VZNZ*TIME)/C2
2442 C(X,NZ,T+1)=C(X,NZM1,T+1)*EXP(EXPON)
2443 190 CONTINUE
2444 1001 CONTINUE
2445 IF(LTYPE.EQ.1.AND.MSU.EQ.1) GO TO 350
2446 DO 340 Z=1,NZ

```

```

2447 IF(C(2,Z,T+1).LE.0.0) GO TO 340
2448 VX1=VX(1,Z)
2449 XX=-UPPER+DELTA+XSF+VX1*TSTART*86400.
2450 EX1=EX(1,Z)
2451 C2=4.*EX1*TIME
2452 EXPON=( 2.*XX*DELTA-(DELTA*DELTA)-2.*DELTA*VX1*TIME)/C2
2453 C(1,Z,T+1)=C(2,Z,T+1)*EXP(EXPON)
2454
2455 340 CONTINUE
2456 350 CONTINUE
2457 IF(LTYPE.EQ.1.AND.MSL.EQ.NZ) GO TO 400
2458 DO 390 Z=1,NZ
2459 IF(C(NX-1,Z,T+1).LE.0.0) GO TO 390
2460 VXNX=VX(NX,Z)
2461 XX=(NX-2)*DELTA-LOWER+XSF+VXNX*TSTART*86400.
2462 EXNX=EX(NX,Z)
2463 C2=4.*EXNX*TIME
2464 EXPON=(-2.*XX*DELTA-(DELTA*DELTA)+2.*DELTA*VXNX*TIME)/C2
2465 C(NX,Z,T+1)=C(NX-1,Z,T+1)*EXP(EXPON)
2466 390 CONTINUE
2467 400 CONTINUE
      GO TO 2000
C-----
C      CONSTANT SLOPE EXTRAPCLATION
C-----
1004 CONTINUE
2468 IF(NZ.EQ.1) GO TO 1002
2469 DO 540 X=2,NXM1
2470 C(X,1,T+1)=2.*C(X,2,T+1)-C(X,3,T+1)
2471 IF(C(X,1,T+1).LT.0.0) C(X,1,T+1)=0.0

```

```

2473 C(X,NZ,T+1)=2.*C(X,NZM1,T+1)-C(X,NZM2,T+1)
2474 IF(C(X,NZ,T+1).LT.0.0) C(X,NZ,T+1)=0.0
2475 540 CONTINUE
2476 1002 CONTINUE
2477 DO 590 Z=1,NZ
2478 C(1,Z,T+1)=2.*C(2,Z,T+1)-C(3,Z,T+1)
2479 IF(C(1,Z,T+1).LT.0.0) C(1,Z,T+1)=0.0
2480 C(NX,Z,T+1)=2.*C(NXM1,Z,T+1)-C(NXM2,Z,T+1)
2481 IF(C(NX,Z,T+1).LT.0.0) C(NX,Z,T+1)=0.0
2482 590 CONTINUE
2483 GO TO 2000
2484 1005 CONTINUE

```

```

C-----
C THIS OPTION ALLOWS THE PROGRAM USER TO SUBSTITUTE HIS OWN
C EXTRAPOLATION ROUTINE
C-----

```

```

2485 ZZ=-40.
2486 EZ1=0.025
2487 VZ1=0.
2488 C2=4.*EZ1*TIME
2489 EXPON=(+2.*ZZ*DELTAZ-(DELTAZ**2.)-2*DELTAZ*VZ1*TIME)/C2
2490 DO 240 X=1,NX
2491 C(X,1,T+1)=C(X,2,T+1)*EXP(EXPON)
2492 C(X,NZ,T+1)=C(X,1,T+1)
2493 240 CONTINUE
2494 GO TO 1001

```

```

C-----
C INVERTED DIFFERENCES EXTRAPOLATION (ACFI)
C-----

```

```

2495 C-----
2496     1006 CALL EXTRAP
2497     GO TO 2000
2498     2000 CONTINUE
2499     IF(NZ.EQ.1) GO TO 2005
2500     IF(OPTION.EQ.3.OR.OPTION.EQ.4.OR.OPTION.EQ.5) GO TO 2005
2501     DO 700 X=1,NX
2502     C(X,1,T+1)=C(X,2,T+1)
2503     C(X,NZ,T+1)=C(X,NZM1,T+1)
2504     700 CONTINUE
2505     2005 CONTINUE
C-----
C
C     IF LTYPE EQUALS 1, SET CONCENTRATIONS AT SOURCE POINTS.
C     IF LTYPE EQUALS 3, THE USER CAN INCLUDE A SPECIAL SUBROUTINE
C     OR SET OF CALCULATIONS IN THIS PART OF THE PROGRAM.
C-----
2505     IF(T.EQ.3) GO TO 290
2506     IF(LTYPE.EQ.1.AND.NS.LE.2) GO TO 275
2507     IF(LTYPE.EQ.1.AND.NS.GT.2) GO TO 281
2508     GO TO 290
2509     275 CONTINUE
2510     DO 280 Z=1,NZ
2511     C(MSU,Z,T+1)=C(MSU,Z,T)
2512     C(MSL,Z,T+1)=C(MSL,Z,T)
2513     280 CONTINUE
2514     GO TO 290
2515     281 CONTINUE
2516     DO 285 I=1,NC
2517     DO 285 Z=1,NZ
2518     C(INSECT(I),Z,T+1)=CONCIN(I,Z)
2519     285 CONTINUE
2520     290 CONTINUE
2521     RETURN
2522     END

```

```

C-----
2523 SUBROUTINE EXTRAP
C
C *****
C ***** SUBROUTINE EXTRAP *****
C *****
C
C THIS SUBROUTINE EXTRAPOLATES THE CONCENTRATION PROFILE BY
C USING A CONTINUED FRACTIONS AND INVERTED DIFFERENCES SCHEME. IT IS
C A MODIFIED VERSION OF THE IBM SCIENTIFIC SUBPROGRAM ACFI.
C
C IF THE EXTRAPOLATED CONCENTRATION IS GREATER THAN THE
C ADJACENT CONCENTRATION OR IF IT IS NEGATIVE, THEN THE BOUNDARY
C CONCENTRATION IS EXTRAPOLATED ACCORDING TO THE PROPORTION
C BETWEEN THE CONCENTRATIONS AT THE TWO ADJACENT INTERNAL POINTS
C (I.E. C1/C2 = C2/C3 ).
C
C-----
2524 LOGICAL SFCND
2525 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
2526 REAL KO,KD,KDMAX,KDMIN,KDAYS,LOWER
2527 COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CCNCIN(51,21),INSECT(51),KDAY(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
2528 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXMI,NXM2,NZMI,RDAYS(51,21),OXSAT(51),AK(51),
2529 AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
1 SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN
2

```

```

2530 DIMENSION ARG(10), VALY(10)
2531 NZ1=2
2532 IF(NZ.EQ.1) NZ1=1
2533 IF(NZ.EQ.1) NZM1=1
2534 IF(MSU.EQ.1.AND.LTYPE.EQ.1) GO TO 228
C
C-----
C      EXTRAPOLATE THE PROFILE IN THE X DIRECTION
C-----
2535 DO 225 Z=NZ1,NZM1
2536 IF(C(2, Z, T+1) . LE. 0.00) GO TO 220
C
C-----
C      CHOOSE THE NUMBER OF POINTS (NDIM) TO BE USED IN THE
C      EXTRAPOLATION.
C-----
2537 NDIM=3
2538 NSTOP=NDIM+1
C
C-----
C      PLACE CONCENTRATIONS IN PROPER ORDER FOR SUBROUTINE ACFI
C-----
2539 DO 210 X=2,NSTOP
2540 ARG(X - 1) = FLOAT(X)
2541 VALY(X-1) = C(X, Z, T+1)
2542 IF(VALY(X-1).LE.0.0) GO TO 205

```

```

2543 GO TO 210
2544 NDIM = X-1
2545 GO TO 211
2546 210 CONTINUE
2547 211 CONTINUE
2548 EPS = VALY(1)/1000.
2549 CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
2550 C(1, Z, T+1)=Y
C
C-----
C IF THE EXTRAPOLATED VALUE IS TOO HIGH OR IS NEGATIVE, THEN USE
C A PROPORTION EQUATION.
C-----
2551 IF(Y.LE.0.0.OR.Y.GT.C(2,Z,T+1)) C(1,Z,T+1)=C(2,Z,T+1)*C(2,Z,T+1)
      $ /C(3,Z,T+1)
2552 GO TO 225
2553 220 C(1, Z, T+1) = 0.0
2554 225 CONTINUE
2555 228 CONTINUE
2556 IF(LTYPE.EQ.1.AND.MSL.EQ.NX) GO TO 251
2557 DO 250 Z=NZ1,NZM1
2558 IF ( C(NX-1, Z, T+1) . LE. 0.00) GO TO 245
2559 NDIM=3
2560 NSTOP=NDIM
2561 DO 235 I=1,NSTOP
2562 IBACK = NX - I
2563 ARG(I) = FLOAT(I+1)
2564 VALY(I) = C( IBACK, Z, T+1)
2565 IF( VALY(I) . LE. 0.00) GO TO 230
2566 GO TO 235

```

```

2567 230 NDIM = I
2568   GO TO 236
2569 235 CONTINUE
2570 236 CONTINUE
2571   EPS = VALY(1)/1000.
2572   CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
2573   C( NX, Z, T+1) = Y
2574   IF(Y.LE.0.0.OR.Y.GT.C(NX-1,Z,T+1))C(NX,Z,T+1)=C(NXMI,Z,T+1)*
$     C(NXMI,Z,T+1)/C(NXM2,Z,T+1)
2575   GO TO 250
2576 245 C(NX, Z, T+1) = 0.0
2577 250 CONTINUE
2578 251 CONTINUE
2579   IF(NZ.EQ.1.OR.OPTION.EC.6) RETURN
C-----
C     EXTRAPOLATE THE PROFILE IN THE Z DIRECTION
C-----
2580 DO 275 X = 1, NX
2581   IF(C(X, 2, T+1) . LE. 0.00) GO TO 270
2582   NDIM=3
2583   NSTOP=NDIM
2584   DO 260 Z = 1, NSTOP
2585     ARG(Z) = FLOAT(Z+1)
2586     VALY(Z) = C(X, Z+1, T+1)
2587     IF( VALY(Z) . LE. 0.00) GO TO 255
2588     GO TO 260
2589   255 NDIM = Z
2590     GO TO 261
2591   260 CONTINUE

```



```

2592
2593 EPS = VALY(1)/1000.
2594 CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
2595 C ( X, 1, T+1) = Y
2596 IF(Y.LE.0.0.OR.Y.GT.C(X,2,T+1))C(X,1,T+1)=C(X,2,T+1)*C(X,2,T+1)
$ /C(X,3,T+1)
2597 IF(C(X,1,T+1).LT.0.0) C(X,1,T+1)=0.0
2598 GO TO 275
2599 270 C(X, 1, T+1) = 0.00
2600 275 CONTINUE
2601 DO 295 X = 1, NX
2602 IF (C(X, NZ-1, T+1) . LE. 0.00) GO TO 292
2603 NDIM=3
2604 NSTOP=NDIM
2605 DO 285 I = 1, NSTOP
2606 ARG(I) = FLOAT(I+1)
2607 IBACK = NZ - I
2608 VALY(I) = C(X, IBACK, T+1)
2609 IF(VALY(I). LE. 0.00) GO TO 280
2610 GO TO 285
2611 280 NDIM = I
2612 GO TO 286
2613 285 CONTINUE
2614 286 CONTINUE
2615 EPS = VALY(1)/ 1000.
2616 CALL ACFI ( 1.0, ARG, VALY, Y, NDIM, EPS, IER, J )
2617 C(X, NZ, T+1) = Y
2618 IF(Y.LE.0.0.OR.Y.GT.C(X,NZ-1,T+1)) C(X,NZ,T+1)=C(X,NZM1,T+1)*
$ C(X,NZM1,T+1)/C(X,NZM2,T+1)
2619 GO TO 295
2620 292 C(X, NZ, T+1)=0.0
2621 295 CONTINUE
2622 RETURN
2623 END

```

```

C-----
C      SUBROUTINE ACFI ( X, ARG, VALY, Y, NDIM, EPS, IER, J )
C
C      *****
C      ***** SUBROUTINE ACFI *****
C      *****
C
C .....
C
C PURPOSE
C TO INTERPOLATE FUNCTION VALUE Y FOR A GIVEN ARGUMENT VALUE
C X USING A GIVEN TABLE (ARG,VAL) OF ARGUMENT AND FUNCTION
C VALUES.
C
C DESCRIPTION OF PARAMETERS
C X      - THE ARGUMENT VALUE SPECIFIED BY INPUT.
C ARG    - THE INPUT VECTOR (DIMENSION NDIM) OF ARGUMENT
C         VALUES OF THE TABLE (POSSIBLY DESTROYED).
C VAL    - THE INPUT VECTOR (DIMENSION NDIM) OF FUNCTION
C         VALUES OF THE TABLE (DESTROYED).
C Y      - THE RESULTING INTERPOLATED FUNCTION VALUE.
C NDIM   - AN INPUT VALUE WHICH SPECIFIES THE NUMBER OF
C         POINTS IN TABLE (ARG,VAL).
C EPS    - AN INPUT CONSTANT WHICH IS USED AS UPPER BOUND
C         FOR THE ABSOLUTE ERROR.
C IER    - A RESULTING ERROR PARAMETER.
C

```

2624


```

C
C
2638 START INTERPOLATION LOOP
2639 DO 16 I=2,NDIM
2640 II=0
2641 P1=P2
2642 P2=P3
2643 Q1=Q2
2644 Q2=Q3
2645 Z=Y
2646 DELT1=DELT2
JEND=I-1
C
C
2647 COMPUTATION OF INVERTEC DIFFERENCES
2648 3 AUX=VAL(I)
2649 DO 10 J=1,JEND
2650 H=VAL(I)-VAL(J)
2651 IF (ABS(H)-1.E-6*ABS(VAL(I)))4,4,9
2652 IF (ARG(I)-ARG(J))5,17,5
5 IF (J-JEND)8,6,6
C
2653 INTERCHANGE ROW I WITH ROW I+II
2654 II=II+1
2655 III=I+II
2656 IF (III-NDIM)7,7,19
2657 VAL(I)=VAL(III)
2658 VAL(III)=AUX
2659 AUX=ARG(I)
2660 ARG(I)=ARG(III)
2661 ARG(III)=AUX
GOTO 3
C

```

```

C      COMPUTATION OF VAL(I) IN CASE VAL(I)=VAL(J) AND J LESS THAN I-1
2662  8 VAL(I)=1.E75
2663  GOTO 10
C
C      COMPUTATION OF VAL(I) IN CASE VAL(I) NOT EQUAL TO VAL(J)
2664  9 VAL(I)=(ARG(I)-ARG(J))/H
2665  10 CONTINUE
C      INVERTED DIFFERENCES ARE COMPUTED
C
C      COMPUTATION OF NEW Y
2666  P3=VAL(I)*P2+(X-ARG(I-1))*P1
2667  Q3=VAL(I)*Q2+(X-ARG(I-1))*Q1
2668  IF(Q3)11,12,11
2669  11 Y=P3/Q3
2670  GOTO 13
2671  12 Y=1.E75
2672  13 DELT2=ABS(Z-Y)
2673  IF(DELT2-EPS)19,19,14
2674  14 IF(I-8)16,15,15
2675  15 IF(DELT2-DELT1)16,18,18
2676  16 CONTINUE
C      END OF INTERPOLATION LCOP
C
C      RETURN
2677
C      THERE ARE TWO IDENTICAL ARGUMENT VALUES IN VECTOR ARG
2678  17 IER=3

```

```
2679          RETURN  
C  
C      TEST VALUE DELT2 STARTS OSCILLATING  
18  Y=Z  
    IER=1  
    RETURN  
C  
C      THERE IS SATISFACTORY ACCURACY WITHIN NDIM-1 STEPS  
19  IER=0  
20  RETURN  
    END  
2683  
2684  
2685
```

```

2686 C-----
      SUBROUTINE PRINT1
C
C *****
C ***** SUBROUTINE PRINT1 *****
C *****
C
C
C      THIS SUBROUTINE CALCULATES THE CONVERSION VALUES FOR MANY OF
C      THE INPUT PARAMETERS AND PRINTS OUT A SUMMARY OF THE INPUT DATA
C      AND STABILITY CRITERIA.
C-----
      LOGICAL SECOND
      INTEGER X,Z,I,TNEXT,IPRINT,XPRINT,COUNT,OPTION
      REAL KD,KDMAX,KDMIN,KDAYS,LOWER
      COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1      W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
2      DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
      COMMON/NAMES/X,Z,I,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1      KDMAX,KDMIN,ITER,ITER1,ITER2,DELTI,DELTY,TNEXT,
2      TPRINT,XPRINT,STABLX,STABLZ,STABL,R1,R2,VZMAX,SET,
3      VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAY,
4      DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5      MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
      COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
1      AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
2      SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN
      UNITS1 = 86400./5280.
      UNITS2 = 86400./15280.*5280.)
      DELDAY = DELTAT/86400.

```



```

2696 SECS1=DELT1*86400.
2697 SECS2=DELT2*86400.
2698 DELMX=DELTAX/5280.
2699 DELMZ=DELT AZ/5280.
2700 TOTALX=(NX-1)*DELTAX
2701 TOTMX=TOTALX/5280.
2702 TOTALZ=(NZ-1)*DELT AZ
2703 TOTMZ=TOTALZ/5280.
2704 VX1=VXMAX*UNITS1
2705 VX2=VXMIN*UNITS1
2706 VZ1=VZMAX*UNITS1
2707 IF(VZMIN.EQ.1000000.) VZMIN=0.
2708 VZ2=VZMIN*UNITS1
2709 EX1=EXMAX*UNITS2
2710 EX2=EXMIN*UNITS2
2711 EZ1=EZMAX*UNITS2
2712 IF(EZMIN.EQ.1000000.) EZMIN=0.
2713 EZ2=EZMIN*UNITS2
2714 XKD1=KDMAX*86400.
2715 XKD2=KDMIN*86400.
2716 XAK1=AKMAX*86400.
2717 XAK2=AKMIN*86400.
2718 XR1=RMAX*86400.
2719 XR2=RMIN*86400.
2720 SXM=STABLX/5280.
2721 SZM=STABLZ/5280.
2722 STD=STABLT/86400.
2723 PRINT 500
2724 PRINT 505

```

```
2725 PRINT 507
2726 PRINT 510
2727 PRINT 507
2728 IF(NZ.EQ.1) PRINT 515
2729 IF(NZ.NE.1) PRINT 520
2730 PRINT 507
2731 IF(LTYPE.EQ.1) GO TO 7
2732 IF(LTYPE.EQ.2) GO TO 5
2733 PRINT 530
2734 GO TO 10
2735 5 PRINT 525
2736 GO TO 10
2737 7 PRINT 523
2738 10 CONTINUE
2739 PRINT 507
2740 PRINT 535
2741 PRINT 507
2742 PRINT 505
2743 PRINT 540, DELTAX, DELMX
2744 PRINT 545, DELTAZ, DELMZ
2745 PRINT 550, SECS1, DELT1
2746 PRINT 551, ITER1
2747 PRINT 552, SECS2, DELT2
2748 PRINT 553, ITER2
2749 PRINT 555, NX
2750 PRINT 560, NZ
2751 PRINT 565, VXMAX, VX1
2752 PRINT 570, VXMIN, VX2
2753 PRINT 572, VZMAX, VZ1
2754 PRINT 573, VZMIN, VZ2
```

2755
2756
2757
2758
2759
2760
2761
2762
2763
2764

PRINT 575, EXMAX, EX1
PRINT 580, EXMIN, EX2
PRINT 585, EZMAX, EZ1
PRINT 590, EZMIN, EZ2
PRINT 600, WMAX
PRINT 605, WMIN
PRINT 610, XKD1, KDMAX
PRINT 615, XKD2, KDMIN
PRINT 620, DEMAX
PRINT 630, NC

C
C
C
C
C

C XUPEAK AND XLPEAK ARE CALCULATED ASSUMING DISTANCES ARE
C DECREASING IN THE DOWNSTREAM DIRECTION
C -----

2765
2766
2767
2768
2769
2770
2771
2772
2773
2774
2775
2776
2777
2778
2779

XUPEAK = XSTART - (MSU-1)*DELMX
XLPEAK = XSTART - (MSL-1)*DELMX
PRINT 635, XUPEAK, MSU
PRINT 637, XLPEAK, MSL
PRINT 500
IF(.NOT.SECOND) GO TO 20
PRINT 800
PRINT 810, XR1, RMAX
PRINT 815, XR2, RMIN
PRINT 820, XAK1, AKMAX
PRINT 825, XAK2, AKMIN
PRINT 830, OUTMAX
20 CONTINUE
PRINT 505
PRINT 502

```

2780 PRINT 640
2781 PRINT 645, STABLX, SXM
2782 PRINT 650, STABLZ, SZM
2783 PRINT 655, STABLT, STD
2784 PRINT 660, R1
2785 PRINT 665, R2
2786 PRINT 505
2787 IF(NC.EQ.0) GO TO 100
2788 PRINT 500
2789 PRINT 700
2790 IF(NZ.EQ.1) GO TO 50
2791 DO 25 I=1,NC

```

C
C
C
C
C

 WHERE IS CALCULATED ASSUMING DISTANCES ARE DECREASING IN
 THE DOWNSTREAM DIRECTION

```

2792 WHERE=XSTART-(INSECT(I)-1)*DELMX
2793 PRINT 710, INSECT(I), WHERE, DELTAZ
2794 DO 25 Z=1,NZ
2795 PRINT 715, INSECT(I), Z, CONCIN(I,Z)
2796 25 CONTINUE
2797 GO TO 100
2798 50 DO 75 I=1,NC
2799 WHERE=XSTART-(INSECT(I)-1)*DELMX
2800 PRINT 705, INSECT(I), WHERE, CONCIN(I,1)
2801 PRINT 502
2802 75 CONTINUE
2803 100 CONTINUE
2804 PRINT 500

```

```

2805 500 FORMAT(1H1)
2806 502 FORMAT(/)
2807 505 FORMAT(T45,*,*,T82,**)
2808 507 FORMAT(T45,*,*,T82,**)
2809 510 FORMAT(T45,*,*,T56,*,ESTUARY SIMULATION', T82, **)
2810 515 FORMAT(T45,*,*,T48,*,ONE DIMENSIONAL EXPLICIT METHOD',T82,**)
2811 520 FORMAT(T45,*,*,T48,*,TWO DIMENSIONAL EXPLICIT METHOD',T82,**)
2812 523 FORMAT(T45,*,*,T53,*,CONSTANT CONCENTRATION',T82,**)
2813 525 FORMAT(T45,*,*,T54,*,INSTANTANEOUS RELEASE', T82,**)
2814 530 FORMAT(T45,*,*,T56,*,CONTINUOUS LOADING', T82, **)
2815 535 FORMAT(T45,*,*,T51,*,PROGRAMMER -- JONATHAN YOUNG', T82,**)
2816 540 FORMAT( /, T53, *,X INCREMENT =', F7.0, *, FEET (' , F8.5, * MILES
      $),,/)
2817 545 FORMAT(T53, *,Z INCREMENT =', F7.0, *, FEET (' , E10.3, * MILES )'//)
2818 550 FORMAT(T42, *,INITIAL TIME INCREMENT =',F7.0, *, SECONDS (' , F6.4,
      $ , DAYS )'//)
2819 551 FURMAT(T44,*,NUMBER OF ITERATIONS = ',I5,/)
2820 552 FORMAT( T42, *,REVISED TIME INCREMENT = ', F7.0, *, SECONDS(' ,F6.4,
      $ , DAYS )'//)
2821 553 FORMAT(T44,*,NUMBER OF ITERATIONS = ',I5,/)
2822 555 FORMAT(T37, *,NUMBER OF HORIZONTAL PCINTS =', I4,/)
2823 560 FORMAT(T39, *,NUMBER OF VERTICAL POINTS =',I4,/)
2824 565 FORMAT(T37, *,MAXIMUM HORIZONTAL VELOCITY = ', F5.2, *, FEET/SECOND
      $(' ,F6.2, *, MILES/DAY )'//)
2825 570 FORMAT(T37, *,MINIMUM HCRIZONIAL VELOCCITY = ', F5.2, *, FEET/SECND
      $(' ,F6.2, *, MILES/DAY )'//)
2826 572 FORMAT(T39, *,MAXIMUM VERTICAL VELOCITY =', E10.3, *, FEET/SECOND ('
      $ , E10.3, *, MILES/DAY )'//)
2827 573 FORMAT(T39, *,MINIMUM VERTICAL VELOCITY =', E10.3, *, FEET/SECOND ('
      $ , E10.3, *, MILES/DAY )'//)

```

```

2828 575 FORMAT(T35, 'MAXIMUM HORIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
      $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )',/)
2829 580 FORMAT(T35, 'MINIMUM HORIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
      $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )',/)
2830 585 FORMAT(T37, 'MAXIMUM VERTICAL DISPERSION = ', F9.3, ' FEET SQUARED
      $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )',/)
2831 590 FORMAT(T37, 'MINIMUM VERTICAL DISPERSION = ', F9.3, ' FEET SQUARED
      $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )',/)
2832 600 FORMAT(T51, 'MAXIMUM WIDTH = ', F5.0, ' FEET',/)
2833 605 FORMAT(T51, 'MINIMUM WIDTH = ', F5.0, ' FEET',/)
2834 610 FORMAT(T46, 'MAXIMUM DECAY RATE = ', F5.3, ' PER DAY ( ', E10.3,
      $ ' PER SECOND )',/)
2835 615 FORMAT(T46, 'MINIMUM DECAY RATE = ', F5.3, ' PER DAY ( ', E10.3,
      $ ' PER SECOND )',/)
2836 620 FORMAT(T42, 'OTHER DEMANDS, MAXIMUM = ', F5.2,/)
2837 630 FORMAT(T26, 'NUMBER OF INITIAL CONCENTRATION VALUES = ', I2,/)
2838 635 FORMAT(T16, 'INITIAL LOCATION OF UPPERMOST PEAK CONCENTRATION = ',
      $ F5.2, ' MILES (SECTION NUMBER ', I2, ')',/)
2839 637 FORMAT(T16, 'INITIAL LOCATION OF LOWERMOST PEAK CONCENTRATION = ',
      $ F5.2, ' MILES (SECTION NUMBER ', I2, ')',/)
2840 640 FORMAT(T56, 'STABILITY CRITERIA',/)
2841 645 FORMAT(T35, 'MAXIMUM ALLOWABLE X INCREMENT = ', F7.0, ' FEET ( ',
      $ F8.5, ' MILES )',/)
2842 650 FORMAT(T35, 'MAXIMUM ALLOWABLE Z INCREMENT = ', F7.0, ' FEET ( ',
      $ F8.5, ' MILES )',/)
2843 655 FORMAT(T32, 'MAXIMUM ALLOWABLE TIME INCREMENT = ', F7.0, ' SECONDS
      $ ( ', E10.3, ' DAYS )',/)
2844 660 FORMAT(T38, 'ACTUAL DELTAT/(DELTAX**2.) = ', E10.3,/)
2845 665 FORMAT(T38, 'ACTUAL DELTAT/(DELTAZ**2.) = ', E10.3,/)

```

```

2846 700 FORMAT(T25, 'LOCATIONS OF INITIAL CONCENTRATIONS',//)
2847 705 FORMAT( 'A WASTE SOURCE IS LOCATED AT STATION ', I2, ' (MILE ',
      $ F6.3, ' ). THE CONCENTRATION IS ', F10.2, ' PPM. )
2848 710 FORMAT( 1X, 'AN INITIAL CONCENTRATION IS FOUND AT STATION ', I2,
      1 ' (MILE ', F6.3, ' ).', //, ' THE CONCENTRATIONS AT ', F7.1,
      2 ' FOOT INTERVALS WITH DEPTH ARE )
2849 715 FORMAT(T15, 'C( ', I2, 'I2', I1) = ', F9.2, ' PPM )
2850 800 FORMAT(T45, '*** SECONDARY INPUT PARAMETERS ***',//)
2851 810 FORMAT(T41, 'MAXIMUM REAERATION RATE = ',
      $ F5.3, ' PER DAY ( ', E10.3, ' PER SECOND )', //)
2852 815 FORMAT(T41, 'MINIMUM REAERATION RATE = ',
      $ F5.3, ' PER DAY ( ', E10.3, ' PER SECOND )', //)
2853 820 FORMAT(T41, 'MAXIMUM ANAEROBIC DECAY = ',
      $ F5.3, ' PER DAY ( ', E10.3, ' PER SECOND )', //)
2854 825 FORMAT(T41, 'MINIMUM ANAEROBIC DECAY = ',
      $ F5.3, ' PER DAY ( ', E10.3, ' PER SECOND )', //)
2855 830 FORMAT(T35, 'OTHER OXYGEN DEMANDS, MAXIMUM = ', F6.3, //)
2856 RETURN
2857 END

```

```

C-----
2858 SUBROUTINE PRINT2
C
C *****
C ***** SUBROUTINE PRINT2 *****
C *****
C *****
C
C THIS SUBROUTINE PRINTS OUT THE TIME AND THE CONCENTRATION
C PROFILE ACCORDING TO A FORMAT PREVIOUSLY READ INTO THE PROGRAM.
C THIS SUBROUTINE CAN BE CHANGED ACCORDING TO THE NEEDS OF THE USER.
C ***WARNING--DO NOT CHANGE ANY VARIABLE OCCURRING IN A COMMON BLOCK.
C
C-----

```

```

2859 LOGICAL SECOND
2860 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,CCOUNT,OPTION
2861 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
2862 DIMENSION NZREV(20)
2863 COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CONCIN(51,21),INSECT(51),KDAYS(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
2864 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
2865 COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
1 AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
2 SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN

```



```

2866 DAYS = TIME/86400.
2867 HOURS=DAYS*24.
2868 XFIRST=XSTART-(NX-1)*DELTA/5280.
2869 DO 100 IZ = 1, NZ
2870 NZREV(IZ)=IFIX(DELTA*(IZ-2))
2871 CONTINUE
2872 IF(T.EQ.3) GO TO 300
2873 WRITE(6,FMT1) HOURS,DAYS
2874 WRITE(6,FMT2) (NZREV(I),I=2,NZM1)
2875 DO 200 X=1,NX,XPRINT
2876 XMILE = XFIRST + ((X-1)*DELTA*XPRINT)/5280.
2877 M=NX+1-X
2878 WRITE(6,FMT3) XMILE, (C(M,Z,T),Z=1,NZ)
2879 CONTINUE
2880 GO TO 600
2881 CONTINUE
2882 WRITE(6,FMT4) HOURS, DAYS
2883 WRITE(6,FMT5) (NZREV(I), I=2,NZM1)
2884 DO 500 X=1,NX,XPRINT
2885 XMILE = XFIRST + ((X-1)*DELTA*XPRINT)/5280.
2886 M=NX+1-X
2887 WRITE(6,FMT6) XMILE, (C(M,Z,T),Z=1,NZ)
2888 CONTINUE
2889 CONTINUE
2890 RETURN
2891 END

```

```

2892 SUBROUTINE ERROR(*)
C-----
C
C *****
C ***** SUBROUTINE ERROR *****
C *****
C
C
C THIS SUBROUTINE CORRECTS SOME OF THE MOST COMMON ERRORS
C IN THE INPUT DATA AND PRINTS OUT APPROPRIATE ERROR MESSAGES.
C-----
C
2893 LOGICAL SECOND
2894 INTEGER X,Z,T,TNEXT,TPRINT,XPRINT,COUNT,OPTION
2895 REAL KD,KDMAX,KDMIN,KDAYS,LOWER
2896 COMMON/ARRAYS/ C(51,21,4),VX(51,21),VZ(51,21),EX(51,21),EZ(51,21),
1 W(51),CONCIN(51,21),INSECT(51),KDAY5(51),KD(51),
2 DEMAND(51,21),FMT1(40),FMT2(40),FMT3(20)
2897 COMMON/NAMES/X,Z,T,NX,NZ,NC,NS,EXMAX,EXMIN,VXMAX,VXMIN,WMAX,WMIN,
1 KDMAX,KDMIN,ITER,ITER1,ITER2,DELT1,DELT2,TNEXT,
2 TPRINT,XPRINT,STABLX,STABLZ,STABLT,R1,R2,VZMAX,SET,
3 VZMIN,EZMAX,EZMIN,DEMAX,DEMIN,NPPAGE,DELTAX,DELTAZ,
4 DELTAT,XSTART,XSF,TSTART,COUNT,IPAGE,UPPER,LOWER,
5 MSU,MSL,TIME,NXM1,NXM2,NZM1,NZM2,ZSF,LTYPE,OPTION
2898 COMMON/OXYGEN/OXOUT(51,21),R(51,21),RDAYS(51,21),OXSAT(51),AK(51),
1 AKDAYS(51),FMT4(40),FMT5(40),FMT6(20),
2 SECOND,AKMAX,RMAX,OUTMAX,AKMIN,RMIN,OUTMIN
2899 IF(OPTION.LT.8) GO TO 200
2900 PRINT 181
2901 181 FORMAT(//////, ' *****PROGRAM TERMINATED BECAUSE OPTION IS GREATER TH
$AN 7****,/,1H1)
2902 RETURN
2903 200 IF(LTYPE.LT.3) GO TO 300

```

```

2904 PRINT 201
2905 201 FORMAT(///,IX,***** WARNING -- LTYPE IS GREATER THAN OR EQUAL T
      10 3.,/,4X,IF LTYPE EQUALS 3, THE USER MUST SUPPLY A SUBROUTINE F
      20R THE LOADING CONDITIONS.,/,4X,IF LTYPE IS GREATER THAN 3, THE
      3 PROGRAM IS TERMINATED *****/,IHL)
2906 IF(LTYPE.GT.3) RETURN1
2907 RETURN
2908 300 CONTINUE
2909 IF((SECOND).AND.(OPTION.EQ.3.OR.OPTION.EQ.4.OR.OPTION.EQ.7))
      $ GO TO 350
2910 GO TO 400
2911 350 PRINT 360
2912 360 FORMAT(///,IX,***** PROGRAM TERMINATED BECAUSE WHEN SECOND EQUAL
      1S 2 THEN OPTION CANNOT EQUAL 3 OR 4 OR 7 ***** )
2913 RETURN1
2914 400 CONTINUE
2915 IF(ITER.NE.0) GO TO 820
C-----
C TERMINATE PROGRAM WITH PRINTED MESSAGE IF INPUT DATA
C VIOLATES STABILITY CRITERIA
C-----
2916 PRINT 815
2917 815 FORMAT(///,IX,10(.'*'),PROGRAM TERMINATED BECAUSE STABILITY CONDI
      $TIONS WERE VIOLATED*****/,IHL)
2918 RETURN1
2919 820 CONTINUE
2920 RETURN
2921 END

```

Input Data for OXTRANS-I

Each line represents a new card unless single spaced.

```
1.  
.TRUE.  
33, 7  
0.25, 10.,  
0.006944444, 2  
0.0, 0  
1, 1  
17., 4.0, 1, 1, 1  
1, 6  
33*0.0, 33*0.366667, 33*0.440, 33*0.433333, 33*0.273333,  
33*0.233333, 33*0.0  
231*450.  
497., 510., 524., 537., 550., 562., 575., 562., 550., 537.,  
525., 568., 611., 654., 697.,  
756., 814., 872., 931., 863., 796., 728., 661., 657., 654.,  
650., 647., 633., 619., 605.,  
591., 586., 582.  
33*0.25  
231*0.0  
2  
7*13.  
231*0.0
```

66*0.00333333, 33*0.004, 33*0.00394, 33*0.002485, 66*0.00212

(///, 12X, 100('*'), /, 12X, '*', /, 12X, '*', 2X,
'CONCENTRATIONS AT TIME = ',

F7.2, ' HOURS (', F6.4, ' DAYS)', /, 12X, '*', /, 12X,
100('*'))

(12X, '*', /, 12X, '*', 42X, 'DEPTH', /, 12X, '*', /, 12X,
'*', 3X, 'IMAGE', 4X,

5(12, 'FEET'), 'IMAGE', /, 1X, 111('*'), /, 12X, '*')

(1X, 'MILE', F5.2, 1X, '*', 2X, 7(F6.1, 4X))

(///, 12X, 100('*'), /, 12X, '*', /, 12X, '*', 2X,
'OXYGEN AT TIME = ',

F7.2, 'HOURS (', F6.4, ' DAYS)', /, 12X, '*', /, 12X,
100('*'))

(12X, '*', /, 12X, '*', 42X, 'DEPTH', /, 12X, '*', /, 12X,
'*', 3X, 'IMAGE', 4X,

5(12, ' FEET '), 'IMAGE', /, 1X, 111('*'), /, 12X, '*')

(1X, 'MILE', F5.2, 1X, '*', 2X, 7(F6.1, 4X))

33*0., 33*0.1, 165*0.0

33*0.08

33*8.

231*0.

COMPUTER PROGRAM FOR
STABLE-I

Object Code = 928 bytes

Array Area = 0 bytes

Total = 928 bytes

STABLE-I

THIS PROGRAM COMPUTES THE STABILITY CRITERIA FOR TIME
 INCREMENTS AND DISTANCE INCREMENTS FOR A LARGE NUMBER OF
 COMBINATIONS OF DISPERSION COEFFICIENTS AND VELOCITIES. THESE
 CRITERIA APPLY TO ONE DIMENSIONAL FINITE DIFFERENCE MODELS FOR
 MASS TRANSPORT IN AN ESTUARY.

LOGIC AND PROGRAMMING--JONATHAN YOUNG, TEXAS A&M UNIVERSITY

*** OUTPUT VARIABLES ***

- E = DISPERSION COEFFICIENT (MILES**2/DAY)
- EF = DISPERSION COEFFICIENT (FEET**2/SECOND)
- U = VELOCITY (MILES/DAY)
- VF = VELOCITY (FEET/SECOND)
- DELX = ALLOWABLE DISTANCE INCREMENT (MILES)
- DELXF = ALLOWABLE DISTANCE INCREMENT (FEET)
- DELT = ALLOWABLE TIME INCREMENT (DAYS)
- DELTS = ALLOWABLE TIME INCREMENT (SECONDS)

```

3202 PRINT 10
3203 10 FORMAT(1H1, T5, 'E', T20, 'EF', T35, 'U', T50, 'UF', T65, 'DELX',
$ T80, 'DELXF', T95, 'DELT', T110, 'DELTS',//)
3204 DELX=.25
3205 E=.333E-05
3206 DO 200 J=1,7
3207 UF=0.5E-05
3208 DO 100 I=1,7
3209 U=UF*86400./5280.
3210 IF(U.NE.0) DELX=2.*E/U
3211 DELT=(DELX**2.)/(2.*E)
3212 DELXF=5280.*DELX
3213 DELTS=86400.*DELT
3214 EF=(5280.*5280.*E)/86400.
3215 PRINT 75, E,EF,U,UF,DELX,DELXF,DELT,DELTS
3216 75 FORMAT(1X, 8E15.5)
3217 UF=UF*2.
3218 100 CONTINUE
3219 E=2.*E
3220 200 CONTINUE
3221 STOP
3222 END

```

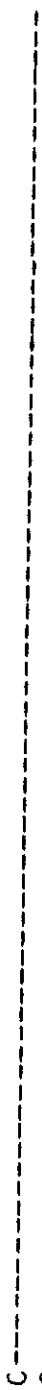

COMPUTER PROGRAM FOR
STABLE-II

Object Code = 4016 bytes

Array Area = 0 bytes

Total = 4016 bytes

C EXMIN = MINIMUM DISPERSION COEFFICIENT IN THE X DIRECTION
C (FEET**2/SECOND)
C EXMAX = MAXIMUM DISPERSION COEFFICIENT IN THE X DIRECTION
C (FEET**2/SECOND)
C EZMIN = MINIMUM DISPERSION COEFFICIENT IN THE Z DIRECTION
C (FEET**2/SECOND)
C EZMAX = MAXIMUM DISPERSION COEFFICIENT IN THE Z DIRECTION
C (FEET**2/SECOND)
C VXMAX = MAXIMUM VELOCITY IN THE X DIRECTION (FEET/SECOND)
C VZMAX = MAXIMUM VELOCITY IN THE Z DIRECTION (FEET/SECOND)
C KDAY = MAXIMUM DECAY RATE (/DAY)
C
C
C
C



```

2923 REAL KDMAX
2924 REAL KDAY5
2925 READ , NSETS
2926 PRINT, NSETS
2927 DO 1000 II=1,NSETS
2928 READ , DELMX,DELTAZ,NZ,EXMIN,EXMAX,EZMIN,EZMAX,VXMAX,VZMAX,KDAY5
2929 PRINT,DELMX,DELTAZ,NZ,EXMIN,EXMAX,EZMIN,EZMAX,VXMAX,VZMAX,KDAY5
2930 DELTAX=DELMX*5280.
2931 XKD1=KDAY5
2932 KDMAX=KDAY5/86400.
2933 UNITS1 = 86400./5280.
2934 UNITS2 = 86400./{5280.*5280.}
2935 DELMZ=DELTAZ/5280.
2936 VX1=VXMAX*UNITS1
2937 VZ1=VZMAX*UNITS1
2938 EX1=EXMAX*UNITS2
2939 EX2=EXMIN*UNITS2
2940 EZ1=EZMAX*UNITS2
2941 EZ2=EZMIN*UNITS2
2942 STABLX = DELTAX
2943 STABLZ = DELTAZ
2944 IF(NZ.NE.1) GO TO 500
2945 IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
2946 TERM=2.*EXMAX+DELTAX**2.*KDMAX
2947 IF(TERM.NE.0.0) STABLZ={DELTAX**2.}/TERM
2948 GO TO 800
2949 500 CONTINUE
2950 IF(EXMIN.GT.0.0.AND.VXMAX.GT.0.0) STABLX=2.*EXMIN/VXMAX
2951 IF(EZMIN.GT.0.0.AND.VZMAX.GT.0.0) STABLZ=2.*EZMIN/VZMAX

```

```

2952 TERM=2.*(EXMAX*DELTAZ**2.+EZMAX*DELTAZ**2.)+KDMAX*(DELTAZ**2)*
      (DELTAZ**2)
2953 IF (TERM.NE.0.0) STABLT=(DELTAZ**2.)*(DELTAZ**2.)/TERM
2954 800 CONTINUE
2955 SXM=STABLX/5280.
2956 SZM=STABLZ/5280.
2957 STD=STABLT/86400.
2958 PRINT 100
2959 PRINT 540, DELTAX, DELMX
2960 PRINT 545, DELTAZ, DELMZ
2961 PRINT 560, NZ
2962 PRINT 565, VXMAX, VX1
2963 PRINT 572, VZMAX, VZ1
2964 PRINT 575, EXMAX, EX1
2965 PRINT 580, EXMIN, EX2
2966 PRINT 585, EZMAX, EZ1
2967 PRINT 590, EZMIN, EZ2
2968 PRINT 610, XKD1, KDMAX
2969 PRINT 505
2970 PRINT 502
2971 PRINT 640
2972 PRINT 645, STABLX, SXM
2973 PRINT 650, STABLZ, SZM
2974 PRINT 655, STABLT, STD
2975 PRINT 505
2976 100 FORMAT(IH1)
2977 502 FORMAT(/)
2978 505 FORMAT(T45, 38(('*'))
2979 540 FORMAT( /, T53, 'X INCREMENT =', F7.0, ' FEET (', F8.5, ' MILES
      $),', /)

```

```

2980 545 FORMAT(I53, ' Z INCREMENT =', F7.0, ' FEET (', E10.3, ' MILES )'//)
2981 560 FORMAT(I39, ' NUMBER OF VERTICAL POINTS =', I4, '//)
2982 565 FORMAT(I37, ' MAXIMUM HORIZONTAL VELOCITY = ', F5.2, ' FEET/SECOND
      $(', F6.2, ' MILES/DAY )', '//)
2983 572 FORMAT(I39, ' MAXIMUM VERTICAL VELOCITY =', E10.3, ' FEET/SECOND ( '
      $ , E10.3, ' MILES/DAY )', '//)
2984 575 FORMAT(I35, ' MAXIMUM HORIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
      $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )', '//)
2985 580 FORMAT(I35, ' MINIMUM HORIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
      $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY )', '//)
2986 585 FORMAT(I37, ' MAXIMUM VERTICAL DISPERSION = ', F6.3, ' FEET SQUARED
      $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )', '//)
2987 590 FORMAT(I37, ' MINIMUM VERTICAL DISPERSION = ', F6.0, ' FEET SQUARED
      $ / SECOND ( ', E10.3, ' MILES SQUARED / DAY )', '//)
2988 610 FORMAT(I46, ' MAXIMUM DECAY RATE = ', F5.3, ' PER DAY ( ', E10.3,
      $ ' PER SECOND )', '//)
2989 640 FORMAT(I56, ' STABILITY CRITERIA', '//)
2990 645 FORMAT(I35, ' MAXIMUM ALLOWABLE X INCREMENT = ', F7.0, ' FEET ( ',
      $ F8.5, ' MILES )', '//)
2991 650 FORMAT(I35, ' MAXIMUM ALLOWABLE Z INCREMENT = ', F7.0, ' FEET ( ',
      $ F8.5, ' MILES )', '//)
2992 655 FORMAT(I32, ' MAXIMUM ALLOWABLE TIME INCREMENT = ', F7.0, ' SECONDS
      $ ( ', E10.3, ' DAYS )', '//)
2993 1000 CONTINUE
2994 STOP
2995 END

```

Input Data for STABLE-II

Each line represents a new card unless single spaced.

1

0.2, 5., 11, 500., 500., 0.001, 0.005, 0.441, 0., 0.384

COMPUTER PROGRAM FOR
EXACT-I

Object Code = 5664 bytes

Array Area = 400 bytes

Total = 6064 bytes

C
C
C
C
C
C
C
C
C
C
C
C
C
C
C
C
C
C
C
C
C
C

***** EXACT-I *****
***** ANALYTICAL SOLUTIONS FOR *****
***** ONE DIMENSIONAL CONVECTIVE DISPERSION *****

THIS COMPUTER PROGRAM COMPUTES THE PROFILES FOR VARIOUS
CLOSED-FORM SOLUTIONS TO THE ESTUARY MASS TRANSPORT EQUATION.
THESE SOLUTIONS ARE OBTAINED BY ASSUMING CONSTANT COEFFICIENTS
AND ESTUARIES OF INFINITE LENGTH. SOLUTIONS ARE CALCULATED FOR
CONSTANT CONCENTRATIONS, INSTANTANEOUS RELEASES, AND CONTINUOUS
DISCHARGES. THIS PROGRAM CAN BE USED TO CHECK THE ACCURACY OF
FINITE DIFFERENCE FORMULATIONS.

LOGIC AND PROGRAMMING--JONATHAN YOUNG, TEXAS A&M UNIVERSITY


```

1783 DIMENSION C(100)
1784 DATA C/100*0.0/, PI/ 3.141593/
1785 READ , DELTAX, DELTAT, U, EX, CZERO, ITEND, IXEND, XBEGIN, LTYPE,
$ XKD
1786 PRINT, DELTAX, DELTAT, U, EX, CZERC, ITEND, IXEND, XBEGIN, LTYPE,
$ XKD
1787 DAYS=DELTAT
1788 SECS=DELTAT
1789 IF(DELTAT.LE.1.) SECS=DELTAT*86400.
1790 IF(DELTAT.GT.1.) DAYS=DELTAT/86400.
1791 DELF=DELTAX
1792 DELM=DELTAX
1793 IF(DELTAX.GT.2.0) DELM=DELTAX/5280.
1794 IF(DELTAX.LE.2.0) DELF=DELTAX*5280.
1795 UM=U*86400./5280.
1796 D=EX
1797 DM=EX
1798 IF(EX.LE.25.0) D=EX*5280.*5280./86400.
1799 IF(EX.GT.25.0) DM=EX*86400./(5280.*5280.)
1800 XKDS=XKD/86400.
1801 PRINT 5
1802 5 FORMAT(1H1,1X,110(' '),//)
1803 PRINT 7
1804 7 FORMAT(40X, 42(' '))
1805 PRINT 25
1806 25 FORMAT(40X, ***** ONE DIMENSIONAL ANALYSIS *****)

```

```

1807 GO TO (10,20,30,40,50), LTYPE
1808 10 PRINT 11, CZERO
1809 11 FORMAT(40X, '***** CONSTANT CONCENTRATION *****', //, 10X,
    $ , ' THE FOLLOWING PARAMETERS ARE APPLIED TO A CONSTANT CONCENTRAT
    $ION OF ', F7.0, ' PPM MAINTAINED AT X = 0.0', /)
1810 GO TO 60
1811 20 PRINT 11, CZERO
1812 GO TO 60
1813 30 PRINT 31, CZERO
1814 31 FORMAT(40X, '***** INSTANTANEOUS RELEASE *****', //, 10X,
    $ , ' THE FOLLOWING PARAMETERS ARE APPLIED TO AN INSTANTANEOUS RELEAS
    $E OF ', F10.0, ' UNITS OF MASS PER UNIT AREA', /, 15X, ' AT X=0.0 A
    $ND TIME = 0.0', /)
1815 GO TO 60
1816 40 PRINT 31, CZERO
1817 GO TO 60
1818 50 PRINT 51, CZERO
1819 51 FORMAT(40X, '***** CONTINUOUS DISCHARGE *****', //, 10X,
    $ , ' THE FOLLOWING PARAMETERS ARE APPLIED TO A CONTINUOUS DISCHARG
    $E OF ', F9.0, ' UNITS OF MASS PER UNIT AREA PER UNIT TIME AT X = C
    $ .0 ', /)
1820 GO TO 60
1821 60 CONTINUE
1822 PRINT 535, LTYPE
1823 535 FORMAT(//, T59, 'LTYPE =', I3, /)
1824 PRINT 540, DELF, DELM
1825 540 FORMAT( T53, 'X INCREMENT =', F7.0, ' FEET (', F8.5, ' MILES
    $)', /)
1826 PRINT 550, SECS, DAYS
1827 550 FORMAT(T50, 'TIME INCREMENT =', F7.0, ' SECONDS (', E10.3, ' DAYS
    $)', /)

```

```

1828 PRINT 565, U,UM
1829 565 FORMAT(T37, ' HCRIZONTAL VELOCITY = ', F5.2, ' FEET/SECND
      $(,F6.2, ' MILES/DAY ),,/)
1830 PRINT 575, D, DM
1831 575 FORMAT(T35, ' HCRIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
      $ED / SECOND ( ', F5.2, ' MILES SQUARED / DAY ),,/)
1832 PRINT 580, XKD, XKDS
1833 580 FORMAT(T54, 'DECAY RATE = ', F6.3, ' PER DAY ( ', E10.3,
      $ ' PER SECOND ),,/)
1834 PRINT 65
1835 65 FORMAT(//,IX,110('*),//)
1836 DO 1000 IT=1,ITEND
1837 T=SECS*IT
1838 TIME=T/86400.
1839 GO TO (100, 300, 500) , LTYPE
1840 100 CONTINUE
1841 IF(U.NE.0.0) GO TO 200
1842 DO 120 IX=1,IXEND
1843 X=(IX-1)*DELF+XBEGIN
1844 B3=X/(2.*SQRT(D*T))
1845 C(IX)=CZERO*ERFC(B3)
1846 120 CONTINUE
1847 GO TO 800
1848 200 DO 220 IX=1,IXEND
1849 X=(IX-1)*DELF+XBEGIN
1850 A=U*X/D
1851 B1=(X+U*T)/(2.*SQRT(D*T))
1852 B2=(X-U*T)/(2.*SQRT(D*T))
1853 C(IX)=(CZERO/2.)*(EXP(A)*ERFC(B1)+ERFC(B2))

```

```

1854 220 CONTINUE
1855 GO TO 800
1856 300 XM=CZERO
1857 DECAY = -XKD*TIME
1858 DO 350 IX=1,IXEND
1859 X=(IX-1)*DELF+XBEGIN
1860 C1=XM/SQRT(4.*PI*D*T)
1861 C2=-(X-U*T)**2/(4.*D*T)
1862 C(IX)=C1*EXP(C2)
1863 C(IX)=C(IX)*EXP(DECAY)
1864 350 CONTINUE
1865 GO TO 800
1866 500 XM=CZERO
1867 DO 520 IX=1,IXEND
1868 X=(IX-1)*DELF+XBEGIN
1869 E1=XM*T**0.5/SQRT(PI*D)
1870 E2=-X**2/(4.*D*T)
1871 E3=XM*X/(2.*D)
1872 E4=X/(2.*SQRT(D*T))
1873 C(IX)=E1*EXP(E2)-E3*ERFC(E4)
1874 520 CONTINUE
1875 GO TO 800
1876 800 CONTINUE
1877 PRINT 825, T, TIME
1878 825 FORMAT('///', ' TIME = ', F7.0, ' SECONDS ( ', F8.5, ' DAYS )',/)

```

```

1879 MSTOP=IXEND/2
1880 DO 875 M=1,MSTOP
1881 P1=DELF*(M-1)
1882 P2=DELM*(M-1)
1883 P3=DELF*(M+MSTOP-1)
1884 P4=DELM*(M+MSTOP-1)
1885 M1=M+MSTOP
1886 PRINT 850, P1,P2,C(M),P3,P4,C(M1)
1887 850 FORMAT(2(' C AT ', F7.0, ' FEET (' , F8.4, ' MILES ) = ', F10.2,
      $      10X))
1888 875 CONTINUE
1889 1000 CONTINUE
1890 STOP
1891 END

```

Input Data for EXACT-I

Each line represents a new card unless single spaced.

0.25, 0.1, 5., 10., 100., 2, 20, -5280., 1, 0.

COMPUTER PROGRAM FOR
EXACT-II

Object Code = 4,656 bytes

Array Area = 40,160 bytes

Total = 44,816 bytes


```

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1914
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1916
1917
1918
1919
1920

C
DIMENSION C(100,100)
DIMENSION FMT1(40)
INTEGER COUNT
DATA C/10000*0.0/
READ , NX,NZ,DELTA, DELFZ,DELTA, XBEGIN,ZBEGIN,ITER
PRINT, NX,NZ,DELTA, DELFZ,DELTA, XBEGIN,ZBEGIN,ITER
READ , EXF,EZF,VXF,VZF, XM,XKD
PRINT, EXF,EZF,VXF,VZF, XM,XKD
READ 205, (FMT1(I), I=1,40)
PRINT 202
202 FORMAT(/,/,1X, 'FORMAT FOR OUTPUT')
PRINT 205, (FMT1(I),I=1,40)
205 FORMAT (20A4,/,20A4)
DELTA=1./24.
COUNT=2
PI=3.14159
SECS=DELTA*86400.
DELFX=DELTA*5280.
DELTAZ=DELFZ/5280.
VX=VXF*86400./5280.
VZ=VZF*86400./5280.
EX=EXF*86400./5280.*5280.)
EZ= EZF*86400./5280.*5280.)
XKDS = XKD/86400.
PRINT 5
5 FORMAT(1H1,1X,110('**'),/)
PRINT 7
7 FORMAT(40X, 42('**'))

```

```

1921 PRINT 25
1922 25 FORMAT(40X, '***** TWO DIMENSIONAL ANALYSIS *****')
1923 PRINT 31, XM
1924 31 FORMAT(40X, '***** INSTANTANEOUS RELEASE *****', /,
    $ 40X, 42(' '), //,
    $ ' THE FOLLOWING PARAMETERS ARE APPLIED TO AN INSTANTANEOUS RELEASE
    $E OF ', F10.0, ' PPM AT X = 0.0 AND TIME = 0.0', /)
1925 PRINT 540, DELFX, DELTAX
1926 540 FORMAT( T53, ' X INCREMENT =', F7.0, ' FEET (', F8.5, ' MILES
    $)', //)
1927 PRINT 545, DELFZ, DELTAZ
1928 545 FORMAT( T53, ' Z INCREMENT =', F7.0, ' FEET (', F8.5, ' MILES
    $)', //)
1929 PRINT 550, SECS, DELTAT
1930 550 FORMAT(T50, ' TIME INCREMENT =', F7.0, ' SECONDS (', E10.3, ' DAYS
    $)', //)
1931 PRINT 565, VXF, VX
1932 565 FORMAT(T37, ' HORIZONTAL VELOCITY = ', F5.2, ' FEET/SECOND
    $)', F6.2, ' MILES/DAY )', //)
1933 PRINT 570, VZF, VZ
1934 570 FORMAT(T37, ' VERTICAL VELOCITY = ', F5.2, ' FEET/SECOND
    $)', F6.2, ' MILES/DAY )', //)
1935 PRINT 575, EXF, EX
1936 575 FORMAT(T35, ' HORIZONTAL DISPERSION = ', F6.0, ' FEET SQUAR
    $E / SECOND ( ', F5.2, ' MILES SQUARED / DAY )', //)
1937 PRINT 580, EZF, EZ
1938 580 FORMAT(T35, ' VERTICAL DISPERSION = ', F9.3, ' FEET SQUAR
    $E / SECOND ( ', F8.5, ' MILES SQUARED / DAY )', //)
1939 PRINT 582, XKD, XKDS
1940 582 FORMAT(T54, ' DECAY RATE =', F6.3, ' PER DAY ( ', E10.3, ' PER SECCN
    $D )', //)

```

```

1941 XBF=XBEGIN*5280.
1942 PRINT 585, XBF, XBEGIN
1943 585 FORMAT(T49,'INITIAL X VALUE =', F7.0, ' FEET (' , F8.5, ' MILES
    $)',',/')
1944 ZBF=ZBEGIN*5280.
1945 PRINT 590, ZBF, ZBEGIN
1946 590 FORMAT(T49,'INITIAL Z VALUE =', F7.0, ' FEET (' , F8.5, ' MILES
    $)',',/')
1947 PRINT 595
1948 595 FORMAT( 1X, 110('**') )
1949 DO 300 IT=1,ITER
1950 T=IT*DELTA T
1951 DO 200 I=1,NX
1952 X=(I-1)*DELTA X+XBEGIN
1953 DO 100 J=1,NZ
1954 Z=(J-1)*DELTA Z+ZBEGIN
1955 TERM1=-((X-VX*T)**2)/(4.*T*EX)
1956 IF(TERM1.LT.-50.) TERM1=-50.
1957 TERM2=SQRT(2.*T*EX)
1958 TERM3=-((Z-VZ*T)**2)/(4.*T*EZ)
1959 IF(TERM3.LT.-50.) TERM3=-50.
1960 TERM4=SQRT(2.*T*EZ)
1961 C(I,J)=(XM/(2.*PI))*EXP(TERM1)*EXP(TERM3)/(TERM2*TERM4)
1962 XKDT=-XKD*T
1963 C(I,J)=C(I,J)*EXP(XKDT)
1964 100 CONTINUE
1965 200 CONTINUE
1966 PRINT 275
1967 275 FORMAT(1H1)

```

```
1968  
1969  
1970  
1971  
1972  
1973  
1974  
1975  
  
TIME=T*86400.  
HOURS=T*24.  
WRITE(6,FM1) HOURS,T  
PRINT25, ((C(I,J),J=1,NZ),I=1,NX)  
225 FORMAT(1(1X,11(F6.0)))  
300 CONTINUE  
STOP  
END
```

Input Data for EXACT-II

Each line represents a new card unless single spaced.

28, 11, 0.2, 5., 0.04083333, -2.6, 0.0, 1

500., 0.010, 0., 0., 5.0, 0.

(///, 12X, 100('*'), /, 12X, '*',/, 12X, '*', 2X,
'CONCENTRATIONS AT TIME = ',

F7.2, ' HOURS (' , F6.4, ' DAYS)', /, 12X, '*',/, 12X,
100('*'))

COMPUTER PROGRAM FOR
PROFILE-I

Object Code = 1864 bytes
Array Area = 0 bytes
Total = 1864 bytes


```

14  ADOWN=U/(2.*E)*(1.0-SQRT(1.0+4.*XKD*E/(J**2.)))
15  AUP= U/(2.*E)*(1.0+SQRT(1.0+4.*XKD*E/(J**2.)))
16  X=-11.0
17  DO 100 I=1,9
18  X=X+1.0
19  C=CZERO*EXP(AUP*X)
20  WRITE(6,50)X,C
21  50 FORMAT(F10.2,4X,F9.4,/)
22  100 CONTINUE
23  DO 150 J=1,8
24  X=X+0.25
25  C=CZERO*EXP(AUP*X)
26  WRITE(6,50)X,C
27  150 CONTINUE
28
29  DO 200 K=1,20
30  X=X+0.25
31  C=CZERO*EXP(ADOWN*X)
32  WRITE(6,50)X,C
33  200 CONTINUE
34  DO 250 L=1,45
35  X=X+1.0
36  C=CZERO*EXP(ADOWN*X)
37  WRITE(6,50)X,C
38  250 CONTINUE
39  STOP
    END

```

Input Data for PROFILE-I

Each line represents a new card unless single spaced.

10.0 1.0 0.25 10.0

COMPUTER PROGRAM FOR
PROFILE-II

Object Code = 2,864 bytes

Array Area = 12,060 bytes

Total = 14,924 bytes


```

3224 REAL L,K,K2,M1,M2,LZ
3225 DIMENSION L(1005),D(1005),C(1005)
3226 READ , W,Q,U,E,K,K2,CSAT,XUP,XDOWN,DELMX , DI
3227 PRINT, W,Q,U,E,K,K2,CSAT,XUP,XDOWN,DELMX , DI
3228 Q=Q*62.4
3229 W=W/86400. * 1.E6
3230 XXK=K
3231 XXK2=K2
3232 K=K/86400.
3233 K2=K2/86400.
3234 DELTAX=DELMX*5280.
3235 IF(XXK.EQ.XXK2) XUP=0.0
3236 NDIM=(-XUP+XDOWN)/DELMX + 2
3237 TERM1=1. + (4.*K*E)/(U*U)
3238 M1=SQRT(TERM1)
3239 TERM2=1.+(4.*K2*E)/(U*U)
3240 M2=SQRT(TERM2)
3241 LZ=W/(Q*M1)
3242 U2E=U/(2.*E)
3243 XJ=U2E*(1.-M1)
3244 X=(XUP-DELMX)*5280.
3245 PRINT 50
3246 50 FORMAT(IH1,/,/,IX,'BOD AND OXYGEN PROFILES',/,/,
$ T5, 'X', T15, 'L', T25, 'D', T32, 'OX',/)
SIGN=1.
3247 TEST=ABS(XXK-XXK2)
3248 IF(TEST-.001) 250,25,25
3249 25 CONTINUE
3250 DZ=(K*W)/((K2-K)*Q)
3251 DO 200 I=1,NDIM
3252

```

```

3253 X=X+DELTAX
3254 IF(X.GE.0.0) SIGN=-1.
3255 EXPON1=U2E*X*(1.+SIGN*M1)
3256 EXPON2=U2E*X*(1.+SIGN*M2)
3257 L(I)=LZ*EXP(EXPON1)
3258 D(I)=DZ*((1./M1)*EXP(EXPCN1))-((1./M2)*EXP(EXPON2))
3259 D(I)=D(I) + DI*EXP(EXPON2)
3260 C(I)=CSAT-D(I)
3261 XM=X/5280.
3262 PRINT 100, XM,L(I),D(I),C(I)
3263 100 FORMAT(1X,F7.2,3X,F8.3,3X,F5.2,3X,F5.2)
3264 200 CONTINUE
3265 GO TO 400
3266 250 CONTINUE
3267 X=-DELTAX
3268 DO 300 I=1,NDIM
3269 X=X+DELTAX
3270 IF(X.GE.0.0) SIGN=-1.
3271 EXPON=U2E*X*(1.+SIGN*M1)
3272 L(I)=LZ*EXP(EXPON)
3273 DZ=K*LZ*X/(U-2.*E*XJ)
3274 XJX=XJ*X
3275 D(I)=DZ*EXP(EXPON) + DI*EXP(XJX)
3276 C(I)=CSAT-D(I)
3277 XM=X/5280.
3278 PRINT 100, XM,L(I),D(I),C(I)
3279 300 CONTINUE
3280 400 CONTINUE
3281 STOP
3282 END

```


Input Data for PROFILE-II

Each line represents a new card unless single spaced.

50000., 1000., .2, 400., .23, .10, 8., -10., 20., 0.5, 0.0